

Applicant 2019 Submittal and Supplemental Documents

Containing the Following:

- Submittal Cover Letter, Applicant January 2019
- Response to Comments Letter, RSA+ January 2019
- Proposed Reductions Chart, Applicant January 2019
- Winery Comparison Chart 30,000-50,000 Gallons, Applicant, January 2019
- Access Retaining Wall Logistics Plan/Letter, Ledcor Group November 2018
- Tree Easement Document
- Water System Feasibility Study, RSA+ March 2019
- Winery Wastewater Feasibility Study, RSA+ January 2019
- Hydrology Report, RSA+ January 2019
- Agricultural Erosions Control Plan, RSA+ January 2019
- Landscape Plan, Claud Schmidt January 2019

Anthem Winery P14-00320-MOD and Exception to Road and Street Standards, Variance P14-00321-VAR and Viewshed, and Agricultural Erosion Control Plan P14-00322-ECPA Planning Commission Hearing Date (Wednesday, February 5, 2020)



January 11, 2019

Don Barrella Napa County PBES 1195 Third Street, Second Floor Napa, California



3454 Redwood Rd. Napa, CA 94558

www.anthemwinery.com

Delivery via email to Donald.Barrella@countyofnapa.org

RE: Anthem Winery – P14-00320

Don,

Our substantive responses to the questions raised during and after our October $3^{\rm rd}$ Planning Commission hearing are attached, including all the items you requested. These items include:

- 1) Revised Tier 1 Water Calculations;
- 2) Revised Landscaping Plan;
- 3) Revised Hydrology Report;
- 4) Revised Winery Wastewater Feasibility Report;
- 5) Revised Water Feasibility Study;
- 6) Revised Erosion Control Plan;
- 7) A viewshed/driveway exhibit depicting the winery and related improvements from Dry Creek Road;
- 8) A letter from our general contractor detailing how Anthem Winery's driveway will be constructed within the bounds of our property;
- A response from RSA+ addressing questions raised regarding our driveway;
- 10) A winery comparison chart for all wineries with 30,000 gallons 50,000 gallons production that the County has granted visitation since January 1, 2019;
- 11) A copy of the reciprocal "Tree Easement" we shared with Donald Harms and Patricia Damery. (Please note this easement simply prohibits removal or excessive cutting of "the existing mature oak trees" unless the tree is dead or dying. The easement applies within approximately 60-160 feet of a portion of the boundary line between the two properties. This application does not propose the removal of any tree within the tree easement, much less existing mature oak trees. Even if

the Planning Commission had jurisdiction over the "Tree Easement", that easement has no bearing on this application.)

12) A chart showing that our family's proposed visitation and marketing reductions including a new reduction from the levels considered by the Planning Commission on October 3, 2018. As explained in Rob Anglin's December 3rd letter, we have been communicating with neighbors since the October 3rd hearing. On November 13, we proposed a compromise of reduced visitation and marketing event numbers in response to neighbor concerns. Neighbors have not responded or provided any counter-proposal to the compromise that we proposed and discussed with the neighbors on November 13 and 14, 2018.

Regarding availability of off-site parking for our one remaining larger event (for 200 persons), we will lease offsite parking (if necessary) from one of the nearby facilities with parking lots and provide shuttle service to our guests. Nearby facilities with parking lots include: Justin-Siena High School (2.2 miles), Las Flores Community Center (1.6 mile), St. John's Lutheran School and Church (2.5 miles), Alston Park (1.1 mile), and Church of Latter Day Saints (1.4 mile).

Thank you for your diligence and ensuring that these items are part of the public record and provided to the Planning Commissioners.

Respectfully,

Julie Arbuckle

cc: Charlene Gallina (via email)

#4111010.0 January 11, 2019

Donald Barrella County of Napa PBES, Planning Division 1195 Third Street Napa, CA 94559

RE: Anthem Winery Planning Commission Hearing Public Comments

Dear Don:

Please find below our response to comments in your letters as listed below:

Paul K. Rowe – August 21, 2017 Letter

- Comment 1 The winery is also seeking to abandon its existing access from Redwood Road and to create new access from Dry Creek Road, utilizing a 20-foor wide flagpole portion of its property.
- Response 1 The existing access from Redwood Road will not be abandoned. It will remain, and will be used for vineyard, residential, and emergency access.
- Comment 2 The County must disregard the winery's option 1.
- Response 2 We agree that the Planning Commission should focus on Option 2. Option 1 will not be used unless an agreement can be made with Mr. Rowe. Option 1 is included in this Use Permit application to eliminate the need of an additional Use Permit Modification if such an arrangement is reached in the future.
- Comment 3 In requesting an exception to allow for the one-way bridge, the winery states that it will utilize the easement over my property to provide a 22-foot wide turnout on the downhill side of the bridge.
- Response 3 The Exception to Road and Street Standards for Existing Driveway Option 2, prepared by RSA⁺, states in the last paragraph of page 5, "The adjacent 40' easement (No. 1996-014263, N.C.R., and 1996-026341) would be used for all residential traffic and would provide an emergency pull-out area to allow passage of emergency vehicles."

In this case, an "emergency pull-out" is not the same as a standard 22-foot wide turnout. A Napa County standard turnout is constructed with a 25' taper on each end, to allow a vehicle to pull into the turnout, and continue forward out of the turnout. A pull-out area is any location, such as a shoulder or private driveway, where under emergency conditions a vehicle is physically able to pull off the road to allow passage of an emergency vehicle. This pull out will be used to support emergency response, not winery use.

- Comment 4
- The deed cited by the winery as the basis for the claimed width (1996-014263) was superseded by a subsequent amendment (1996-026341), which limits the easement for all purposes to conform to the driveway "as built" on the date of the amendment.
- Response 4

As stated above, Option 1 will not be used unless an agreement can be made with Mr. Rowe. Option 1 is included in this Use Permit application to eliminate the need of an additional Use Permit Modification if such an agreement is ever reached.

REAX Fire Safety Assessment – October 2, 2018

- Comment 1 The Anthem Winery marketing plan describes assembly occupancy characteristics as defined in California Building and Fire Codes wherein the potential for multiple fatalities and injuries from fire is comparatively high.
- Response 1 This is true for all wineries. This project is designed to conform to applicable building and fire codes for this use. Additionally, the project has been revised to remove the 300-person events, which were the largest category of events previously proposed.
- Comment 2 Proposed reductions in road access for firefighting has not been supported by a rational engineering analysis demonstrating equivalency to the intent of fire code requirements.
- Response 2 Section 3 of the Napa County Road and Street Standards outlines the strict requirements for exceptions to the standards. The Exception to Road and Street Standards for Existing Driveway, prepared by RSA+, meets these requirements, and is the result of four years of collaboration with Cal Fire and the Napa County PBES Engineering Division. The former Fire Marshal and current PBES Engineering Manager walked the driveway with the property owner and RSA+ as part of this process, to ensure that the proposed exception provides the same overall practical effect as these Standards towards providing defensible space, and consideration towards life, safety and public welfare.
- Comment 3 Anthem Winery is in an area of elevated wildland fire risk as demonstrated by historic fires and risk assessments from Cal Fire and the California State Public Utilities Commission.
- Response 3 Figure 2, included with the REAX report, shows the Anthem Winery parcel to be within the "Moderate" Fire Hazard Severity Zone. It is entirely outside the "High" and "Severe" Fire Hazard Severity Zones.

The Emergency Ingress/Egress Plan, prepared by RSA⁺, outlines measures incorporated into the design to mitigate for the "Moderate" Fire Hazard Severity.

Figures 5 and 6, included with the REAX report, shows the Anthem Winery parcel entirely outside the perimeter of the 2017 Napa Fire Complex, and all other recorded fire perimeters dating back to 1858. The proposed

improvements will present a substantial increase to emergency access, fire safety, and defensible space, over the existing condition.

Comment 4

The proposed winery use would shift occupant type from a few workers who are familiar with the site, operating largely during daylight hours, and possibly trained in emergency procedures, to large groups of 50 to 300 recreational visitors. These visitors would likely not be familiar with the site, egress routes, or emergency procedures. Groups indoors, in social situations, likely consuming alcohol, may delay egress for significant periods after becoming aware of an emergency, and might be making their escape after dark. The Building and Fire codes anticipate these higher risk elements associated with assemblies and provide extra levels of protection for occupants.

Response 4

As noted in the REAX report, the Building and Fire codes anticipate these higher risk elements associated with assemblies and provide extra levels of protection for occupants. These extra levels of protection are incorporated into the design of this project.

An emergency ingress / egress plan was created to provide permanent and operational control measures to support access for emergency wildland fire equipment, safe civilian evacuation, and to avoid delays in emergency equipment response to the site. The project has been revised to remove the 300-person events, which were the largest category of events previously proposed.

The on-site staff will be trained to direct evacuation in a safe and controlled manner, dependent on the emergency situation. This training will be specific to winery patrons, and on-site conditions.

The irrigated vineyard also provides a fire break from the dense shrubs and trees on the adjacent land to the north and west. The irrigated vineyard was identified in a previously approved exception request by Bartelt Engineering dated, April 2, 2001, as a "safe to stay" area in the event of a catastrophic fire.

Comment 5

The risk of fire at a working winery is likely higher than typical Assembly occupancies. That risk is magnified considerably by this winery's location in a recognized and historically higher fire threat area.

Response 5

Proposed improvements will follow applicable state codes and the Napa County Fire Marshall development guidelines for commercial projects. Additional fire flows and emergency water storage will be provided, in conformance with the California Fire Code, as amended by Napa County for wineries.

As noted earlier, the Anthem Winery parcel is entirely outside the "High" and "Severe" Fire Hazard Severity Zones.

- Comment 6
- Reduction in prescriptive requirements for access and egress have not seen quantitative description of the basis for prescriptive requirements or substantial rational analysis for alternate adoption. This is especially important when referencing local amendments that may be anticipating rural and agricultural uses more common in unincorporated areas.
- Response 6
- There are no established quantitative procedures referenced within the Napa County Road and Street Standards with which to qualify prescriptive requirements or exceptions thereto. Rather, proposed exceptions must meet the "same practical effect" as the standards. The proposed development has been found by Cal Fire and Napa County Engineering Division staff to meet this requirement.

Meese and Christensen - November 8, 2018 Letter

Comment 1 During the time we owned the property, we dug three wells. The first two were dry and the third well, within 30 feet of the shared property line with the Arbuckle's, initially produced 15 gallons per minute (GPM). The well's productivity fell to 4 GPM after the first year of use, in spite_ of the fact that we used the well for house consumption only. Dayna Manning has subsequently told me that the well's productivity has now dropped to less than 1 GPM.

Response 1 It is not possible to speculate as to the cause of Meese and Christensen's well problems without additional information regarding depth, testing, and metered use data. The extensive hydrogeologic study by Richard Slade & Associates shows that the Project Wells on the Anthem Winery parcels are capable of providing sufficient groundwater for the proposed development.

<u>Caloyannidis – October 3, 2018 Letter</u>

- Comment 1 The last winery audits we have, showed that 40% of Napa valley wineries were out of compliance.
- Response 1 RSA⁺ has been informed by the owners of Anthem Winery that it is currently in compliance with its Use Permit.
- Comment 2 In addition, this winery can only be approved if variance for 14 County Road

 Standards are granted in addition to one public encroachment permit.
- Response 2 The requested exceptions and associated mitigations have been found to meet the same practical effect as the Napa County Road and Street Standards by Cal Fire and Napa County PBES. As the two parcels are under a common ownership, the owners are effectively requesting a setback variance from themselves.

Brooks WAA Comments – September 2018

Comment 1 Assume 100% rainwater collection. Not supported by research, we modeled 90%.

Response 1 The USDA TR-55 standards for Urban Hydrology for Small Watersheds establishes a Curve Number of 98 for paved parking lots, roofs, driveways, and roads. Curve Numbers range from 0 to 100, and model the amount of runoff after losses, such as surface depressions, vegetation, evaporation, and infiltration. Notwithstanding the USDA TR-55 standard, the Tier 1 Water Use Calculations have been revised to assume a conservative 85% rainwater collection.

Comment 2 Assume 100% winery process collection and reuse. Not supported by research, we modeled 90%.

Response 2 There is no industry standard for estimating winery process wastewater loss.

Napa County code requires Process Wastewater treatment systems to be sized to treat 100% of the Process Water used.

Concrete crush pads with a dedicated drain system will provide similar collection characteristics to harvested rainwater. Losses due to evaporation will be negligible, considering that processing will occur in temperature- and humidity-controlled interior spaces. Captured Process Wastewater will also be supplemented by grape juice lost during the crush process. While we expect a much higher collection factor, we revised the Tier 1 Water Use Calculations to assume a conservative collection factor of 90%.

- Comment 3 Assumed Residential water at low end (.6 af/yr). We modeled .75 af as used by Anthem in Tier 1 calculations.
- The Napa County Water Availability Analysis Guidance Document estimates 0.5 to 0.75 af/year for a Primary Residence with minor to moderate landscaping (e.g. 1000 sf of lawn). The existing residences on parcels 1 and 2 have minor, drought-tolerant landscaping. They are each eligible to be modeled using the lower range of 0.5 af/yr, which is reflected in our revised Tier 1 Water Use Calculations.
- Comment 4 Sized storage tanks based on average rain year with evenly distributed rainfall.

 We sized tanks based on 2017/2018 average year with uneven rainfall.

- Response 4 The Napa County Water Availability Analysis Guidance Document address water use on an annual basis. As a conservative measure, we modeled rain water capture on a monthly basis, to account for monthly variations in average rainfall. This is standard industry practice to model water balances for ponds, tank capacity, and irrigation. Due to the year-to-year variability in precipitation, data for any one year cannot be used to characterize the future rainfall. An average distribution over many years must be used.
- Comment 5 Mixed rainwater capture of ground runoff with roof runoff. Anthem proposal calls out separating these streams. This impacts size of tanks and water treatment. We separated the systems and analyzed impact on storage.
- Response 5 "Treatment" of rainwater is a specific industry term to describe effective removal of pollutants from runoff. In this case, rainwater captured from the ground will be treated by passing through a bioretention planter, designed per Bay Area Stomwater Management Agencies Association (BASMAA) standards. Following treatment, ground runoff will be combined with roof runoff.
- Comment 6 Our model attempt to utilize reported real world Best Management Practices.
- Response 6 "Best Management Practices (BMP)" is a specific term used to describe structural or operational storm water pollution prevention controls. It is misused in this context, and is not defined, or even mentioned in Napa County's Water Availability Analysis Guidance Document.
- Comment 7 Assumed Wells at twice normal year flow rate during droughts-not always possible but left.
- Response 7 This is misleading. Wells were tested to sustainably flow at the Drought Year flow rate. The Normal Year flow rate is half of the sustainable Drought Year flow rate.
- Comment 8 At most use a 20% reduction in water use due to SDI installation rather than 40%.
- Response 8 Flowmeter data from a 2015 DRI vs. Non-DRI test showed bimonthly reductions between 41% 45% for underground water efficient irrigation. A 40% credit is conservative and appropriate in this case.
- Comment 9 Do not take a reduction in water use due to planting Sauvignon blanc, not supported by the literature, and but more importantly, not enforceable.

Response 9 The reduced water demand of Sauvignon Blanc is documented from personal experience by the project's vineyard manager and winemaker. It is also supported by the shorter irrigation season for this varietal. The draft conditions of approval limit total groundwater extraction onsite and require monitoring to ensure compliance. These enforceable conditions of approval will drive farming practices including varietal selection.

Please feel free to contact me with any questions.

| Sincerely, | OF PROFESSIONAL |
|--------------------|-----------------|
| | 77859 |
| Bruce Fenton, P.E. | * Exp. 06/30/19 |
| PW/kp | VIE CIVIL OR |

| | 2014 Application | 10/3/18 Application | Current Request |
|---------------------------|---|---|----------------------|
| Marketing Events | 48 30-person events | 24 30-person events | 22 30-person events |
| | 24 100-person events | 10 100-person events | 6 100-person events |
| | | | 2 50-person events |
| | 2 300-person events | 1 200-person event | 1 200-person event |
| | | 1 300-person event | No 300-person events |
| | | | |
| Total Annual Marketing | 4,440 | 2,220 | 1,560 |
| | | | |
| Event Hours | 11 am – 12 am with events over 30 guests moving indoors by 10 pm | 11 am – 12 am with events over 30 guests moving indoors by 10 pm | 11 am – 10 pm¹ |
| | / 3 3 3 | / 7 7 7 7 | (1) |
| Tastings | 40/day weekdays | 32/day weekday | 32/day weekday |
| | 60/day weekends | 48/day weekend | 48/day weekend |
| | 320/week | 256/week | 224/week |
| | | | |

¹ County staff and Commissioners already have signaled this change will be required.

Comparison to 30,000-50,000 Gallon Wineries Granted Visitation Since 1/1/09

| Winery | App Type | Tastings/ Week | Annual Event Guests | Production | Parcel Size/ Zoning |
|------------------------------|------------|-------------------|---------------------------|------------|------------------------|
| 40K – 50K Gallons | | | | | |
| Reynolds Winery | Major Mod | 280 | 1,906 | 40,000 | 13.5 acres/AW |
| Benessere | Major Mod | 300 | 1,760 | 44,000 | 42.6 acres/AP |
| B-Cellars Winery | Major Mod | 450 | 2,235 | 45,000 | 11.5 acres/AP |
| Ceja Vineyards | New Winery | 168 | 4,485 | 45,000 | 10.3/AW |
| Calistoga Artisan Village | New Winery | 240 | 384 | 48,000 | 22 acres/AP |
| Piazza Del Dotto | New Winery | 200 | 1,146 | 48,000 | 10.1 acres/AP |
| Vine Cliff Winery | Major Mod | 350 | 2,516 | 48,000 | 99.6 acres/AW |
| Titus Vineyards | Major Mod | 350 | 1,700 | 48,000 | 32 acres/AP |
| Outpost | Major Mod | 180 | 325 | 50,000 | 37.6 acres/AP |
| Robert Keenan | Major Mod | 245 | 1,050 | 50,000 | 147.4 acres/AW |
| Regusci | Major Mod | 400 | 1,450 | 50,000 | 162.6 acres/AP |
| Woolls Ranch | New Winery | 350 | 4,640 | 50,000 | 236 acres/AW |
| Cairdean Winery | New Winery | 175 | 1,400 | 50,000 | 50.3 acres/AW |
| Refuge | New Winery | 868 | 3,370 | 50,000 | 13 acres/AP |
| Wheeler Farms | New Winery | 224 | 1,452 | 50,000 | 11.7 acres/AP |
| Robert Foley | Major Mod | 60 | 244 | 50,000 | 13 acres/AW |
| Gamble Family | New Winery | 300 | 1410 | 50,000 | 11.2 acres/AP |
| Average 40K-50K | - | 302 | 1851 | - | - |
| Median 40K-50K | - | 280 | 1452 | - | - |
| Anthem Revised | Major Mod | 224 | 1,560 | 50,000 | 44.7 acres/AW |
| Proposal | | | | | |
| 35K - 45K Gallons | | | | | |
| Hartwell | Major Mod | 120 | 465 | 36,000 | 30 acres/AP |
| Reynolds Winery | Major Mod | 280 | 1,906 | 40,000 | 13.5 acres/AW |
| Benessere | Major Mod | 300 | 1,760 | 44,000 | 42.6 acres/AP |
| B-Cellars Winery | Major Mod | 450 | 2,235 | 45,000 | 11.5 acres/AP |
| Ceja Vineyards | New Winery | 168 | 4,485 | 45,000 | 10.3 acres/AW |
| Average 35K-45K | - | 264 | 2,170 | - | - |
| Median 35-45K | - | 280 | 1,906 | - | - |
| Anthem Revised Proposal | Major Mod | 224 | 1,560 | 50,000 | 44.7 acres/AW |

| 30K - 40K Gallons | | | | | |
|-------------------|------------------|-----|-------|--------|---------------|
| Catellucci | New Winery | 210 | 830 | 30,000 | 19.3 acres/AP |
| Goosecross | Minor Mod | 350 | 710 | 30,000 | 11.3 acres/AP |
| Trefethen H&L | New Winery | 140 | 400 | 30,000 | 41.2 acres/AP |
| Beautiful Day | New Winery | 385 | 1,300 | 30,000 | 29 acres/AP |
| Chateau Lane | New Winery | 147 | 270 | 30,000 | 11 acres AW |
| Diogenes Ridge | New Winery | 90 | 1,020 | 30,000 | 13 acres AW |
| Eagle Eye | New Winery | 112 | 1,452 | 30,000 | 13 acres AW |
| Hyde | New Winery | 120 | 400 | 30,000 | 12 acres AW |
| Ideology | New Winery | 105 | 240 | 30,000 | 10 acres AP |
| Joseph Cellars | New Winery | 525 | 4,560 | 30,000 | 26 acres AW |
| Mahoney Vineyards | New Winery | 84 | 450 | 30,000 | 10 acres AW |
| Rogers | New Winery | 120 | 300 | 30,000 | 53 acres AW |
| Sleeping Giant | New Winery | 85 | 300 | 30,000 | 11 acres AW |
| Sleeping Lady | New Winery | 140 | 450 | 30,000 | 104 acres AP |
| Wallis Family | New Winery | 108 | 225 | 30,000 | 16.8 acres AW |
| Young Inglewood | New Winery | 112 | 1745 | 30,000 | 16 acres AP |
| Yountville | New Winery | 175 | 400 | 30,000 | 11 acres AP |
| Washington St. | | | | | |
| Hartwell | Major Mod | 120 | 465 | 36,000 | 30 acres/AP |
| Reynolds Winery | Major Mod | 280 | 1,906 | 40,000 | 13.5 acres/AW |
| Average 30K-40K | - | 179 | 917 | - | - |
| Median 30K-40K | - | 120 | 450 | - | - |
| Anthem Revised | Major Mod | 224 | 1,560 | 50,000 | 44.7 acres/AW |
| Proposal | | | | | |
| | | | | | |



11/1/18

Re: Anthem Winery Driveway

To whom it may concern,

Per your request, please find the attached description on how we are planning to construct the driveway leading to Anthem Winery located at 3123 Dry Creek Road in Napa.

Our logistical plan to construct this roadway between two property lines with a twenty-foot clearance and stay within the property owned by Anthem Winery are as follows,

- Install silt fencing along property lines adjacent to road construction that occurs on flat or down slope topography. This fencing is a precaution to keep excess material within property boundaries. Where the road is constructed with an elevated topography a shotcrete flash coat could be installed to stabilize slope.
- Use an articulating vertical auger to drill piers for retaining wall structural supports. This equipment access will be from the existing 3123 Dry Creek Road driveway.
- All retaining wall constructing will be within property lines of Anthem Winery.
- All road bases section and asphalt will be installed between twenty-foot property section owned by Anthem Winery.

Respectfully,

Rod Field

Ledcor Builders Inc.

Order No: 111947 11/10/95-am-4

When recorded mail to:

JAMES D. DALY SHARON E. DALY 6918 Harmon Drive Ventura, CA 93003-7146 RECORD COUNTY CO

1995 028957
OFFICIAL RECORDS OF NAPA COUNTY
H. KATHLEEN BONDS

AT REQUEST OF: FIRST AMERICAN TITLE 12/27/1995 08:00 AM Fee: \$ 48.00 Pgs: 8 TT: \$.00

For Recorder's Use Only

MAIL TAX STATEMENTS TO:

SAME AS ABOVE

THE UNDERSIGNED GRANTOR DECLARES DOCUMENTARY TRANSFER TAX \$ 0.00

Computed on the consideration or value of property conveyed; OR

Computed on the consideration or value less liens or encumbrances remaining at time of sale.

LOT LINE ADJUSTMENT

APN PTN. 035-460-025

CORPORATION GRANT DEED

FOR A VALUABLE CONSIDERATION, receipt of which is hereby acknowledged,

FIRST AMERICAN TITLE COMPANY OF NAPA

a corporation organized under the laws of the State of California, does hereby

GRANT to

Signature

JAMES D. DALY and SHARON E. DALY, husband and wife, as Community Property

the real property in the County of Napa, State of California, described as

LEGAL DESCRIPTION ATTACHED HERETO AND MADE A PART HEREOF AND DESIGNATED EXHIBIT "A"

The purpose of this conveyance and the conveyances being recorded concurrently herewith is to create a Lot Line Adjustment pursuant to California Government Code Section 66412(d) and local subdivision ordinances.

| Dated: 12-26-95 | FIRST AMERICAN TITLE COMPANY OF NAPA |
|---|--------------------------------------|
| STATE OF CALIFORNIA }ss. COUNTY OF NAPA } | |
| On 12-26-95 1995, before me, D. R. Tate | PAUL DURBIN, Vice President |
| personally appeared PAUL DURBIN and LARRY FRATTINI, | TAGE DORDIN, VICE HOSIGAI |
| personally known to me (or proved to me on the basis of satisfactory evidence) to be the persons whose names are subscribed to the within | |
| instrument and acknowledged to me that they executed the same in their authorized capacities and that by their signatures on the | Bur Dur North |
| instrument the persons, or the entity upon behalf of which the | ARRY FRATTINI, Vice President |
| persons acted, executed the instrument. | D.R. TATE Z |
| WITNESS my hand and official seal. | COMM. #993716 COMM TO THE CALFORNIA |

MAIL TAX STATEMENTS AS DIRECTED ABOVE

My Comm. Expires JULY 27, 1997

Form NAP1115SP

FATCO/DALY 11/10/95-am-4

EXHIBIT "A"

BEGINNING on the line between the old Jackson Gridley tract of land and the tract formerly owned by E. C. Webber, North 67° 42' 30" East, 132.00 feet from the Northwest corner of the last named tract; thence North 23° 09' 26" West, 719.96 feet; thence North 67° 30' 00" East 1634.65 feet to the Westerly line of Dry Creek Road; thence North 27° 50' 49" West 20.09 feet to a rebar monument; thence South 67° 30' 00" West, 1633.01 feet to a rebar monument on the West bank of a small creek; thence South 67° 30' 00" West, 801.43 feet to an oak tree, 24 inches in diameter at an angle in the fence; thence South 67° 19' 02" West 277.17 feet; thence North 72° 59' 42" West 1028.6 feet, more or less, to the point of intersection with the Easterly line of the tract of land now or formerly owned by Mrs. W.B. Pieratt, said point of intersection bears South 44° 45' East 367.00 feet from the most Northern corner of that certain tract of land described as Exhibit "A" in the Amended Certificate of Compliance document recorded May 29, 1990 in Book 1742 at page 207 of Official Records of Napa County; thence along the Northerly and Easterly lines of the tract of land conveyed to William West by Deed recorded in Book 2 at page 337 of Official Records of Napa County, South 44° 45' East 491 feet; thence North 87° 00' East, 250.14 feet; thence South 28° 00' East, 49.50 feet; thence South 62° 45' East 117.48 feet; thence South 56° 00' East, 105.60 feet; thence South 9° 00' East, 84.48 feet; thence North 88° 30' East, 149.16 feet; thence South 75° 00' East 79.2 feet, more or less, to the center of the main branch of Napa Creek; thence down the middle of said creek to the Northwest corner of the tract conveyed to Stanley E. Wood by Deed of record in Book 725 at page 234 of Official Records of Napa County; thence following Wood's line, North 67° 42' 30" East, 1056 feet, more or less, to the point of beginning.

The consolidation of underlying lots, parcels or portions thereof as set forth in the above metes and bounds description, constitutes an expressed written statement of the Grantor, merging said underlying lots, parcels or portions thereof pursuant to Section 1093 of the California Civil Code.

FATCO/DALY

This conveyance is subject to the following covenants restricting the use of the foregoing described real property. These covenants are for the benefit and protection of the real property described in <u>Exhibit B</u> to this deed (the "Benefitted Property"), all of which covenants shall run with the land and shall be enforceable by the record owner of the Benefitted Property and all transferees, assigns and successors in right, title or interest in the Benefitted Property against grantee in this deed and all transferees, assigns and successors in right, title or interest to the property described in <u>Exhibit A</u> to this deed (the "Burdened Property"):

- 1. No building or other structures, including but not limited to patios, decks, tennis courts or pools shall be constructed or maintained within that portion of the Burdened Property described in <u>Exhibit C</u> attached hereto (the "Tree Easement Area"), except that there shall be permitted the construction of a residence which partially intrudes into said Tree Easement Area foregoing as shown on the attached plat.
- 2. None of the existing mature oak trees located within the Tree Easement Area shall be removed or excessively cut or pruned unless any such tree dies or becomes incurably diseased or dying.
- 3. The general intent and purpose of the foregoing restrictive covenants is to reasonably maintain the native forest and meadow conditions within the Tree Easement Area for the protection and enhancement of the value and amenity of the Benefitted Property and for the scenic enjoyment of its owners. These covenants may be expanded, restricted, removed or modified by the execution and recording in the records of the County of Napa by the record owners of the Benefitted Property and Burdened Property of a document which accomplishes such modification.
- 4. Any breach of these covenants and restrictions shall entitle the owner of the Benefitted Property to reasonable damages and equitable enforcement as determined by a court of law. The prevailing party in any legal action to enforce these covenants and restrictions or to recover damages for any breach thereof shall be awarded their reasonable attorney's fees and costs incurred, with the reasonable amount thereof to be fixed by the court, arbitrator or entity rendering a judgment, award, order or determination.

Acknowledgement and Acceptance of Grantee:

 $\sim QD$.

HAPONE DAIY

STATE OF CALIFORNIA)

COUNTY OF Ventura) 85.

on <u>Joseph 8</u> 1995, before me, <u>Joseph A.</u> Butte, <u>Joseph A.</u> personally appeared JAMES D. DALY and SHARON E. DALY, personally known to me (or proved to me on the basis of satisfactory evidence) to be the person(s) whose name(s) is are subscribed to the within instrument and acknowledged to me that he/she(they executed the same in his/hea/their authorized capacity(ies) and that by his/hea/their signature(s) on the instrument the person(s), or the entity upon behalf of which the person(s) acted, executed the instrument.

WITNESS my hand and official seal.

Signature <u>| Oeula (4</u> Notary Public Benita A. Colitti Z Comm #1001007 OTARY PULLIC CALIFORNIA VENTURA COUNTY Comm Expires Sept 24, 1997

EXHIBIT "B"

(Adjusted Lands of Harms)

PARCEL ONE:

Commencing at a point where the Westerly line of Dry Creek Road crosses a small creek, said point being on the Southeasterly line of the 487 acre tract conveyed to Jacob R. McCombs by Deed recorded April 13, 1852 in Book B of Deeds at page 156, said Napa County Records; running thence South 23° 30' East along the Westerly line of said road 225 feet, more or less, to the most Northern corner of the 0.94 acre parcel of land described in the Deed to Margaret Hartson recorded August 27, 1959 in Book 597 at page 925 of Official Records of Napa County; running thence South 67° 30' West along the Northwestern boundary of said Hartson's parcel, 205 feet to the most Western corner thereof; thence along the Southwestern boundary of said Hartson's parcel South 22° 30' East 150 feet, more or less, to the most Northern corner of the parcel of land described in the Deed to Dean A. Faria, recorded July 31, 1962 in Book 656 at page 804 of Official Records of Napa County; running thence along the Northwestern boundary of said Faria's parcel, South 67° 30' West 1100 feet to the most Western corner thereof; running thence along the Southwestern boundary of said Faria's parcel, South 23° 30' East 450 feet to the most Southern corner thereof, being a point on the Northern line of the 33.30 acre tract firstly described in the Deed to Minnie M. West, recorded September 26, 1905 in Book 84 of Deeds at page 104, said Napa County Records; thence along said Northern line South 67° 30' West 1140 feet, more or less, to an oak tree, 24 inches in diameter at an angle in the fence; thence South 67° 19' 02" West 277.17 feet; thence North 72° 59' 42" West 1028.6 feet, more or less, to the Easterly line of the 240 acre tract conveyed to Archie P. Pieratt by Deed recorded April 4, 1923 in Book 138 of Deeds at page 399, said Napa County Records; thence along the Easterly line of said 240 acre tract North 44° 45' West 367.00 feet to the most Northern comer of that certain tract of land described as Exhibit "A" in the Amended Certificate of Compliance document recorded May 29, 1990 in Book 1742 at page 207 of Official Records of Napa County; thence continuing along said Easterly line of Pieratt, North 25° 27' West 198 feet, more or less, to the Southerly line of the 487 acre tract above referred to; thence along the Southerly line of said 487 acre tract South 85° 30' East 2,206.38 feet to a post marked 21; thence continuing South 85° 30' East 66 feet, more or less, to the middle of the small creek above referred to, said creek being the Southerly boundary of said 487 acre tract; thence along the middle of said creek and following the meanderings thereof to the point of commencement.

EXCEPTING THEREFROM, that portion thereof conveyed to Stuart A. Mott, et ux, by Deed recorded May 31, 1967 in Book 766 at page 700 of Official Records of Napa County.

ALSO EXCEPTING THEREFROM that portion conveyed to the County of Napa by Deed recorded August 23, 1967 in Book 771 at page 572 of Official Records of Napa County.

ALSO EXCEPTING THEREFROM that portion described in the document to the County of Napa, recorded February 6, 1860 in Book F of Deeds at page 84, Napa County Records.

PARCEL TWO:

Commencing at the point formed by the intersection of the existing Westerly line and the proposed Northeasterly line of the County Road known as "Dry Creek Road" said existing Westerly line also being the Easterly line of that tract of land described in the Deed recorded in Book 762 at page 393 of Official Records of Napa County; thence from said point of commencement, North 23° 30' 00" West along said existing Westerly line 151.39 feet to the Northerly corner of said tract of land; thence North 73° 36' 52" East along the Easterly extension of the division line between said tract of land and that tract of land described in Deed recorded in Book 349 at page 250 of Official Records of Napa County, 20.15 feet to the point of intersection with the existing centerline of said County Road; thence South 23° 30' 00" East along said existing centerline 209.66 feet to the point of intersection with the proposed Northeasterly line of said County Road; thence North 41° 42' 56" West along said proposed Northeasterly line 63.98 feet to the point of commencement, and being a portion of the Napa Rancho.

PARCEL THREE:

A non-exclusive Easement for the purpose of construction, installation, operation, maintaining and repairing a reservoir, dam and appurtenances together with the right of ingress and egress thereto over the following described parcel of land:

COMMENCING at a rebar and cap stamped LS 4366 at the most Westerly corner of the Lands of Leonard Russell, as shown on the map filed in Book 23 of Surveys at page 19 in the office of the County Recorder of said Napa County; thence from said point of commencement North 67° 30' 00" East 86.00 feet; thence South 20° 03' 10" East 260.80 feet; thence South 38° 00' 00" West 80.00 feet to the Southwesterly line of the aforementioned lands of Leonard Russell; thence North 23° 30' 00" West 300.00 feet to the point of commencement.

EXHIBIT "C"

DESCRIPTION Tree Easement Area

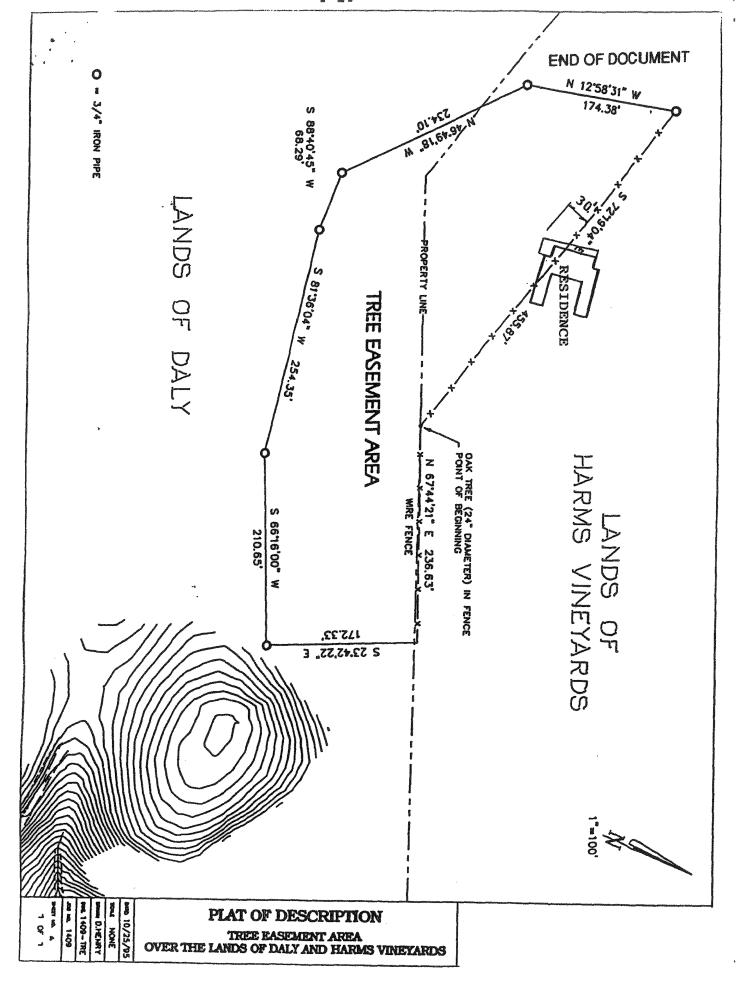
All that real property situate in the County of Napa, State of California, more particularly described as follows:

BEGINNING at an oak tree 24 inches in diameter in a fence marking an angle point in the Northern line of that certain tract of land described as Exhibit "A" in the Amended Certificate of Compliance document recorded May 29, 1990 in Book 1742 at page 207 of Official Records of Napa County; and running thence along an existing wire fence line, North 67° 44' 21" East 236.63 feet; thence leaving said fence, South 23° 42' 22" East 172.33 feet; thence South 66° 16' 01" West 210.65 feet; thence South 81° 36' 04" West 254.35 feet; thence South 88° 40' 45" West 68.30 feet; thence North 46° 49' 18" West 234.10 feet; thence North 12° 58' 31" West 174.38 feet to the intersection with said wire fence; thence running along said fence, South 72° 19' 04" East 455.87 feet to the point of beginning.

EXCEPTING FROM the above described area that portion described as follows:

A strip of land, 20 feet in width, measured at right angles, the Easterly line of which is that certain course described above as "South 23° 42' 22" East 172.33 feet".

á



-



Water System Feasibility Study for a Regulated System

For:

Anthem Winery Napa, CA

APN 035-470-046

Prepared for:

Julie Arbuckle 3454 Redwood Rd Napa, CA 94558



Project #4111010.0

October 30, 2015

Revised: March 18, 2019



Table of Contents

| l. | TECHNICAL CAPACITY | 1 |
|----|--------------------|---|
| П | CONSOLIDATION | 2 |
| | MANAGERIAL | |
| | FINANCIAI | |



I. TECHNICAL CAPACITY

System Description

The proposed Anthem Winery is located at 3454 Redwood Road, Napa. Existing wells on site will serve the proposed 50,000 gallon per year winery, existing dwellings and vineyard.

The proposed winery development will require the establishment of a Transient-Noncommunity water system. The consolidation of this project into an existing public water system has been investigated and no existing system within 3 miles of the Anthem Winery is willing to provide water service to the winery.

There are several wells on the parcel, however, only Well 8 will provide water to the winery public water system which will include the domestic use within the winery. The other wells will be used for supplementary irrigation and process water. The 2015 well completion report for Well 8 shows 50-ft annular seal of bentonite. No chemical or biological treatment will be performed on the well water unless quarterly testing results deem further treatment is necessary. Water for the Public Water System will be stored in a proposed */- 10,000 gallon tank. Separate tanks will be installed for firewater, irrigation and other winery use. Separate pumps will supply the domestic water, irrigation water, fire water, and other winery water. See the Use Permit-Utility Plan for system layout.

Twenty-Year Evaluation of Projected Water Demand

Based on the Tier 1 Water Use Calculations, the annual public water demand (employees, visitors, and events) is 0.29 acre feet per year (94,500 gallons per year). The daily average public water demand is 259 gallons per day. Peak daily public water demand is estimated at 518 gallons per day, being 200% of average daily demand.

If the Winery seeks expansion in the future, thereby increasing the water demand on the public water system, the Winery will need to acquire a use permit modification and prove that increased capacity is available. It will not be permissible for future developments in the vicinity of this project to join this public water system without first justifying that the water supply is available to meet the demand.

Additional non-public water demand for the site includes winery process water, landscape and vineyard irrigation, and existing non-project residential uses. The proposed total water use for the two parcels is 6.72 acre feet per year. Of this, 0.69 acre feet per year will be provided by reclaimed process wastewater, and an average of 1.32 acre feet per year will be provided by harvested rainwater. The total groundwater use for both parcels (public and non-public) is 4.71 acre feet per year.

As noted in the Tier 1 Water Use Calculations, non-project wells (1, 5, 4, and 7) will continue to produce at their current rate of 3.79 acre feet per year. The remaining 0.92 acre-feet per year



(300,000 gallons per year) will be provided by the project wells (3, 6, and 8). The total daily average demand on project wells, public and non-public, is 821 gallons per day. Sufficient storage will be provided on site to mitigate peaking effects and allow for a constant 821 gallons per day demand on project wells.

Twenty-Year Evaluation of Water Supply Capacity

Well 8 is capable of supporting the proposed public water system peak daily groundwater demand of 518 gal/day. Page 24 of the Anthem Winery WAA Memo DRAFT 4-10-17 shows the capacity of Well 8 to be 1 to 2 gpm. When pumped on a 50% operational basis at 1 gpm (pumping 12 hours per day), the daily project well yield is 720 gallons per day. This exceeds the daily demand on Well 8.

1.0 gpm*720 min/ day = 720 gal / day 720 gal / day \geq 518 gallons (peak public water system demand)

The existing water source (project wells) is capable of supporting the proposed total daily groundwater demand of 821 gal/day. Recommendation 5 of the Anthem Winery WAA Memo DRAFT 4-10-17 shows the total capacity of project wells to be 2.5 gpm. When pumped on a 50% operational basis (pumping 12 hours per day), the daily project well yield is 1,800 gallons per day. This exceeds the daily demand on project wells.

2.5 gpm*720 min/ day = 1,800 gal/ day1,800 $gal/ day \ge 821 gallons$ (peak total water demand)

Source Adequacy

Well 8 has a 50-ft annular seal of Bentonite to comply with Napa County Code 13.12.380 as a Class IA wells for a Public Water System. The Application and Permit to Construct a Water Well document outlines the well construction and inspection by the Department of Environmental Management. Application and Permit are on file at Napa County.

Water Quality

Water sampling will be conducted prior to operation of the system. Water quality is expected to meet or exceed all requirements of Chapter 15 of Title 22, California Code of Regulations (CCR).

II. CONSOLIDATION

An investigation of the adjacent Public Water Systems within 3 miles of the project has been performed using the map viewer provided on the California Environmental Health Tracking Program website. The only public water system found within 3 miles of the proposed winery is the City of Napa Community Water System. An Outside Water Service Application was submitted to the City of Napa and the request to connect was denied. Without the possibility of connection, the estimated cost of connection was not investigated.



III. MANAGERIAL

General

The owner of the water system will be the property owner of the parcel. The costs of operation will be covered in the winery operation costs. The owner will also hold the responsibility of water system manager for the property.

Operation and Maintenance

The following is a summary of the required Operations and Maintenance schedule:

| Tasks | Frequency | Action |
|-----------------------------------|-----------|---|
| System Water Level | Daily | Visual Inspection |
| System Pressure and Conveyance | Daily | Visual Inspection |
| Water Tanks | Quarterly | Visual Inspection |
| Manually Operate Valves and Pumps | Quarterly | Operation |
| Water Quality Test & Reporting | Quarterly | Unit Samples Taken & Reported to Napa Co. |

A certified distribution operator or treatment operator (T1 level or above) as specified by Chapter 13 of Title 22 CCR contracted by the owner will be responsible for system repairs.

Monitoring and Testing

Water quality testing will be conducted to comply with Chapter 15 of Title 22 of CCR. Samples will be taken to Caltest or approved laboratory for testing.

IV. FINANCIAL

Below is a brief summary of the system's annual estimated financial capacity. Capital improvement costs and installation of the treatment and distribution systems, are estimated to be a one-time expense of \$50,000, amortized over 20 years.

Capital Improvements: \$2,500

Power: \$2,000

Maintenance: \$3,500

Water Quality Testing: \$1,500

Total: \$9,500

Projected Annual Gross Revenue: \$10,504,000 (Based on 21,008 cases at \$500/case)

Annual Operating Costs: \$8,403,200 (at 20% profit)

Percent of Total Operating Costs: 0.1%



WINERY WASTEWATER FEASIBILITY REPORT

For:

Anthem Winery Napa, CA

APN 039-610-006

Prepared for:

Julie Arbuckle 3454 Redwood Rd Napa, CA 94558

Project# 4111010.0

January 9, 2019





TABLE OF CONTENTS

| INTRODUCTION | 1 |
|---|---|
| | |
| EXISTING SEPTIC SYSTEM | 1 |
| SITE EVALUATION | 1 |
| DOMESTIC WASTEWATER CHARACTERISTICS | 2 |
| WINERY PROCESS WASTEWATER - SURFACE DRIP IRRIGATION | 3 |
| DOMESTIC WASTEWATER - SUB SURFACE DRIP | 4 |
| STORMWATER DIVERSION | 5 |
| OPERATION AND MAINTENANCE | 5 |
| CONCLUSION | |

APPENDICES

- 1. Vicinity Map & USGS Site Map
- 2. Reduced Use Permit Plan Set
- 3. Existing Septic System Documentation
- 4. Site Evaluation
- 5. Water Balance for Irrigation and Storage



INTRODUCTION

The Anthem Winery project is pursuing a Major Modification (MM) of an existing Use Permit to build a larger winery facility including a tasting room, fermentation buildings, offices, and wine caves. All proposed winery facilities will be located on the southern parcel APN 035-470-046 of two adjacent parcels, with winery and visitor access coming through the northern parcel APN 035-460-038. The proposed winery will have seven full-time, and five part-time employees.

The property varies in slope from 1-21%. The properties are currently used as a rural residence on the 035-460-038 parcel and the other is currently a winery. Redwood Creek runs roughly north to south on the western side of the property. Two wells exist on the site. One near the water tank along the existing northern property line. The other is located just south of the existing barrel storage cave. Appendix 1 contains a Site Location Map and a USGS Site Map showing the parcel topography, features and boundary. Appendix 2 contains a reduced version of the proposed winery plan set.

This report will evaluate the disposal of wastewater consisting of winery process wastewater, and winery domestic wastewater.

EXISTING SEPTIC SYSTEM

Information from Napa County files for the parcel shows an existing septic system for the house and winery. The winery system consists of a standard system that has two 1200 gallon septic tanks that feed into an 810 gallon pump tank before being pumped to 1,400 linear feet of line. The residential system information only showed approximate location of existing system.

The existing winery distribution lines are located southwest of the existing winery barrel cave. This area will be impacted by the proposed winery improvements. It is proposed that the existing drain field be abandoned.

SITE EVALUATION

RSA⁺ conducted a site evaluation on the subject parcel on June 20, 2014. Appendix 4 contains a map of test pit locations and test pit logs for the site evaluation. The site evaluation was conducted by Brett Frasier of RSA⁺ and observed by Kim Withrow of Napa County Environmental Management.

WINERY PROCESS WASTEWATER CHARACTERISTICS

The following is a summary of the winery wastewater characteristics:

Wine Production: 50,000 gallons of wine per year

2.38 gallons of wine per case

21,008 cases/year



Wastewater Production: 5 gallons of wastewater/gallon of wine

250,000 gallons/year

Peak Daily Waste Water Flow: Crush Period = 60 days

Annual wine production x 1.5 / 60

1,250 gallons/day

Average Daily Flow: 250,000/365 = 685 gallons/day

Monthly Wastewater Flows:

Table 1

| | % By Month | Waste/Month | |
|--------|------------|-------------|-----------|
| Sep | 15% | 37,500 | Gal/Month |
| Oct | 15% | 37,500 | Gal/Month |
| Nov | 11% | 26,250 | Gal/Month |
| Dec | 8% | 18,750 | Gal/Month |
| Jan | 4% | 10,000 | Gal/Month |
| Feb | 6% | 15,000 | Gal/Month |
| Mar | 6% | 15,000 | Gal/Month |
| Apr | 5% | 11,250 | Gal/Month |
| May | 6% | 15,000 | Gal/Month |
| Jun | 7% | 17,500 | Gal/Month |
| Jul | 9% | 21,250 | Gal/Month |
| Aug | 10% | 25,000 | Gal/Month |
| Totals | 100% | 250,000 | Gal/Year |

DOMESTIC WASTEWATER CHARACTERISTICS

The winery domestic wastewater system has been sized to accommodate the unit values in Table 2 below. The number of visitors and employees is based on information provided by the owner. The projected flow is based on Napa County Environmental Management guidelines. The following is a summary of the estimated flows from the proposed winery.

Table 2

| Use | Source | Number | Projected Flow (gpd) | Total Flow No Event | Total Flow Event Day |
|------------------|----------------------------------|--------|-------------------------|------------------------|-------------------------|
| | | | 110W (gpa) | Day (gpd) | (gpd) |
| | Full-time employees | 7 | 15 | 105 | 105 |
| ¥ | Harvest empolyees | 5 | 15 | 75 | 75 |
| WINERY | Visitors | 48 | 3 | 144 | 144 |
| | Private Event w/ meals (catered) | 100 | 10 | 0 | 1000 |
| | Event Staff | 5 | 15 | 0 | 75 |
| Winery Subtotals | | | | 324 | 1399 |
| Grand Total | | | Total | | |
| | | | Peak | 324 | 1399 |
| | | | Flow | | |



Events with 30 or less guests will be on-site catered and events with more than 30 guests will be off-site catered. The number of visitors is based on a <u>maximum</u> expected daily visitor count. For events with more than 100 persons portable sanitation facilities will be provided.

WINERY PROCESS WASTEWATER - SURFACE DRIP IRRIGATION

According to Napa County Environmental Management Sewage Treatment System Design Guidelines, winery process wastewater must be treated prior to surface discharge. Based on our experience, winery wastewater characteristics are as follows:

| Characteristics | Units | Average |
|-----------------|-------|---------|
| рН | | 3.5 |
| BOD5 | mg/l | 6000 |
| TSS | mg/l | 500 |
| Nitrogen | mg/l | 20 |
| Phosphorus | mg/l | 10 |

The treatment goal is 160 mg/l BOD and 80 mg/l TSS. To meet this treatment goal a treatment train including a septic tank, treatment tank with High Strength Membrane Bio-Reactor (HSMBR) unit, and pump tank are proposed. This treatment train may be modified for more desirable treatment processes prior to submitting construction plans. The following sections describe this process in more detail. This system is shown on Sheet UP3 contained in Appendix 2.

Septic Tank

The septic tank will serve to buffer peak flows and strengths from overwhelming the system and impairing treatment. This tank has been designed with baffles near the outlet. This tank will provide three days storage and will also serve to function as a primary settling basin. This tank will be 4,000 gallons.

Treatment Tank

The treatment tank will serve to treat wastewater flows using a High Strength Membrane Bio-Reactor (HSMBR) unit. This tank will provide ten days storage. This tank will be 13,000 gallons.

Pump Tank

The pump tank will serve to hold wastewater prior to distribution to the dispersal field. This tank will house dual pumps. This tank will be 1,000 gallons.

Holding Tank and Dispersal Field

To provide a preliminary estimate of the amount of storage tanks required, we have prepared a monthly water balance, as shown in Appendix 7. Monthly wastewater production is based on a percentage of the total annual wastewater production. The amount of water allowed to be applied is estimated by the typical vine water demand. The irrigation will be applied to areas of vineyards outside well setback requirements. The area available for irrigation is shown in Appendix 6. An area of 6.0 acres of vineyard and 0.5 acres of cover crop has been used to



calculate the storage capacity required. Based on monthly analysis no storage is required. Storage capacity of 20,000 gallons is provided for treated process wastewater generated during wet weather periods.

During the summer months all of the treated wastewater will be used for irrigation. During the wet winter months, a limited discharge will be consistent with landscape water demand and no discharge will occur within 48-hours of a forecasted rain event and also for 48-hours after a rain event. These irrigation scheduling constraints necessitate installing tanks to store excess water that cannot be discharged during the winter months. All stored water will then be used for irrigation during the summer months.

DOMESTIC WASTEWATER - SUB SURFACE DRIP

For the domestic wastewater we propose installation of a new septic system and dispersal field for the proposed winery. For the winery, the addition of a HOOT treatment system and a new dispersal field is proposed.

Domestic wastewater from the winery will flow into a 5,000 gallon septic tank before flowing into two new HOOT H-1000 tanks. After pretreatment in the HOOT H-1000, wastewater will be pumped to the proposed distribution field.

The subsurface drip field is sized to meet Napa County Environmental Management guidelines. The distribution field will be placed in the area of the site evaluation where the most limiting usable soil type was clay loam. The allowable application rate for clay loam is 0.6 gallons/square foot/day for pre-treated effluent. Peak daily domestic wastewater flow is 1399 gallons/day.

Dispersal Field Area(primary) =
$$\frac{1399 \, gpd}{0.6 \, gpd \, / \, SF}$$
 = 2,332 square feet

In addition to the primary dispersal area of 2,332 square feet, a 200% reserve area is required. The reserve area will be located adjacent to the primary field where the soil application rate is also 0.6 gallons/square foot/day.

Dispersal Field Area (reserve area) =
$$\frac{1399 gpd}{0.6 gpd/SF}$$
 = 2,332 square feet

The total requirement for domestic wastewater reserve dispersal area is 4,664 square feet. Total area required for the primary and reserve is 6,996 square feet.

The system layout is shown on the Use Permit Plans in Appendix 2.



STORMWATER DIVERSION

Operational areas including crush pad, trash and recycling enclosure, and mechanical pad will be covered.

OPERATION AND MAINTENANCE

The winery process and domestic waste systems will be fully automated and has been designed so minimal input from winery staff is required. Per Napa County guidelines, a Registered Civil Engineer, Registered Environmental Health Specialist, or Licensed Contractor will provide semi-annual monitoring and evaluation of the system. The contract with the responsible party will be provided prior to the final inspection for the system installed.

CONCLUSION

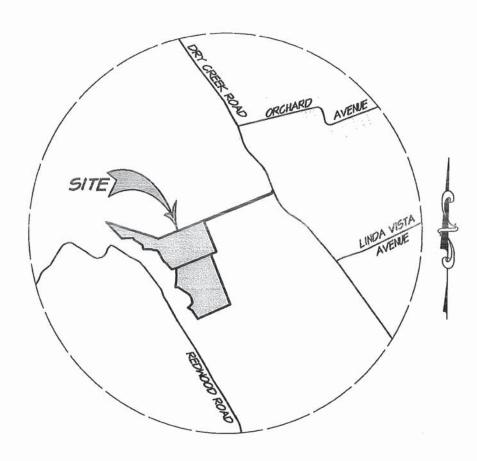
This report demonstrates that enough dispersion area is available making a sub-surface drip system a feasible option for treating the Anthem Winery's domestic wastewater. It has also been demonstrated that it is feasible to treat the winery process wastewater and distribute this to the vineyard using drip irrigation.

The above methodology results in a design that meets the Napa County Environmental Management Design standards for the treatment of winery and domestic wastewater.



Vicinity Map & USGS Map

ARBUCKLE RESIDENCE VICINITY MAP



SCALE: I" = 2000'



S 1515 Fourth Street Napa, Calif. 94559 v 707.252.3301 f 707.252.4966

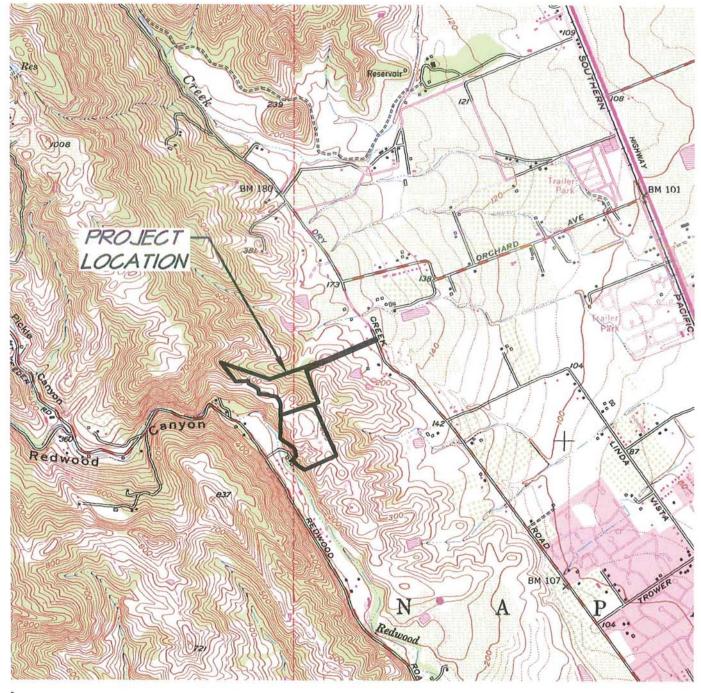
JULY 22, 2014

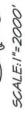
4111010.0 Exh-Pitmap.dwg 1 OF 3

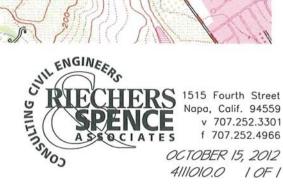
ARBUCKLE VINEYARDS USGS QUAD MAP

NAPA

CALIFORNIA

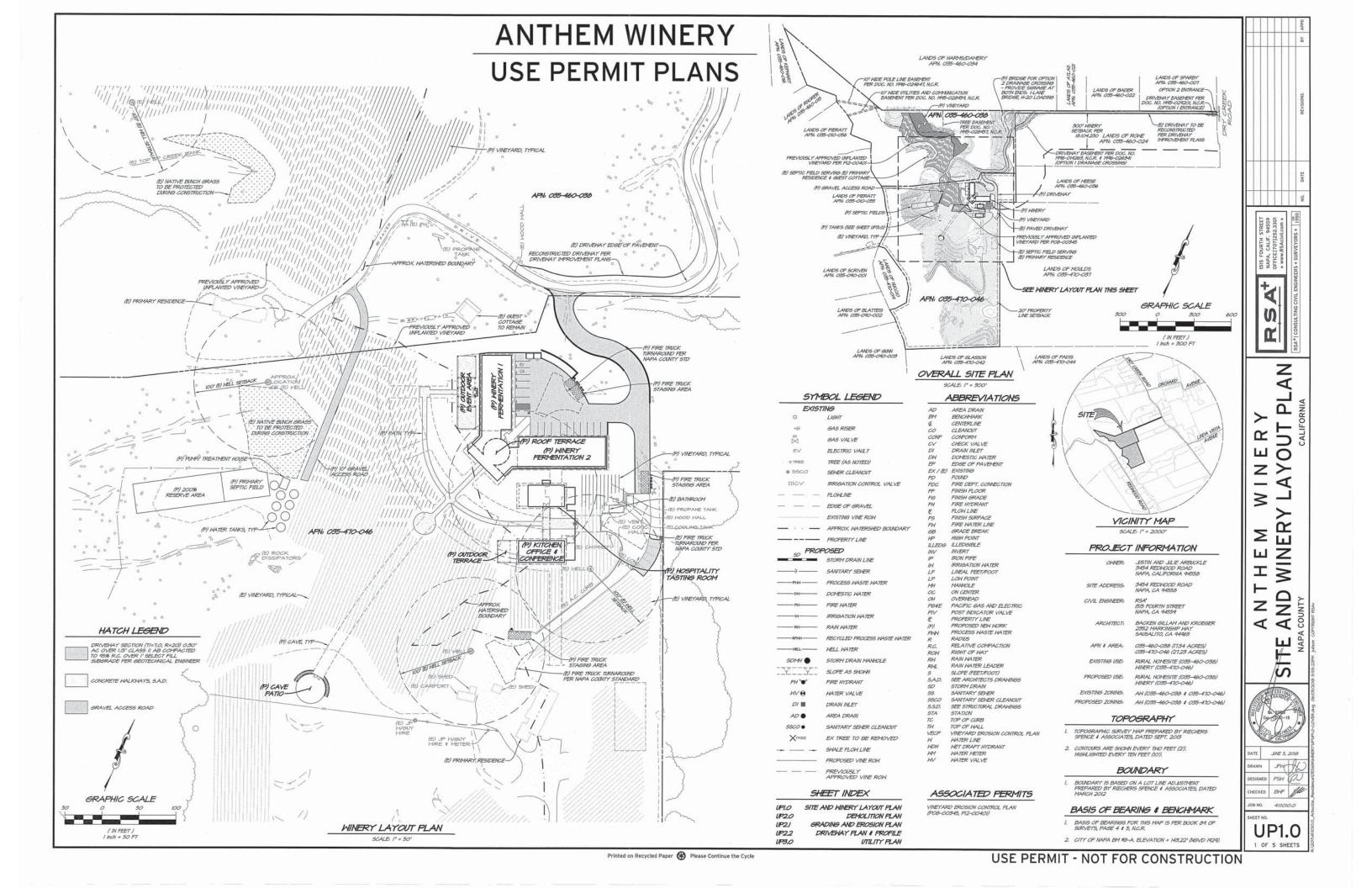


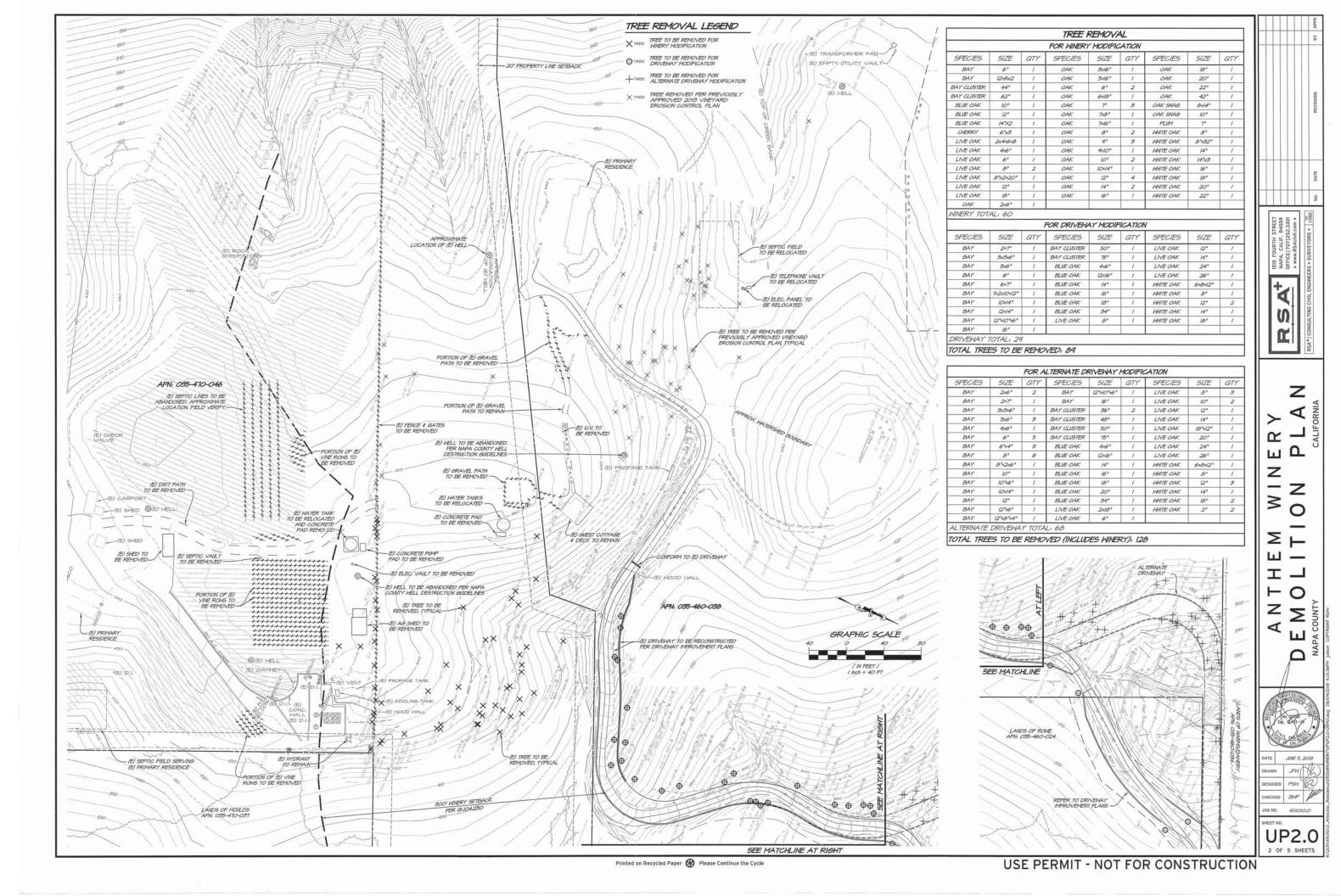


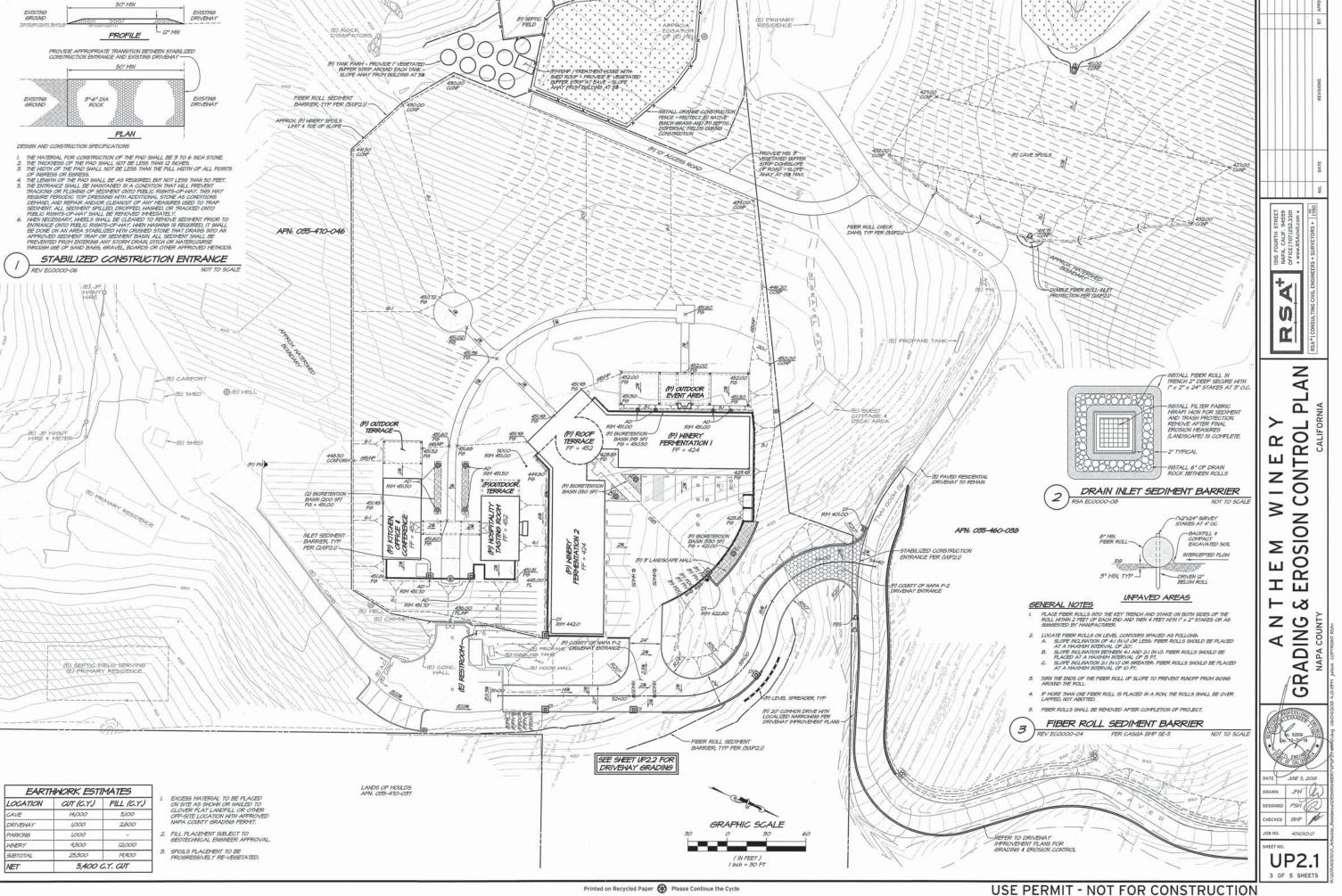


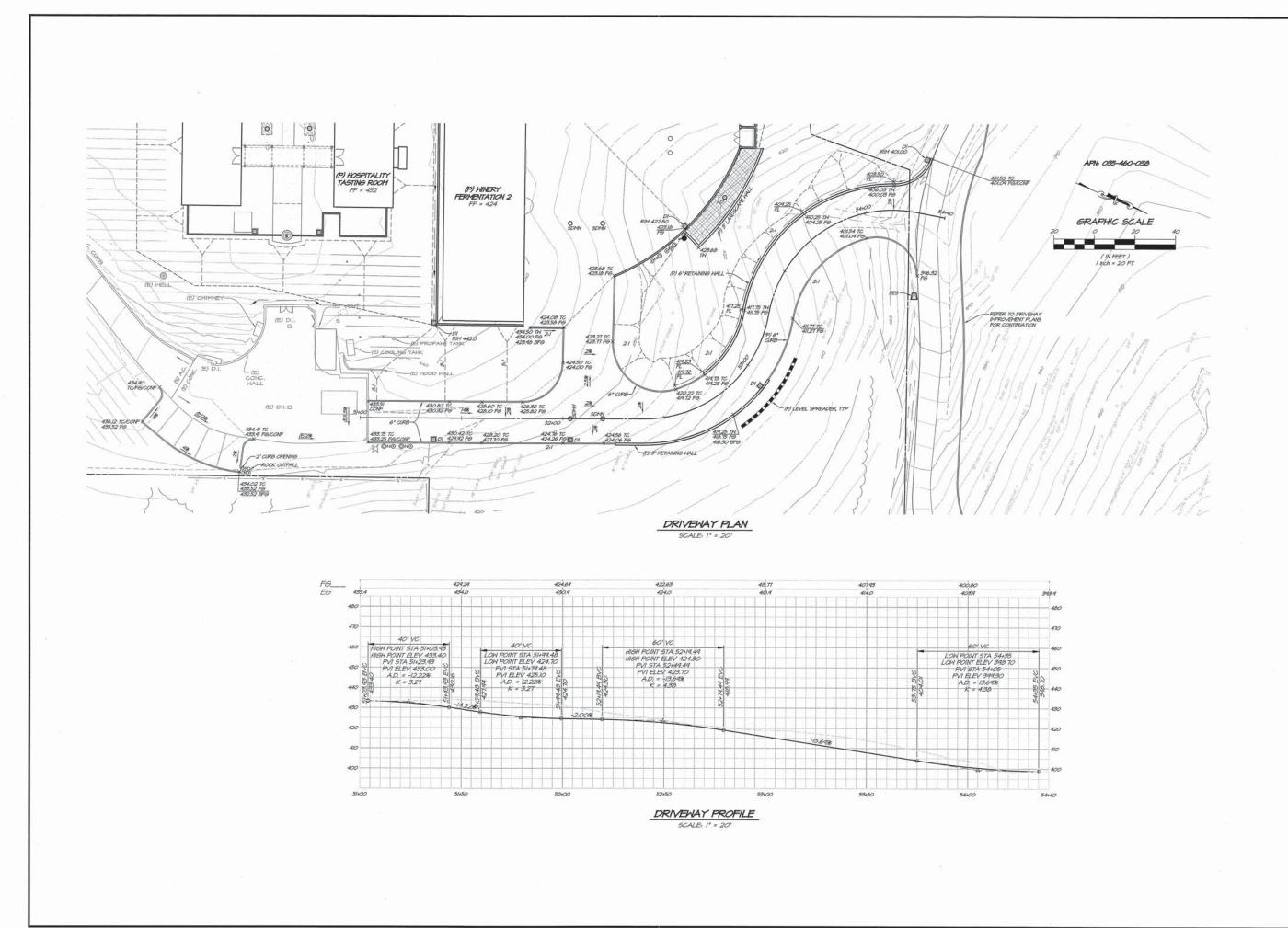


Reduced Use Permit Plan Set

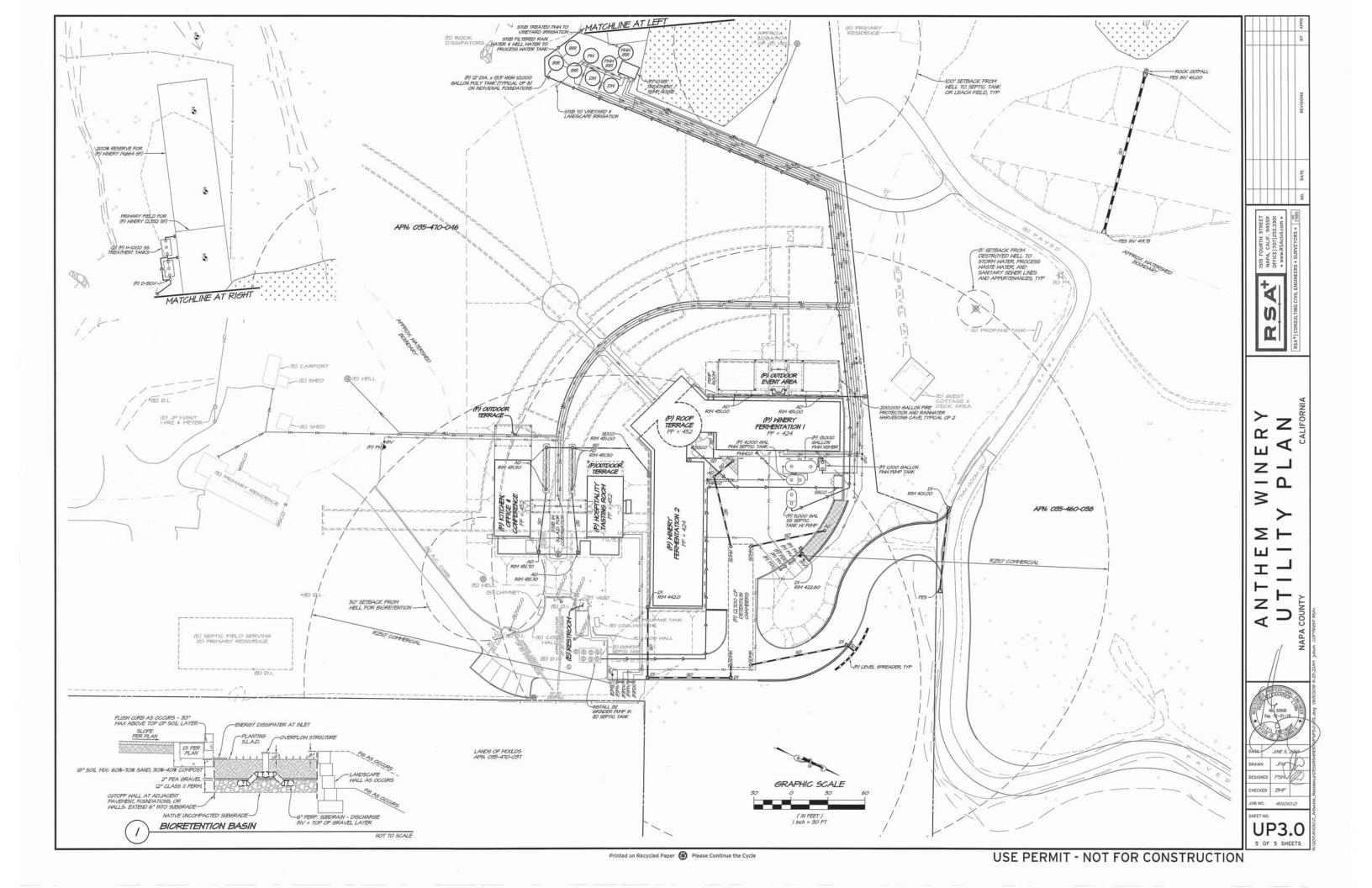






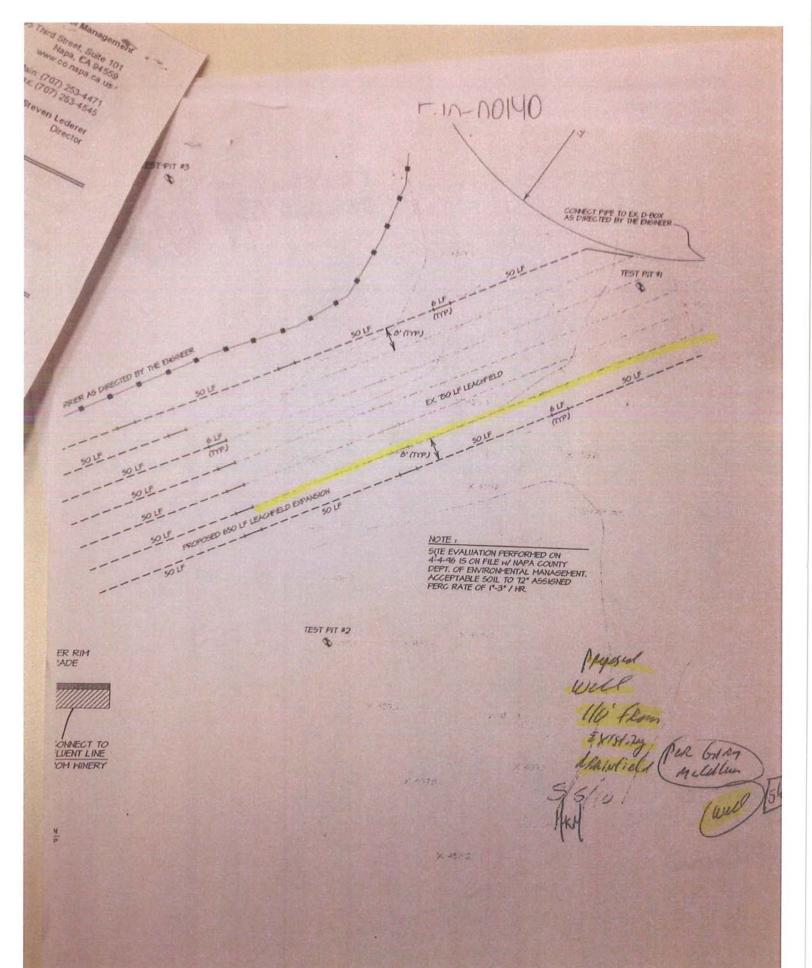


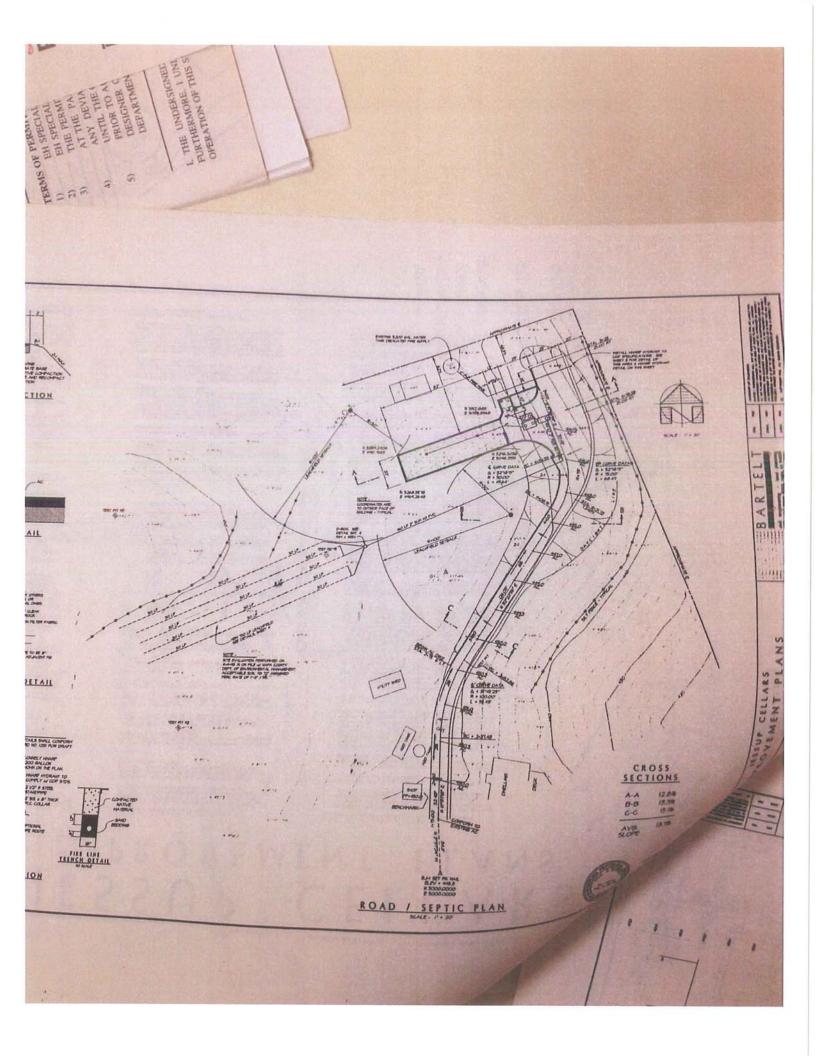
1515 FOURTH STREET
NAPA, CALIF. 94559
OFFICE|707|252.3301
+ www.RSAcivil.com + + ด 4 ш RY ROFILI 피즈 Z Ø ≥ Z Ø Ы Σ ш I RIVEWA Z V $\overline{\Box}$ HECKED BWF MC JOB NO. 4///0/0.0 **UP2.2** 4 OF 5 SHEETS

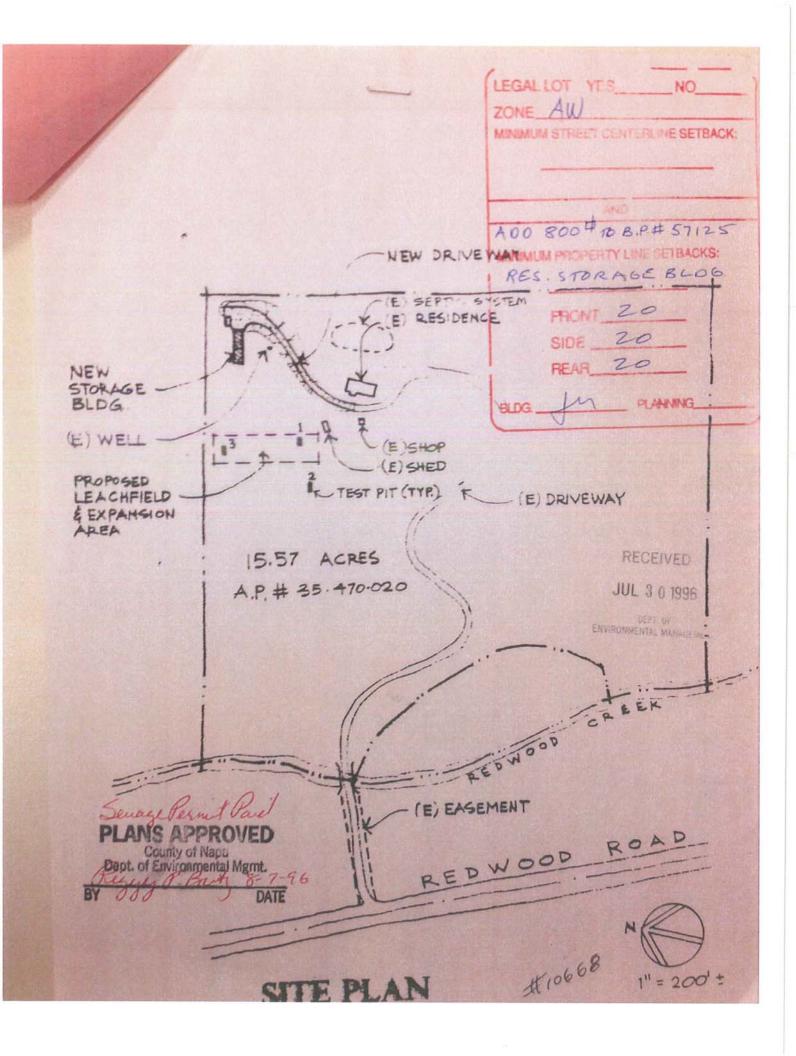


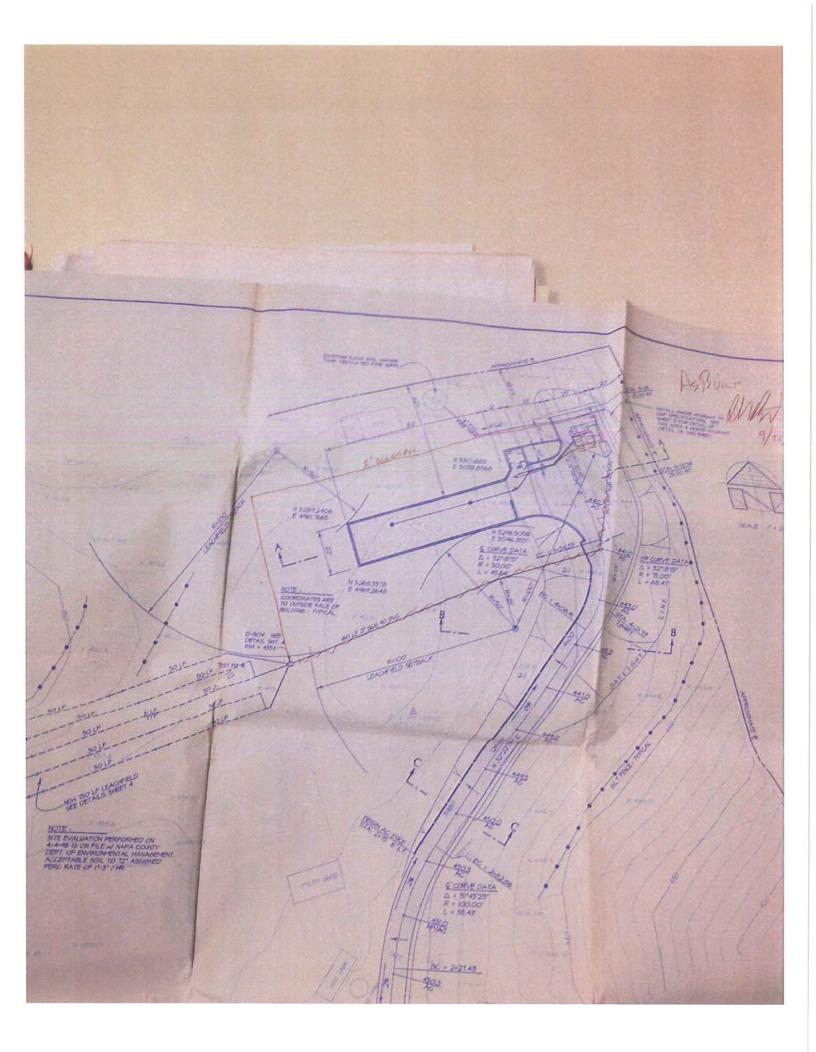


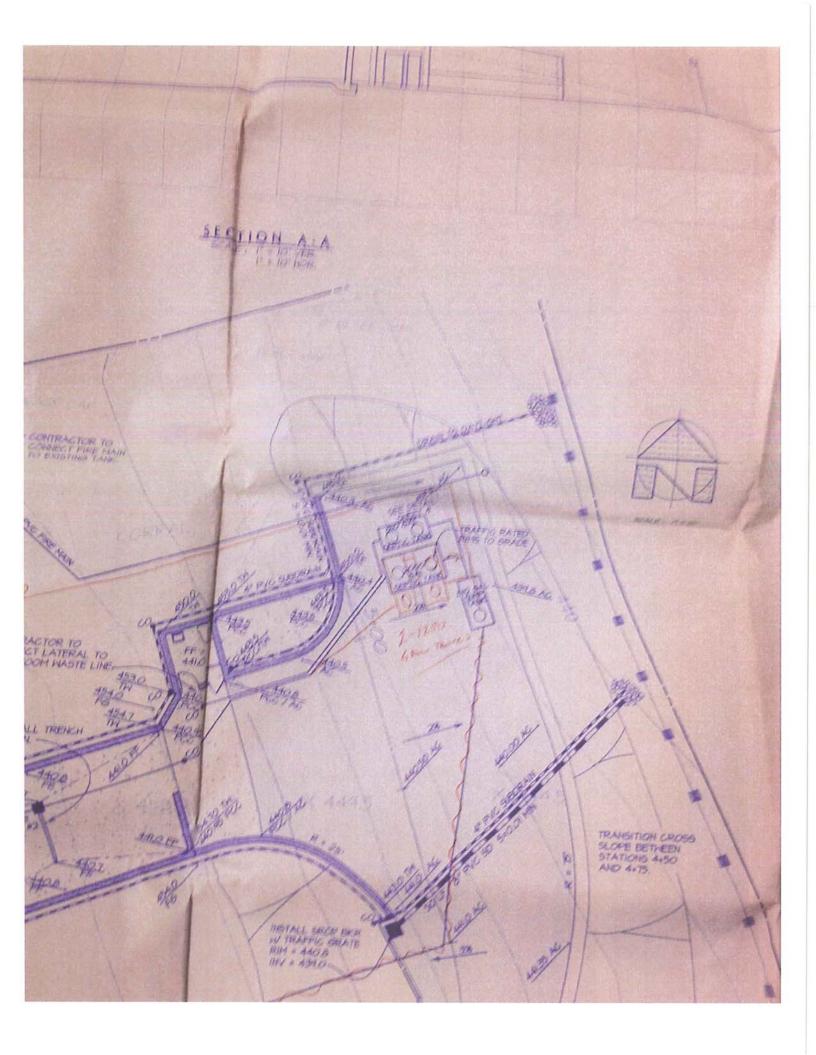
Existing Septic System Documentation













Site Evaluation

Permit Number: E14-00484

APN 035-470-046

RSA Project Number: #4111010.0

Date: 06/23/2014 Page 1 of 3

Napa County Department of Environmental Management SITE EVALUATION REPORT

Please attach an 8.5" x 11" plot map showing the locations of all test pits triangulated from permanent landmarks or known property corners. The map must be drawn to scale and include a North arrow, surrounding geographic and topographic features, direction and % slope, distance to drainages, water bodies, potential areas for flooding, unstable landforms,

existing or proposed roads, structures, utilities, domestic water supplies,

Permit #: E14-00484

APN: 035-470-046

(County Use Only)
Reviewed by: Date:

PLEASE PRINT OR TYPE ALL INFORMATION

wells, ponds, existing wastewater treatment systems and facilities.

| Property Owner | | *** | Т | | | | | | | |
|---|---------------------|----------------------------------|---|----------------------------------|-------------|--------------|--------------------------|--|--|--|
| , , | | | ☐ New Construction | on 🛛 Addition | □ F | Remodel | Relocation | | | |
| Julie Arbuckle | | | Other: | | | | | | | |
| Property Owner Mailing Address 3454 Redwood Road | | | Residential - # of Bedrooms: Design Flow: gpd | | | | | | | |
| | State Zi | | | | | | | | | |
| | CA 94 | .558 | │ ☑ Commercial – 1 | ype: Winery | | | | | | |
| Site Address/Location | | | Sanitary Waste: | 1435 gpd | Proc | ess Waste | e: gpd | | | |
| Same | | | Other: | | | | | | | |
| | | | Sanitary Waste gpd | : gpd | F | rocess W | aste: | | | |
| Evaluation Conducted By: | | | | | | | | | | |
| Company Name RSA+ | Evaluat Brett Fr | or's Name asier | | Signature (Civil I | Engineer, F | LE.H.S., Geo | ologist, Soil Scientist) | | | |
| Mailing Address: 1515 Fourth Street | | Telephone Number 707-252-3301 | | | | | | | | |
| City Napa | | State Zip CA 945 | | Date Evaluation June 20, 2014 | n Condu | cted | | | | |
| Primary Area | | | Expansion Are | <u>a</u> | | | - | | | |
| Acceptable Soil Depth: 40 in. Tes | t pit #'s: 1-4 | | Acceptable Soil Dep | oth: 40 in. Te | st pit #'s | : 1-4 | | | | |
| Soil Application Rate (gal. /sq. ft. /day |): 0.6 | | Soil Application Rate | ə (gal. /sq. ft. /da | y): 0.6 | | | | | |
| System Type(s) Recommended: Sub | -surface drip | | System Type(s) Red | commended: Sul | b-surfac | e drip | | | | |
| Slope: 15% Distance to nearest wa | ater source: 28 | 30 ft. | Slope: 15% Distar | nce to nearest wa | iter sour | ce: 280 ft | i. | | | |
| Hydrometer test performed? | No⊠ Yes [| (attach results) | Hydrometer test per | formed? | No 🛛 | Yes 🗌 | (attach results) | | | |
| Bulk Density test performed? | No⊠ Yes I | ☐ (attach results) | Bulk Density test pe | rformed? | No 🛛 | Yes 🗌 | (attach results) | | | |
| Percolation test performed? | No⊠ Yes I |] (attach results) | Percolation test perf | ormed? | No 🛛 | Yes 🗌 | (attach results) | | | |
| Groundwater Monitoring Performed? | No 🛛 Yes | attach results) | Groundwater Monito | oring Performed? | No 🛛 | Yes 🗌 | (attach results) | | | |
| Site constraints/Recommendations: | | | 1 | | | | | | | |
| | | | | | | | | | | |
| | | | | | | | | | | |
| | | | | | | | | | | |

Permit Number: E14-00484

APN 035-470-046

RSA Project Number: #4111010.0

•

Test Pit#

1

| | | | | | . | C | onsisten | ce | | | |
|----------------------------|------------------------------|----------|-------|---------|---------------------------------|--------------|----------|-----|-----------------------|-----------------------|---------------------------------------|
| X = Limiting Horizon | Horizon Depth (Inches) | Boundary | %Rock | Texture | Structure (Grade / Shape) | Side Wall | Ped | Wet | Pores (QTY / Size) | Roots (QTY / Size) | Mottling (QTY / Size/ Contrast) |
| | 0-40" | С | <30% | CL | S/SB | н | FRB | s | C/F-M | F/F | N/A |
| | 40"-54" | Bottom | <30% | | | | | | | | Yes |
| | | | | | | | | | | | |
| | | | | | | | | | | | |
| | | | | | | | | | | | |
| Notes: | | | | | | | | | | | |

Date: 06/23/2014

Page 2 of 3

Test Pit #

2

| V - | | D | 0/51 | - 1 | a | C | onsisten | ce | _ | | |
|----------------------------|------------------|--------|---------------------------------|--------------|------|-----|-----------------------|-----------------------|---------------------------------------|-------|-----|
| X = Limiting Horizon | ing Depth (Grade | | Structure (Grade / Shape) | Side Wall | Ped | Wet | Pores (QTY / Size) | Roots (QTY / Size) | Mottling (QTY / Size/ Contrast) | | |
| | 0-40" | С | <40% | CL | M/SB | Н | FRB | s | F/F | C/F-C | N/A |
| | 40"-53" | Bottom | ~50% | | - | | | | | | |
| | | | | | | | | | | | |
| | | | | | | | | | | | |
| Notes: | | | | I | I | | | | | | |

Test Pit #

3

| ν - | | Danisalasa | n/ D 1- | | 04. | С | onsisten | се | _ | | |
|----------------------------|------------------------|------------|---------|---------|---------------------------------|--------------|----------|-----|-----------------------|-----------------------|---------------------------------------|
| X = Limiting Horizon | Horizon Depth (Inches) | Boundary | %Rock | Texture | Structure (Grade / Shape) | Side Wall | Ped | Wet | Pores (QTY / Size) | Roots (QTY / Size) | Mottling (QTY / Size/ Contrast) |
| | 0-54" | Bottom | <30% | CL | S/SB | SH | FRB | S | C/F-M | C/F-C | N/A |
| | | | | | | | | | | | |
| | | | | | | | | | | | |
| | | | | | | | | | | | |
| 1-1- | | | | | | | | | | | |
| | | | | | | | | | | | |
| Notes: | <u> </u> | 1 | 1 | | ı | | I | | | | |

Permit Number: E14-00484

APN 035-470-046

RSA Project Number: #4111010.0

Date: 06/23/2014 Page 3 of 3

Test Pit#

4

| X = | Horizon | Roundon | 9/ Book | Tavetresa | 04 | Consistence | | се | _ | | |
|---------------------|-------------------|----------|---------|-----------|---------------------------------|--------------|-----|-----|-----------------------|-----------------------|---------------------------------------|
| Limiting Horizon | Depth (Inches) | Boundary | %Rock | Texture | Structure (Grade / Shape) | Side Wall | Ped | Wet | Pores (QTY / Size) | Roots (QTY / Size) | Mottling (QTY / Size/ Contrast) |
| · | 0-48" | Bottom | <35% | CL | M/SB | Н | FRB | S | M/F-M | M/F-C | N/A |
| | | | | | | | | | | | *** |
| | | | | | | | | | | | |
| | | | | | | ***** | | W | | | |
| *** | | | | | | **** | | | | | |
| | | | | | | | | : | | | |
| Notes: | ····· | | | | | | | | | | |

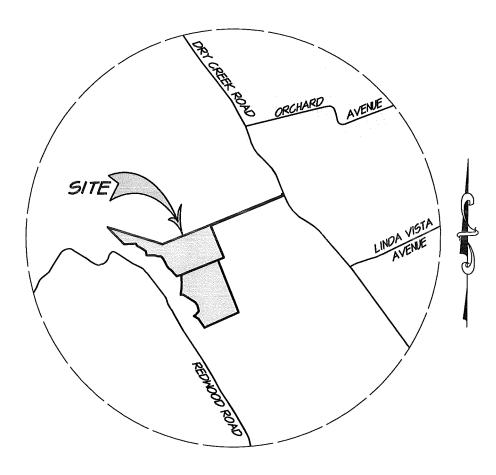
Test Pit #

| X = | Horizon | orizon Boundary %R | 0/ Dook | %Rock Texture | Stanton | С | onsisten | ce | _ | | |
|---------------------|-------------------|--------------------|---------|---------------|---------------------------------|--------------|----------|-----|-----------------------|-----------------------|---------------------------------------|
| Limiting Horizon | Depth (Inches) | Боинцагу | 76ROCK | Texture | Structure (Grade / Shape) | Side Wall | Ped | Wet | Pores (QTY / Size) | Roots (QTY / Size) | Mottling (QTY / Size/ Contrast) |
| | | | | 41 | | | | | | | |
| | | | | | | | | | | | |
| | | | | | | | | | | | |
| | | | | | | | | | | | |
| | | | | | | | | | | | |
| | | | | | | | | | | | |
| | | | | | | | | | | | |
| Notes: | | | | | | | | | | | |

Test Pit # 6

| X = | Horizon | Boundami | 0/Deels | T | 01 1 | С | onsisten | ce | _ | | |
|---------------------|-------------------|----------|---------|--------------|---------------------------------|--------------|----------|-----|-----------------------|-----------------------|---------------------------------------|
| Limiting Horizon | Depth (Inches) | Boundary | %Rock | Texture + | Structure (Grade / Shape) | Side Wall | Ped | Wet | Pores (QTY / Size) | Roots (QTY / Size) | Mottling (QTY / Size/ Contrast) |
| | | | | | | | | | | | |
| | | | | | | , | | | | | |
| | | | i | | | | | | ! | | |
| | | | | | | | | | | | |
| | | | | | | | | | | | |
| | | | | | | | | | | | |
| | | | | | | | | | | | |
| | | | | | • | | | | | | |
| Notes: | | | | | | | | | | | |

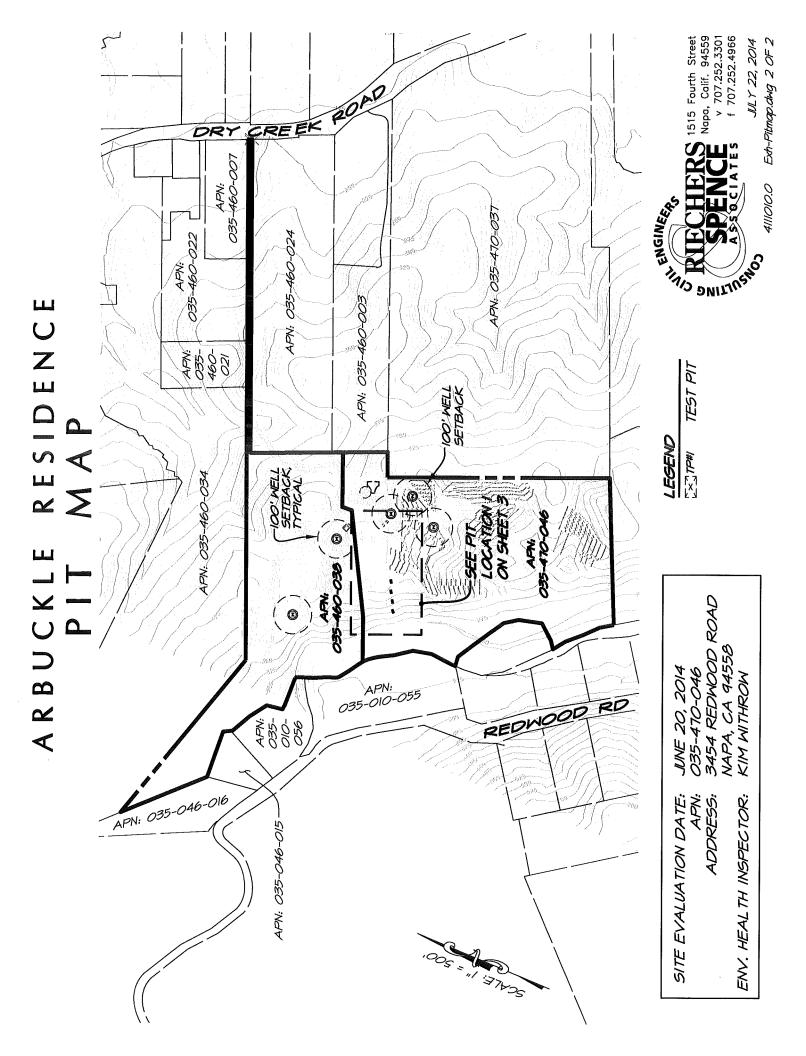
ARBUCKLE RESIDENCE VICINITY MAP

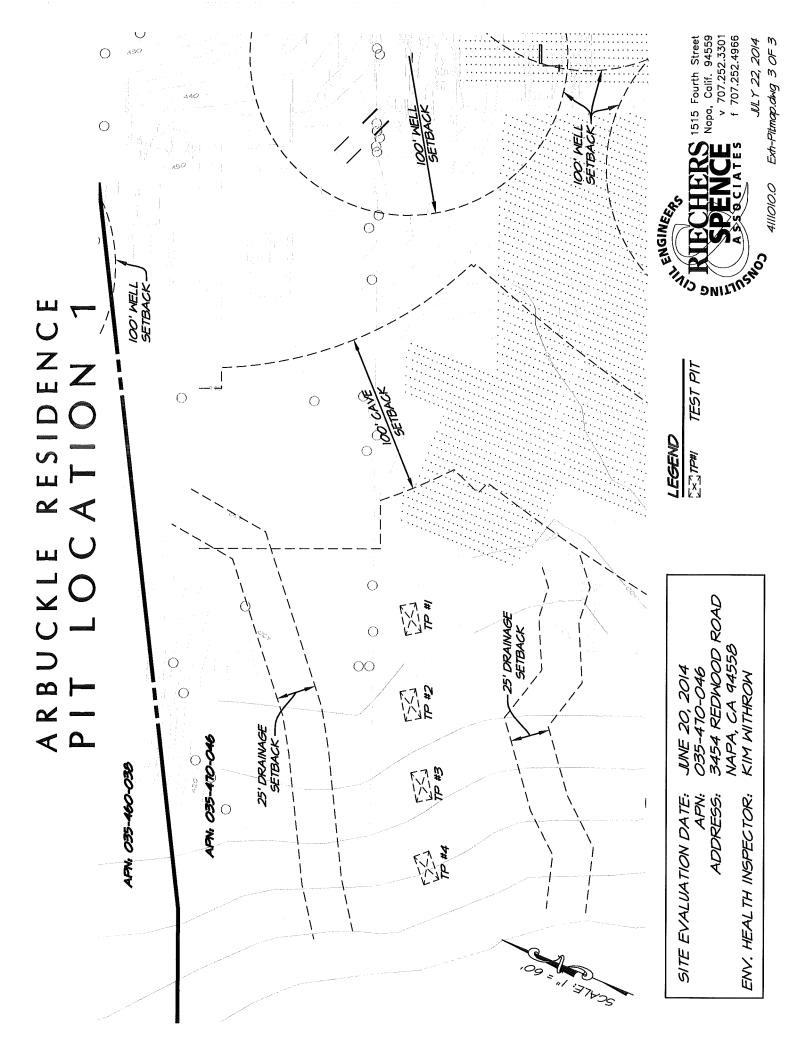


SCALE: I" = 2000'



1515 Fourth Street Napa, Calif. 94559 v 707.252.3301 f 707.252.4966







Water Balance for Irrigation and Storage

Reclaimed Process Wastewater Water Balance for Irrigation and Storage

| Page | Project Description | * | | | | | | | Annual Process Waste Flow Volume | | | | | | | | |
|--|---|-----------------------------|----------------|----------------|---------------|-----------------|----------------|--------------|----------------------------------|---------|---------|------------|----------|------------|--|--|--|
| Mathematical Process Mathematical Proces | | | | | | Wine Produc | tion: | | | | 50,000 | | gal/year | | | | |
| New Proper Normal Proper Norm | - | | | | | Annual Proce | ss Waste per | Gallon Wine: | | | 5 | | gal/year | | | | |
| Note the property of the prop | | | | | | | | | | | 250,000 | | | | | | |
| Note the property of the prop | Vinevard Irrigation Parameters | | Landscan | e Irrigatio | on Param | eters | | | | | | | | | | | |
| Note that the field and the f | | 6.00 acres | • | | <u> </u> | | ve grass and t | rees | | | | | | | | | |
| Marked protection of the part of the par | | | Total irrigate | d acres of cro | p: | | 0.50 | acres | | | | | | | | | |
| Marie dea anathyling from the 1512 flag at | | | | | | | | | | | | | | | | | |
| Manufairy Process Washesware Generation | | | | | | | | | | | | | | | | | |
| Monthly process wareware generated as 6 animal relation 46 | | 121,346 gal | | | | | | | | | | | | | | | |
| Monthly process wareware generated as 6 animal relation 46 | Monthly Process Wastewater Generati | on | | | | | | | | | | | | | | | |
| March Marc | , , , , , , , , , , , , , , , , , , , | | Jan | Feb | Mar | Apr | May | Jun | Jul | Aug | Sep | Oct | Nov | Dec | | | |
| March Marc | Monthly process wastewater generated as % of annua | l total: | 4% | 6% | 6% | 5% | 6% | 7% | 9% | 10% | 14% | 14% | 11% | 8% | | | |
| Monthly Vineyard Irrigation Water Use General rope role rotate use 0 1 | | | | | | | | | | | | | | | | | |
| Part | inominy process wastewater generated (gamons). | | 10,000 | 13,000 | 15,000 | 12,300 | 13,000 | 17,500 | 22,300 | 25,000 | 33,000 | 33,000 | 27,300 | 20,000 | | | |
| Personal process processes processes of the security foundation and of previous methods 7,865 0 0 0 0 0 0 0 0 0 | Monthly Vineyard Irrigation Water Us | se | | | | | | | | | | | | | | | |
| Clase matter brought forward from confer previous month) | | | <u>Jan</u> | <u>Feb</u> | Mar | <u>Apr</u> | May | <u>Jun</u> | <u>Jul</u> | Aug | Sep | <u>Oct</u> | Nov | <u>Dec</u> | | | |
| Interior per month per wise (pillows) | | | 7,865 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | | | |
| Process washerwise perturnal fine from the production of the process washerwise perturnal fine from the production of the process washerwise perturnal fine from the production of the process washerwise perturnal fine from the production of the process washerwise perturnal fine from the production of the process washerwise perturnal fine from the production of the process washerwise perturnal fine from the production of the process washerwise perturnal fine from the production of the process washerwise perturnal fine from the production of the process washerwise perturnal fine from the production of the process washerwise perturnal fine from the production of the process washerwise perturnal from the process w | Vineyard irrigation as % of peak month irrigation den | nand: | 6% | 6% | 10% | 100% | 100% | 100% | 100% | 100% | 100% | 100% | 10% | 10% | | | |
| Maritangard bia irrigated with reclaimed water this mouth? 7 | Irrigation per month per vine (gallons): | | 2 | 2 | 3 | 26 | 26 | 26 | 26 | 26 | 26 | 26 | 3 | 3 | | | |
| Process wasteward generated this month, reclaimed for vineyard irrigation [galborn] 7,281 | Total vineyard irrigation demand [gallons]: | | 7,281 | 7,281 | 12,135 | 121,346 | 121,346 | 121,346 | 121,346 | 121,346 | 121,346 | 121,346 | 12,135 | 12,135 | | | |
| | | | у | у | у | у | у | у | у | у | у | у | у | у | | | |
| | [gallons] | | 7,281 | 7,281 | 12,135 | 12,500 | 15,000 | 17,500 | 22,500 | 25,000 | 35,000 | 35,000 | 12,135 | 12,135 | | | |
| Net storage after vineyard irrigation demand 0 0 0 108.846 106.346 103.846 98.846 96.346 86.346 86.346 0 0 0 0 0 0 0 0 0 | | month's process water | 0 | 0 | 0 | 108,846 | 106,346 | 103,846 | 98,846 | 96,346 | 86,346 | 86,346 | 0 | 0 | | | |
| This month's process wastewater, remaining after vineyard irrigation, available for landscape irrigation (gallons) | Drawdown from storage for remaining vineyard irriga | tion [gallons] | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | | | |
| This month's process wastewater, remaining after vineyard irrigation, available for landscape irrigation[gallons] or Nature Description of Landscape irrigation Mater Use **House Description** **House Desc | Well water required to satisfy remaining vineyard irrig | gation demand | 0 | 0 | 0 | 108,846 | 106,346 | 103,846 | 98,846 | 96,346 | 86,346 | 86,346 | 0 | 0 | | | |
| Second S | Net storage after vineyard irrigation drawdown [gallon | ns] | 7,865 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | | | |
| Monthly Landscape Irrigation Water Use Based on evapotranspiration crop demand and irrigated area) Jan Seb Mar Anv Nav Jun Jul Aug Sep Oct Nov New | | yard irrigation, available | 2,719 | 7,719 | 2,865 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 15,365 | 7,865 | | | |
| Hased on evapotranspiration crop demand and irrigated area) Jan Feb Mar Apr May Jun Jul Aug Sep Oct Nov December 1 Nov Nov December 2 Nov | | | Wate | r balance con | tinues on ne. | xt page for cov | er crop irrigo | tion. | | | | | | | | | |
| This month's process wastewater, remaining after vineyard irrigation, available for landscape irrigation [gallons] (From sheet 1) 1.03 1.53 2.93 4.71 5.82 6.85 7.21 6.44 4.87 3.53 1.64 1.17 Crop Coefficient (k _o) (see note 2) 0.80 0.80 0.80 0.80 0.80 0.80 0.80 0.8 | | | | | | | | | | | - | | | - | | | |
| for landscape irrigation[gallons] (From sheet I) 1.03 1.53 2.93 4.71 5.82 6.85 7.21 6.44 4.87 3.53 1.64 1.17 Crop Coefficient (kc) (see note 2) 0.80 0.80 0.80 0.80 0.80 0.80 0.80 0.8 | | <u> </u> | <u>Jan</u> | Feb | Mar | <u>Apr</u> | May | Jun | <u>Jul</u> | Aug | Sep | Oct | Nov | <u>Dec</u> | | | |
| Reference ET (ETo) (in/month) (see note 1) 1.03 1.53 2.93 4.71 5.82 6.85 7.21 6.44 4.87 3.53 1.64 1.17 Crop Coefficient (kc) (see note 2) 0.80 | | eyard irrigation, available | 2,719 | 7,719 | 2,865 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 15,365 | 7,865 | | | |
| Crop water demand per acre [inches] 0.82 1.22 2.34 3.77 4.66 5.48 5.77 5.15 3.90 2.82 1.31 0.94 Crop water demand per acre [gallons] 22,374 33.235 63.645 102,310 126,422 148,795 156,615 139,889 105,786 76,678 35,624 25,415 Total crop water demand for irrigated area [gallons] 11,187 16,617 31,823 51,155 63,211 74,398 78,308 69,945 52,893 38,339 17,812 12,707 Will landscape be irrigated with reclaimed water this month? Y Y Y N N N N N N N N N N N Y Y Y Process wastewater remaining after vineyard irrigation, reclaimed for landscape irrigation [gallons] 8,468 8,898 28,957 0 0 0 0 0 0 0 0 0 38,339 2,447 4,842 Drawdown from storage for landscape irrigation [gallons] 7,865 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 | | | 1.03 | 1.53 | 2.93 | 4.71 | 5.82 | 6.85 | 7.21 | 6.44 | 4.87 | 3.53 | 1.64 | 1.17 | | | |
| Crop water demand per acre [gallons] 22,374 33,235 63,645 102,310 126,422 148,795 156,615 139,889 105,786 76,678 35,624 25,415 Total crop water demand for irrigated area [gallons] 11,187 16,617 31,823 51,155 63,211 74,398 78,308 69,945 52,893 38,339 17,812 12,707 Will landscape be irrigated with reclaimed water this month? Y Y Y N N N N N N N N N N Y Y Y Process wastewater remaining after vineyard irrigation, reclaimed for landscape irrigation gallons] 8,468 8,898 28,957 0 0 0 0 0 0 0 0 38,339 2,447 4,842 Drawdown from storage for landscape irrigation generated this month, unused for irrigation, to be reclaimed and stored [gallons] 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 | Crop Coefficient (k _c) (see note 2) | | 0.80 | 0.80 | 0.80 | 0.80 | 0.80 | 0.80 | 0.80 | 0.80 | 0.80 | 0.80 | 0.80 | 0.80 | | | |
| Total crop water demand for irrigated area [gallons] 11,187 16,617 31,823 51,155 63,211 74,398 78,308 69,945 52,893 38,339 17,812 12,707 Will landscape be irrigated with reclaimed water this month? Y Y Y N N N N N N N N N N Y Y Y Process wastewater remaining after vineyard irrigation, reclaimed for landscape irrigation [gallons] 2,719 7,719 2,865 0 0 0 0 0 0 0 0 0 0 0 15,365 7,865 Landscape irrigation water required from storage or other source [gallons] 8,468 8,898 28,957 0 0 0 0 0 0 0 0 38,339 2,447 4,842 Drawdown from storage for landscape irrigation [gallons] 7,865 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 Process wastewater generated this month, unused for irrigation, to be reclaimed and stored [gallons] 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 Net end-of-month reclaimed water storage after all irrigation [gallons] 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 | Crop water demand per acre [inches] | | 0.82 | 1.22 | 2.34 | 3.77 | 4.66 | 5.48 | 5.77 | 5.15 | 3.90 | 2.82 | 1.31 | 0.94 | | | |
| Will landscape be irrigated with reclaimed water this month? Y Y Y Y N N N N N N N N N N N N Y | Crop water demand per acre [gallons] | | 22,374 | 33,235 | 63,645 | 102,310 | 126,422 | 148,795 | 156,615 | 139,889 | 105,786 | 76,678 | 35,624 | 25,415 | | | |
| Process wastewater remaining after vineyard irrigation, reclaimed for landscape irrigation [gallons] Landscape irrigation water required from storage or other source [gallons] 8.468 8.898 2.719 7.719 2.865 0 0 0 0 0 0 0 0 0 38.339 2.447 4.842 Drawdown from storage for landscape irrigation [gallons] 7.865 0 0 0 0 0 0 0 0 0 0 0 0 0 | Total crop water demand for irrigated area [gallons] | | 11,187 | 16,617 | 31,823 | 51,155 | 63,211 | 74,398 | 78,308 | 69,945 | 52,893 | 38,339 | 17,812 | 12,707 | | | |
| Integration [gallons] 2,719 7,719 2,865 0 0 0 0 0 0 0 13,365 7,865 Landscape irrigation water required from storage or other source [gallons] 8,468 8,898 28,957 0 0 0 0 0 0 0 38,339 2,447 4,842 Drawdown from storage for landscape irrigation [gallons] 7,865 0 | Will landscape be irrigated with reclaimed water this i | month? | Y | Y | Y | N | N | N | N | N | N | Y | Y | Y | | | |
| Drawdown from storage for landscape irrigation [gallons] 7,865 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 | | n, reclaimed for landscape | 2,719 | 7,719 | 2,865 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 15,365 | 7,865 | | | |
| Process wastewater generated this month, unused for irrigation, to be reclaimed and stored [gallons] 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 | Landscape irrigation water required from storage or o | ther source [gallons] | 8,468 | 8,898 | 28,957 | 0 | 0 | 0 | 0 | 0 | 0 | 38,339 | 2,447 | 4,842 | | | |
| and stored [gallons] Net end-of-month reclaimed water storage after all irrigation [gallons] 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 | Drawdown from storage for landscape irrigation [galle | ons] | 7,865 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | | | |
| | | irrigation, to be reclaimed | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | | | |
| End of Water Balance | Net end-of-month reclaimed water storage after all irr | igation [gallons] | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | | | |
| | | | | | End of Wa | ter Balance | | | | | | | | | | | |

Notes:

Peak Monthly Storage =

- 1. Reference ETo from California Irrigation Management Information System
- 2. Crop Coefficient from Table 1 of "Estimating Irrigation Water Needs of Landscape Plantings in California", University of California Cooperative Extension, August 2000.

0 gallons



HYDROLOGY REPORT

Prepared for

ANTHEM WINERY 3123 DRY CREEK ROAD NAPA, CA 94558

Prepared for: Justin and Julie Arbuckle 400 Spear Street, Suite #122 San Francisco, CA 94105



RSA+ Project No. 4111010.0

December 3, 2015 Revised: January 9, 2019



TABLE OF CONTENTS

| I. PURPOSE | 1 |
|---|---|
| II. EXISTING CONDITIONS | 1 |
| III. PROPOSED DEVELOPMENT | 1 |
| IV. PRE & POST-DEVELOPMENT DRAINAGE STUDY | 1 |
| EXISTING CONDITIONS | 2 |
| PROPOSED CONDITIONS | 3 |
| V CONCLUSIONS | 1 |

ATTACHMENTS

- 1. VICINITY MAP, SOIL MAP
- 2. CIVIL IMPROVEMENT PLANS
- 3. HYDROLOGY CALCULATIONS



I. Purpose

This report addresses the runoff requirements of Napa County Policy CON-50c. This project is identified as a "Discretionary Project". The requirements are outlined in the Napa County General Plan, dated June 23, 2009.

II. Existing Conditions

The Anthem Winery project is located at 3123 Dry Creek Road, Napa, California. The project's APNs are 035-460-038 and 035-470-046, and have a combined area of 44.77 +/- acres. The project site varies in slope from 1-30%. Redwood Creek, a blue-line stream, runs roughly north to south on the western side of the property. There are two residences, a guest house, and a winery located on the parcels. Refer to Attachment 1 for a Vicinity Map and Soil Map.

III. Proposed Development

The Owner is applying to the County of Napa for a modification to a Winery Use Permit that allows operation of a 50,000 gallon per year winery. The proposed winery will be constructed in the area of the existing winery, and will be entirely on the south parcel (035-470-046). Public access to the proposed winery will be from the existing residential driveway connecting to Dry Creek Road. The driveway will be upgraded as required to provide commercial access. The proposed project will disturb an area of approximately 2.5 acres. Refer to Civil Improvement Plans in Attachment 2, for the overall scope of the project.

IV. PRE & POST-DEVELOPMENT DRAINAGE STUDY

This drainage study computes the pre- and post-development total flow rates from the project area for the 2-, 10-, 50- and 100-year, 24-hour design storms, as required by CON-50c. It shows that post-development runoff does not exceed pre-development runoff for these events.

The following precipitation data for the project site was collected from the NOAA Atlas 14, Volume 6 – California (refer to NOAA Precipitation Frequency table in Attachment 3):

Table 1 - NOAA Precipitation Data

| Storm Frequency | Precipitation Depth (inches, in 24 hour period) |
|-----------------|---|
| 2-yr | 3.40 |
| 10-yr | 5.17 |
| 50-yr | 6.99 |
| 100-yr | 7.77 |



Existing Conditions

The method used for studying the site stormwater runoff is a hydrograph analysis. The unit hydrograph rainfall distribution for the County of Napa falls under Type IA-distribution. The SCS hydrograph analysis is based on the National Resources Conservation Service Technical Release 55 for Urban Hydrology for Small Watersheds (TR-55) method. The purpose of the hydrograph analysis is to identify and mitigate the increase in the pre- to post-construction runoff flows. The project site consists of 3 separate watersheds, Redwood Creek and Salvador Channel. Five drainage sub-sheds have been identified within the three watersheds in order to model the pre- to post-construction runoff for the project site. The sub-sheds are then added together and the total existing flow rates are analyzed for the purpose of this study. The existing runoff for the 2-, 10-, 50- and 100-year, 24-hour storm events are as follows (refer to Basin Maps and Hydrology Calculations in Attachment 3.

Table 2 – Existing Stormwater Runoff

| Watershed 1 –Redwood Creek | | | |
|-----------------------------------|----------------------------|-------|--------|
| Shed 1 | A_1 | 7.39 | [acre] |
| Existing Impervious Area | A_{1_Paved} | 0.06 | [acre] |
| Existing Curve Number | CN ₁ | 79 | |
| Existing Time of Concentration | T _{C-1} | 10.24 | [min] |
| Watershed 2 – Salvador Channel | | | |
| Sub-shed 2A | A_{2A} | 1.56 | [acre] |
| Existing Impervious Area | A_{2A_Paved} | 0.12 | [acre] |
| Existing Curve Number | CN _{2A} | 78 | |
| Existing Time of Concentration | T _{C-2A} | 7.44 | [min] |
| Sub-shed 2B | A_{2B} | 8.98 | [acre] |
| Existing Impervious Area | A_{2B_Paved} | 0.59 | [acre] |
| Existing Curve Number | CN_{2B} | 76 | |
| Existing Time of Concentration | T_{C-2B} | 11.53 | [min] |
| Watershed 3 – Salvador Channel | | | |
| Sub-shed 3A | A_{3A} | 1.62 | [acre] |
| Existing Impervious Area | A_{3A_Paved} | 0.24 | [acre] |
| Existing Curve Number | CN _{3A} | 82 | |
| Existing Time of Concentration | T _{C-3A} | 9.39 | [min] |
| Sub-shed 3B | A _{3B} | 0.22 | [acre] |
| Existing Impervious Area | A_{3B_Paved} | 0.10 | [acre] |
| Existing Curve Number | CN _{3B} | 88 | |
| Existing Time of Concentration | T _{C-3B} | 6.00* | [min] |
| Watershed 1 – Redwood Creek | | | |
| Total Existing Peak Flow (2-yr) | Q _{2YEAR-EXIST} | 2.43 | [cfs] |
| Total Existing Peak Flow (10-yr) | Q _{10YEAR-EXIST} | 5.32 | [cfs] |
| Total Existing Peak Flow (50-yr) | Q _{50YEAR-EXIST} | 8.54 | [cfs] |
| Total Existing Peak Flow (100-yr) | Q _{100YEAR-EXIST} | 9.95 | [cfs] |



| Watershed 2 - Salvador Channel | | | |
|-----------------------------------|----------------------------|-------|-------|
| Total Existing Peak Flow (2-yr) | Q _{2YEAR-EXIST} | 2.65 | [cfs] |
| Total Existing Peak Flow (10-yr) | Q _{10YEAR-EXIST} | 6.46 | [cfs] |
| Total Existing Peak Flow (50-yr) | Q _{50YEAR-EXIST} | 10.88 | [cfs] |
| Total Existing Peak Flow (100-yr) | Q _{100YEAR-EXIST} | 12.84 | [cfs] |
| Watershed 3 - Salvador Channel | | | |
| Total Existing Peak Flow (2-yr) | Q _{2YEAR-EXIST} | 0.76 | [cfs] |
| Total Existing Peak Flow (10-yr) | Q _{10YEAR-EXIST} | 1.52 | [cfs] |
| Total Existing Peak Flow (50-yr) | Q _{50YEAR-EXIST} | 2.34 | [cfs] |
| Total Existing Peak Flow (100-yr) | Q _{100YEAR-EXIST} | 2.71 | [cfs] |

See worksheets included in Attachment 3

Proposed Conditions

To comply with the CON-50c requirement for stormwater quantity control, the post-development flow may not exceed pre-development flow for the 2-, 10-, 50-, and 100-year, 24-hour storm event.

The same drainage watersheds and sub-sheds were used to analyze the post-construction runoff flows. Watershed 1 will sheet flow to vegetated areas at the same flow rate as the existing condition. No mitigation is required for Watershed 1. 12,700 cubic feet of detention chambers will be installed under the winery driveway entrance in sub-shed 2A to mitigate post-construction peak flows in the Salvador Channel Watershed 2. Two, 4-foot diameter by 80-foot long detention pipes, providing 2,010 cubic feet of detention, will be installed in the driveway near the Dry Creek Road entrance in sub-shed 3A to mitigate post construction peak flows in the Salvador Channel Watershed 3. The captured stormwater will be conveyed to level spreaders and rock outfalls and will then return to natural flow lines. These measures will limit the peak developed discharge rates to the pre-construction levels. The proposed runoff for the 2-, 10-, 50-, and 100-year, 24-hour storm events are shown in table 3 (refer to Basin Maps and Hydrology Calculations in Attachment 3).

Table 3 – Proposed Stormwater Runoff

| Table 3 Troposed Stormwater Ranon | | | |
|-------------------------------------|-------------------|-------|--------|
| Watershed 1 – Redwood Creek | | | |
| Shed 1 | A ₁ | 7.39 | [acre] |
| Proposed Impervious Area | A ₁ | 0.24 | [acre] |
| Proposed Curve Number | CN ₁ | 79 | |
| Proposed Time of Concentration | T _{C-1} | 10.24 | [min] |
| Watershed 2 Area – Salvador Channel | | | |
| Sub-shed 2A | A _{2A} | 1.56 | [acre] |
| Proposed Impervious Area | A _{2A} | 0.98 | [acre] |
| Proposed Curve Number | CN _{2A} | 92 | |
| Proposed Time of Concentration | T _{C-2A} | 6.00* | [min] |

^{*} Adopted minimum Tc=6.00 min.



| Sub-shed 2B Proposed Impervious Area Proposed Curve Number Proposed Time of Concentration Watershed 3 Area – Salvador Channel Sub-shed 3A Proposed Impervious Area Proposed Impervious Area Proposed Impervious Area Proposed Curve Number Proposed Curve Number Proposed Time of Concentration Sub-shed 3B Proposed Impervious Area Proposed Curve Number Proposed Time of Concentration Watershed 1 – Redwood Creek Total Proposed Peak Flow (2-yr) Total Proposed Peak Flow (10-yr) Q10YEAR-POST Total Proposed Peak Flow (100-yr) Q100YEAR-POST Q50YEAR-POST |
|--|
| Proposed Curve Number Proposed Time of Concentration Watershed 3 Area – Salvador Channel Sub-shed 3A Proposed Impervious Area Proposed Curve Number Proposed Time of Concentration Sub-shed 3B Proposed Impervious Area Proposed Impervious Area Proposed Impervious Area Proposed Time of Concentration Sub-shed 3B Proposed Impervious Area Proposed Impervious Area Proposed Impervious Area Proposed Curve Number Proposed Curve Number Proposed Time of Concentration Watershed 1 – Redwood Creek Total Proposed Peak Flow (2-yr) Total Proposed Peak Flow (10-yr) Total Proposed Peak Flow (50-yr) Total Proposed Peak Flow (50-yr) Total Proposed Peak Flow (100-yr) Q100YEAR-POST Q100YEAR-POST Q100YEAR-POST Q9.95 [cfs] |
| Proposed Time of ConcentrationTC-2B9.64[min]Watershed 3 Area – Salvador ChannelA2C1.62[acre]Sub-shed 3AA2C0.38[acre]Proposed Impervious AreaA2C0.38[acre]Proposed Curve NumberCN2C83Proposed Time of ConcentrationTC-2C6.00*[min]Sub-shed 3BA2C0.22[acre]Proposed Impervious AreaA2C0.12[acre]Proposed Curve NumberCN2C89Proposed Time of ConcentrationTC-2C6.00*[min]Watershed 1 – Redwood CreekTotal Proposed Peak Flow (2-yr)Q2YEAR-POST2.43[cfs]Total Proposed Peak Flow (10-yr)Q10YEAR-POST5.32[cfs]Total Proposed Peak Flow (50-yr)Q50YEAR-POST8.54[cfs]Total Proposed Peak Flow (100-yr)Q100YEAR-POST9.95[cfs] |
| Watershed 3 Area – Salvador Channel Sub-shed 3A Proposed Impervious Area Proposed Curve Number CN2C Proposed Time of Concentration Sub-shed 3B Proposed Impervious Area Proposed Impervious Area Proposed Impervious Area Proposed Impervious Area Proposed Curve Number CN2C Proposed Curve Number CN2C Proposed Time of Concentration TC-2C Proposed Time of Concentration TC-2C Proposed Time of Concentration TC-2C Total Proposed Peak Flow (2-yr) Total Proposed Peak Flow (10-yr) Total Proposed Peak Flow (50-yr) Qsoyear-post Total Proposed Peak Flow (100-yr) Qsoyear-post Total Proposed Peak Flow (100-yr) Qsoyear-post Qsoyear-post Post [cfs] |
| Sub-shed 3A Proposed Impervious Area Proposed Curve Number Proposed Time of Concentration Sub-shed 3B Proposed Impervious Area Proposed Impervious Area Proposed Impervious Area Proposed Impervious Area Proposed Curve Number Proposed Curve Number Proposed Time of Concentration Tour Concentration Watershed 1 – Redwood Creek Total Proposed Peak Flow (2-yr) Total Proposed Peak Flow (10-yr) Total Proposed Peak Flow (50-yr) Total Proposed Peak Flow (100-yr) Quoyear-post |
| Proposed Impervious Area Proposed Curve Number Proposed Time of Concentration Sub-shed 3B Proposed Impervious Area Proposed Impervious Area Proposed Impervious Area Proposed Curve Number Proposed Curve Number Proposed Time of Concentration Watershed 1 – Redwood Creek Total Proposed Peak Flow (2-yr) Total Proposed Peak Flow (10-yr) Total Proposed Peak Flow (50-yr) Total Proposed Peak Flow (50-yr) Q100YEAR-POST |
| $\begin{array}{c ccccccccccccccccccccccccccccccccccc$ |
| Proposed Time of Concentration Sub-shed 3B Proposed Impervious Area Proposed Curve Number Proposed Time of Concentration CN2c Proposed Time of Concentration Watershed 1 – Redwood Creek Total Proposed Peak Flow (2-yr) Total Proposed Peak Flow (10-yr) Total Proposed Peak Flow (50-yr) Total Proposed Peak Flow (100-yr) Q100YEAR-POST |
| Sub-shed 3B Proposed Impervious Area Proposed Curve Number Proposed Time of Concentration Watershed 1 – Redwood Creek Total Proposed Peak Flow (2-yr) Total Proposed Peak Flow (10-yr) Total Proposed Peak Flow (50-yr) Total Proposed Peak Flow (100-yr) Q100YEAR-POST |
| Proposed Impervious Area Proposed Curve Number Proposed Time of Concentration Watershed 1 – Redwood Creek Total Proposed Peak Flow (2-yr) Total Proposed Peak Flow (10-yr) Total Proposed Peak Flow (50-yr) Qsoyear-post |
| Proposed Curve Number Proposed Time of Concentration Watershed 1 – Redwood Creek Total Proposed Peak Flow (2-yr) Total Proposed Peak Flow (10-yr) Total Proposed Peak Flow (50-yr) Total Proposed Peak Flow (100-yr) Total Proposed Peak Flow (100-yr) Qsoyear-post |
| Proposed Time of Concentration T _{C-2C} 6.00* [min] Watershed 1 – Redwood Creek Total Proposed Peak Flow (2-yr) Q _{2YEAR-POST} 2.43 [cfs] Total Proposed Peak Flow (10-yr) Q _{10YEAR-POST} 5.32 [cfs] Total Proposed Peak Flow (50-yr) Q _{50YEAR-POST} 8.54 [cfs] Total Proposed Peak Flow (100-yr) Q _{100YEAR-POST} 9.95 [cfs] |
| Watershed 1 – Redwood Creek Total Proposed Peak Flow (2-yr) Total Proposed Peak Flow (10-yr) Total Proposed Peak Flow (50-yr) Total Proposed Peak Flow (50-yr) Total Proposed Peak Flow (100-yr) Q _{50YEAR-POST} S ₅ [cfs] |
| Total Proposed Peak Flow (2-yr) Q _{2YEAR-POST} 2.43 [cfs] Total Proposed Peak Flow (10-yr) Q _{10YEAR-POST} 5.32 [cfs] Total Proposed Peak Flow (50-yr) Q _{50YEAR-POST} 8.54 [cfs] Total Proposed Peak Flow (100-yr) Q _{100YEAR-POST} 9.95 [cfs] |
| Total Proposed Peak Flow (10-yr) Q _{10YEAR-POST} 5.32 [cfs] Total Proposed Peak Flow (50-yr) Q _{50YEAR-POST} 8.54 [cfs] Total Proposed Peak Flow (100-yr) Q _{100YEAR-POST} 9.95 [cfs] |
| Total Proposed Peak Flow (50-yr) Q _{50YEAR-POST} 8.54 [cfs] Total Proposed Peak Flow (100-yr) Q _{100YEAR-POST} 9.95 [cfs] |
| Total Proposed Peak Flow (100-yr) Q _{100YEAR-POST} 9.95 [cfs] |
| |
| Watershed 2 – Salvador Channel |
| |
| Total Proposed Peak Flow (2-yr) Q _{2YEAR-POST} 2.65 [cfs] |
| Total Proposed Peak Flow (10-yr) Q _{10YEAR-POST} 6.29 [cfs] |
| Total Proposed Peak Flow (50-yr) Q _{50YEAR-POST} 10.32 [cfs] |
| Total Proposed Peak Flow (100-yr) Q _{100YEAR-POST} 12.07 [cfs] |
| Watershed 3 – Salvador Channel |
| Total Proposed Peak Flow (2-yr) Q _{2YEAR-POST} 0.72 [cfs] |
| Total Proposed Peak Flow (10-yr) Q _{10YEAR-POST} 1.28 [cfs] |
| Total Proposed Peak Flow (50-yr) Q _{50YEAR-POST} 1.81 [cfs] |
| Total Proposed Peak Flow (100-yr) Q _{100YEAR-POST} 2.02 [cfs] |

See worksheets included in Attachment 3

Upon successful completion of the project and construction of the detention chambers, the post-development peak flow rates will not exceed the pre-development peak flow rates for the 2-, 10-, 50-, and 100-year 24-hour storm events, for the Redwood Creek and Salvador Channel watersheds.

V. Conclusions

There will be no net increase in post-construction peak runoff during 2-, 10-, 50-, and 100-year storm events as required by the Napa County General Plan Policy CON-50c.

^{*} Adopted minimum Tc=6.00 min.

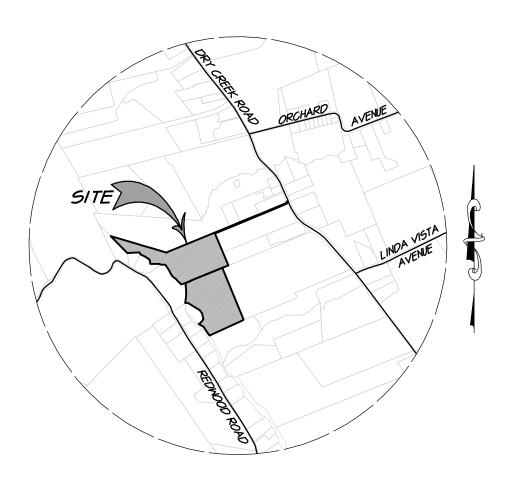


ATTACHMENT 1

Vicinity Map, Soil Map

ANTHEM WINERY VICINITY MAP

NAPA COUNTY CALIFORNIA



VICINITY MAP SCALE: I" = 2000'



1515 FOURTH STREET NAPA, CALIF. 94559 OFFICE | 707 | 252.3301 + www.RSAcivil.com +

155° 21' 38" W

38° 20'21" N

38° 19'51" N

155° 21' 38" W

MAP INFORMATION

The soil surveys that comprise your AOI were mapped at 1:24,000.

Warning: Soil Map may not be valid at this scale.

misunderstanding of the detail of mapping and accuracy of soil line placement. The maps do not show the small areas of contrasting Enlargement of maps beyond the scale of mapping can cause soils that could have been shown at a more detailed scale.

Please rely on the bar scale on each map sheet for map measurements

Web Soil Survey URL: http://websoilsurvey.nrcs.usda.gov Source of Map: Natural Resources Conservation Service Coordinate System: Web Mercator (EPSG:3857)

Albers equal-area conic projection, should be used if more accurate distance and area. A projection that preserves area, such as the Maps from the Web Soil Survey are based on the Web Mercator projection, which preserves direction and shape but distorts calculations of distance or area are required. This product is generated from the USDA-NRCS certified data as of the version date(s) listed below.

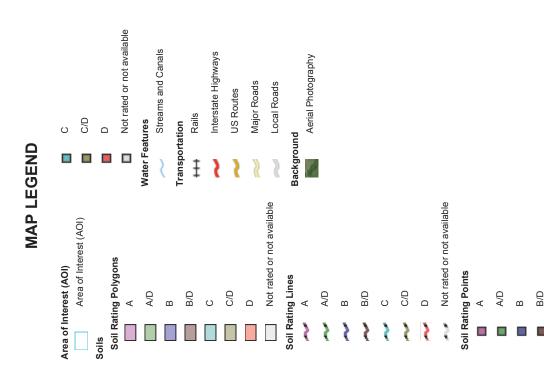
Survey Area Data: Version 7, Sep 25, 2014 Soil Survey Area: Napa County, California

Soil map units are labeled (as space allows) for map scales 1:50,000

or larger.

Date(s) aerial images were photographed: Nov 2, 2010—Feb 17,

imagery displayed on these maps. As a result, some minor shifting The orthophoto or other base map on which the soil lines were compiled and digitized probably differs from the background of map unit boundaries may be evident.



Hydrologic Soil Group

| Hydrologic Soil Group— Summary by Map Unit — Napa County, California (CA055) | | | | | | |
|--|---|--------|--------------|----------------|--|--|
| Map unit symbol | Map unit name | Rating | Acres in AOI | Percent of AOI | | |
| 131 | Fagan clay loam, 5 to 15 percent slopes | С | 0.1 | 0.2% | | |
| 133 | Fagan clay loam, 30 to 50 percent slopes | С | 39.0 | 80.0% | | |
| 136 | Felton gravelly loam, 30 to 50 percent slopes | С | 7.6 | 15.5% | | |
| 168 | Perkins gravelly loam, 2 to 5 percent slopes | С | 1.5 | 3.2% | | |
| 181 | Yolo loam, 0 to 2 percent slopes | В | 0.5 | 1.1% | | |
| Totals for Area of Interest | | 48.7 | 100.0% | | | |

Description

Hydrologic soil groups are based on estimates of runoff potential. Soils are assigned to one of four groups according to the rate of water infiltration when the soils are not protected by vegetation, are thoroughly wet, and receive precipitation from long-duration storms.

The soils in the United States are assigned to four groups (A, B, C, and D) and three dual classes (A/D, B/D, and C/D). The groups are defined as follows:

Group A. Soils having a high infiltration rate (low runoff potential) when thoroughly wet. These consist mainly of deep, well drained to excessively drained sands or gravelly sands. These soils have a high rate of water transmission.

Group B. Soils having a moderate infiltration rate when thoroughly wet. These consist chiefly of moderately deep or deep, moderately well drained or well drained soils that have moderately fine texture to moderately coarse texture. These soils have a moderate rate of water transmission.

Group C. Soils having a slow infiltration rate when thoroughly wet. These consist chiefly of soils having a layer that impedes the downward movement of water or soils of moderately fine texture or fine texture. These soils have a slow rate of water transmission.

Group D. Soils having a very slow infiltration rate (high runoff potential) when thoroughly wet. These consist chiefly of clays that have a high shrink-swell potential, soils that have a high water table, soils that have a claypan or clay layer at or near the surface, and soils that are shallow over nearly impervious material. These soils have a very slow rate of water transmission.

If a soil is assigned to a dual hydrologic group (A/D, B/D, or C/D), the first letter is for drained areas and the second is for undrained areas. Only the soils that in their natural condition are in group D are assigned to dual classes.

Rating Options

Aggregation Method: Dominant Condition

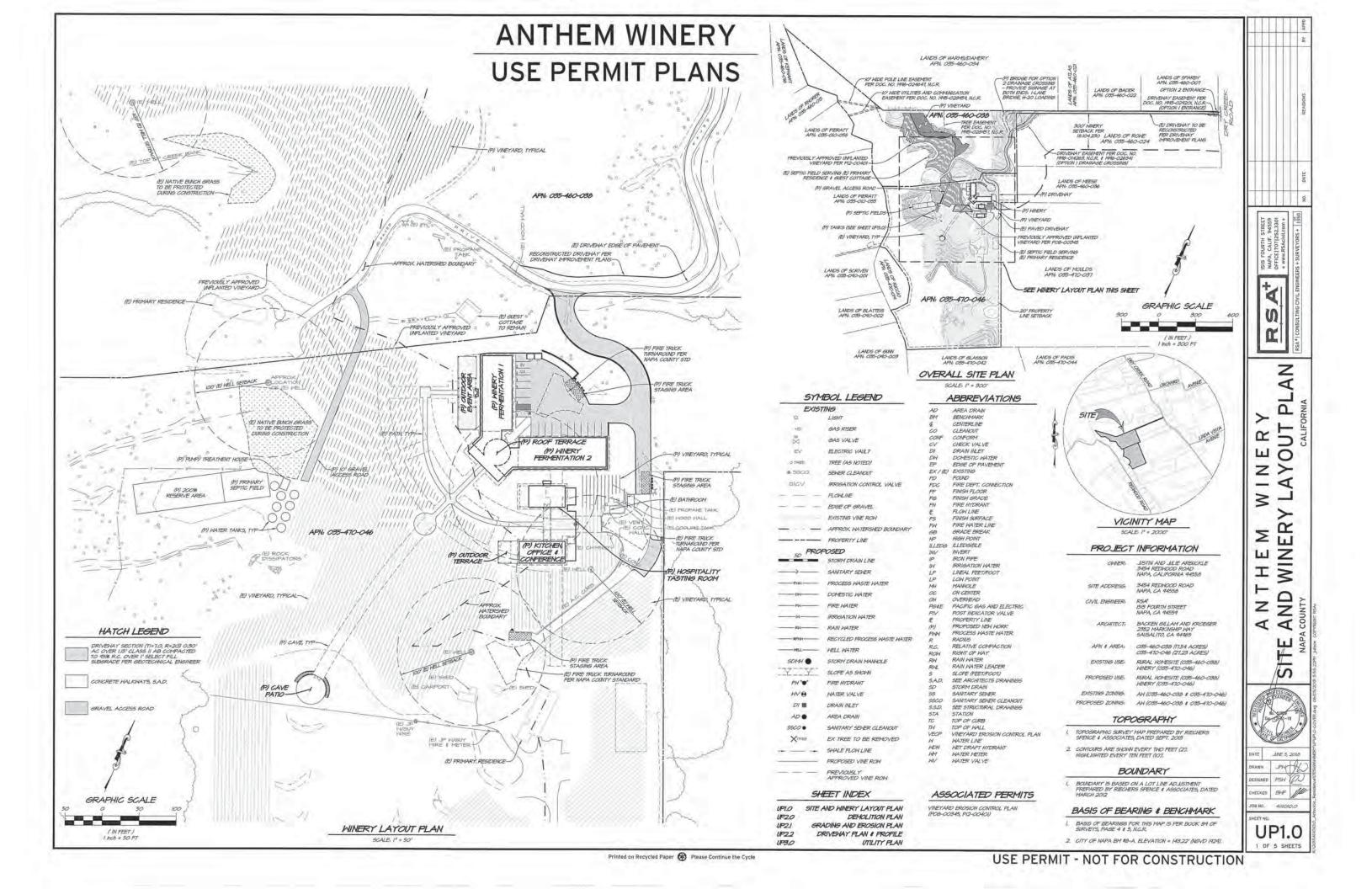
Component Percent Cutoff: None Specified

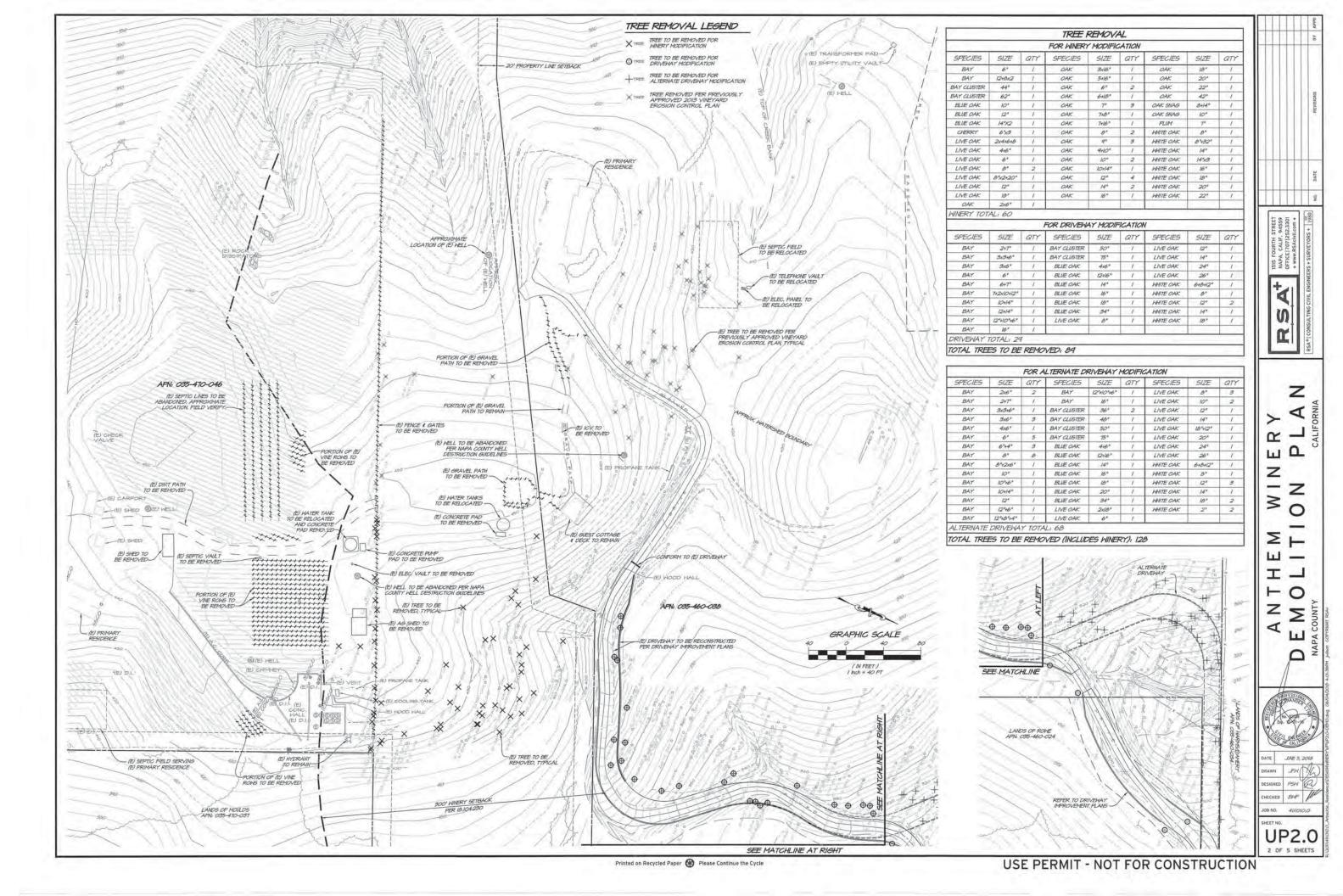
Tie-break Rule: Higher

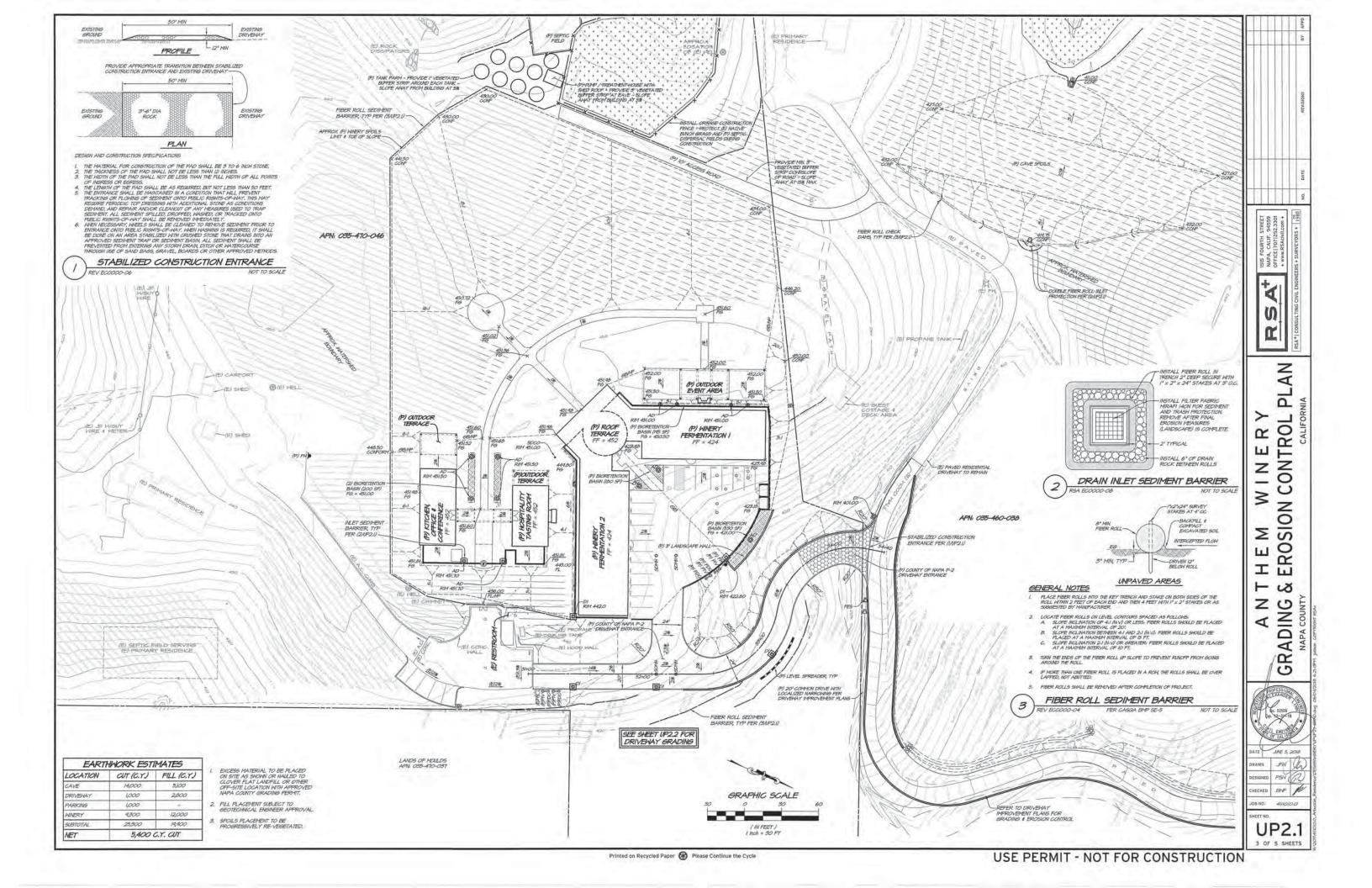


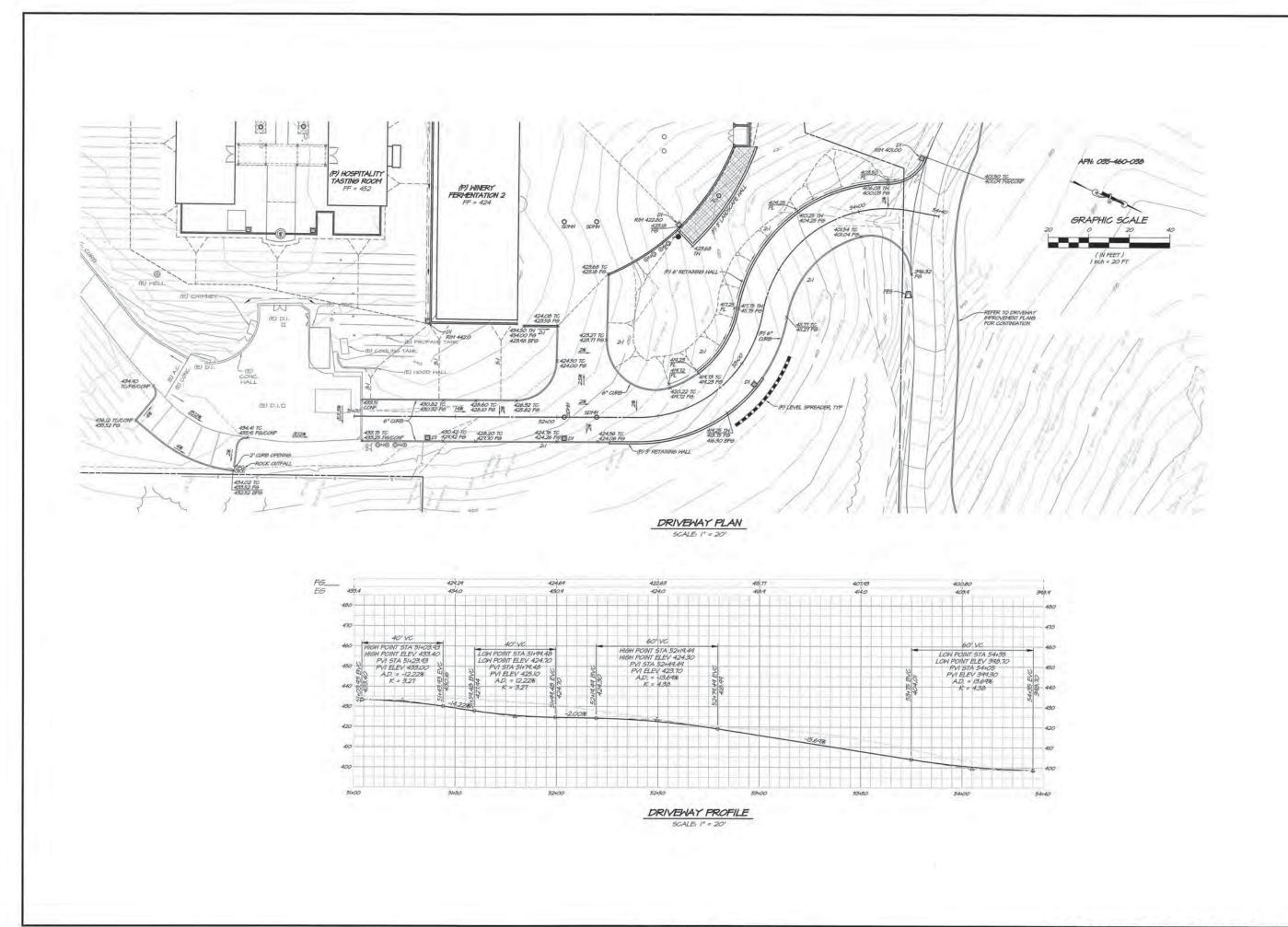
ATTACHMENT 2

Civil Improvement Plans

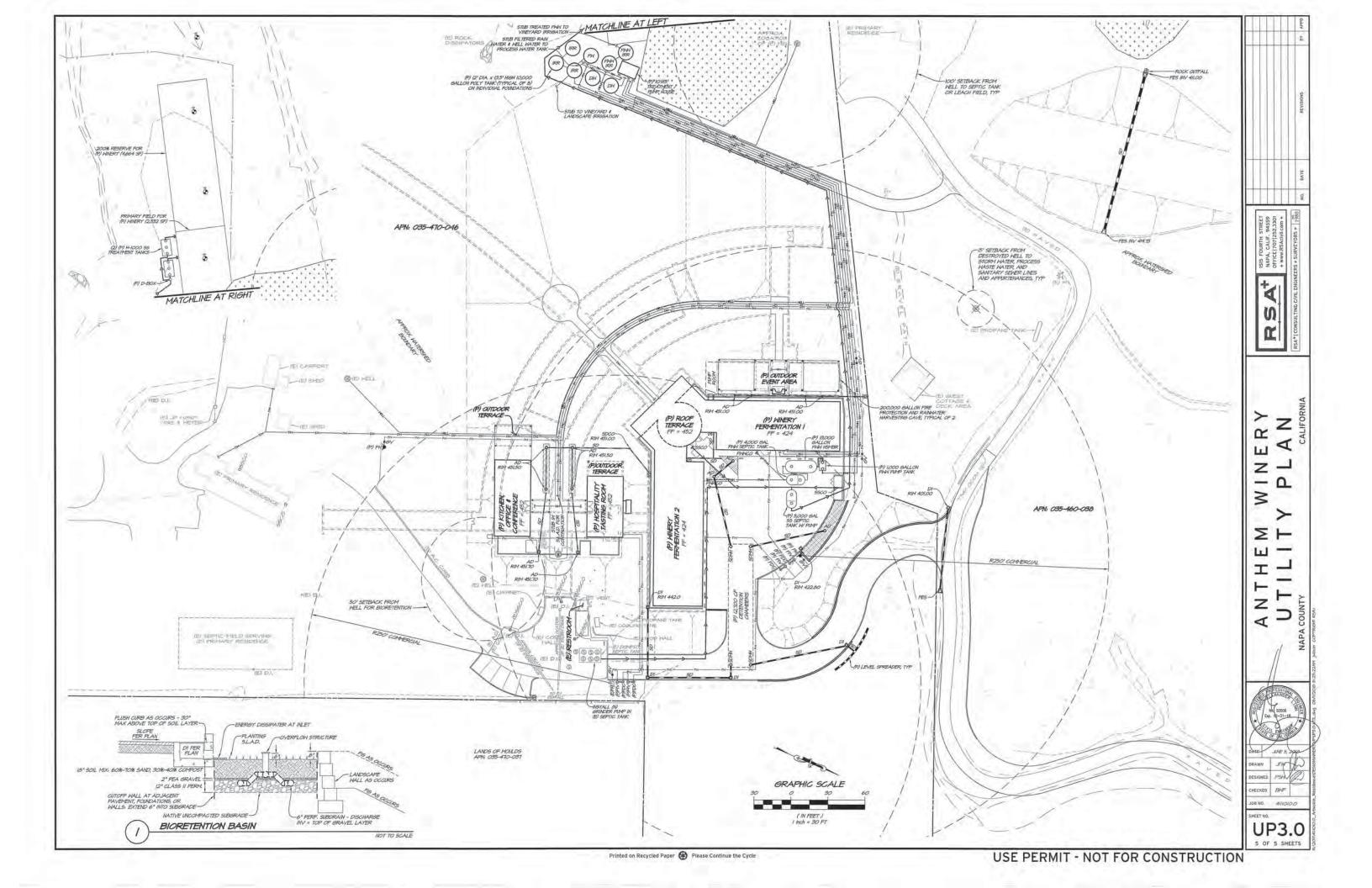








1515 FOURTH STREET
NAPA, CALIF, 94559
OFFICE|707|252.3301
+ www.RSAcivil.com + + W C ш FIL RY ROF M P Z W ≥ Z V Ы Σ ш I RIVEWA Z V HECKED BUF AL JOB NO. 4111010.0 **UP2.2** 4 OF 5 SHEETS

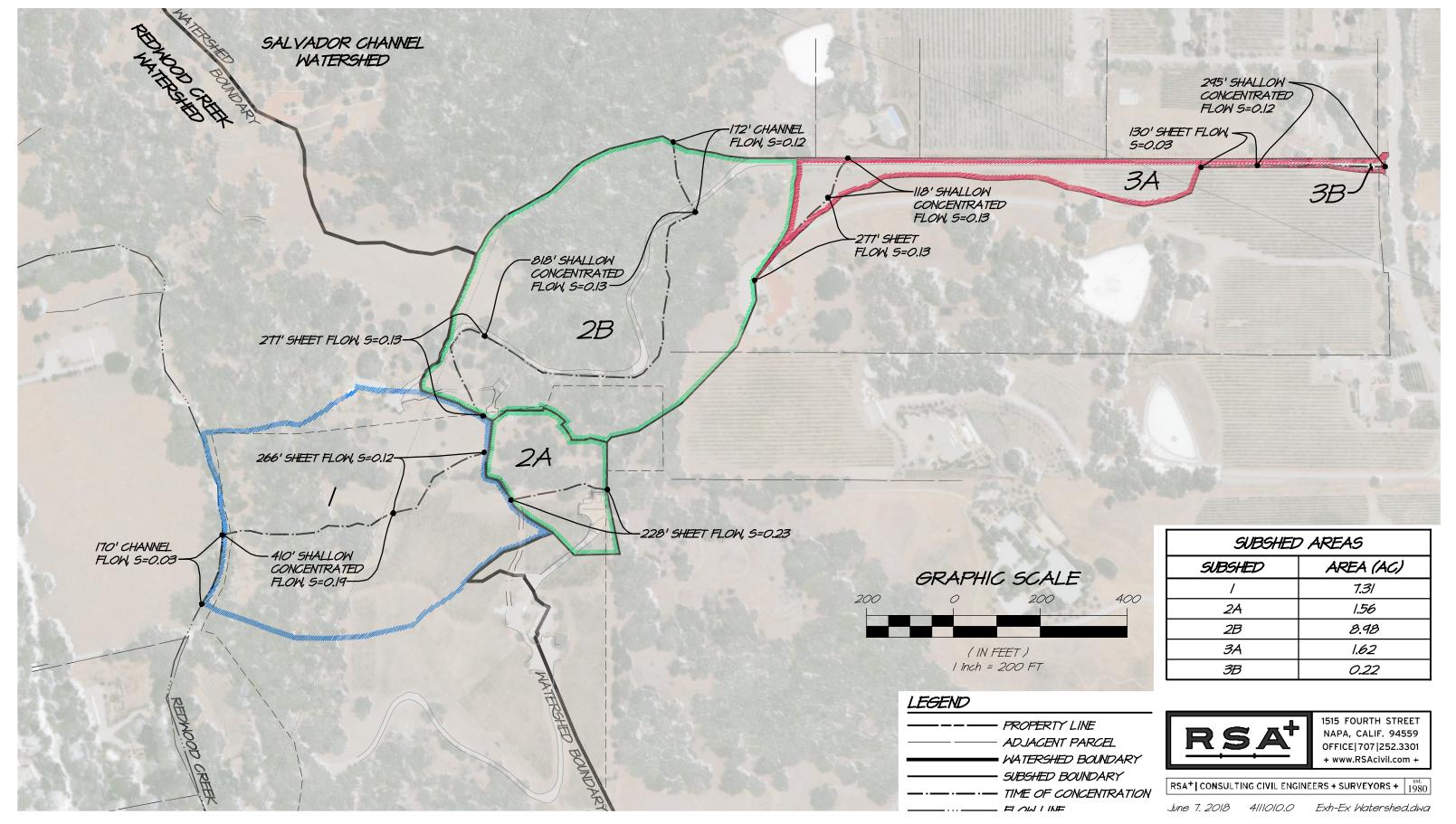




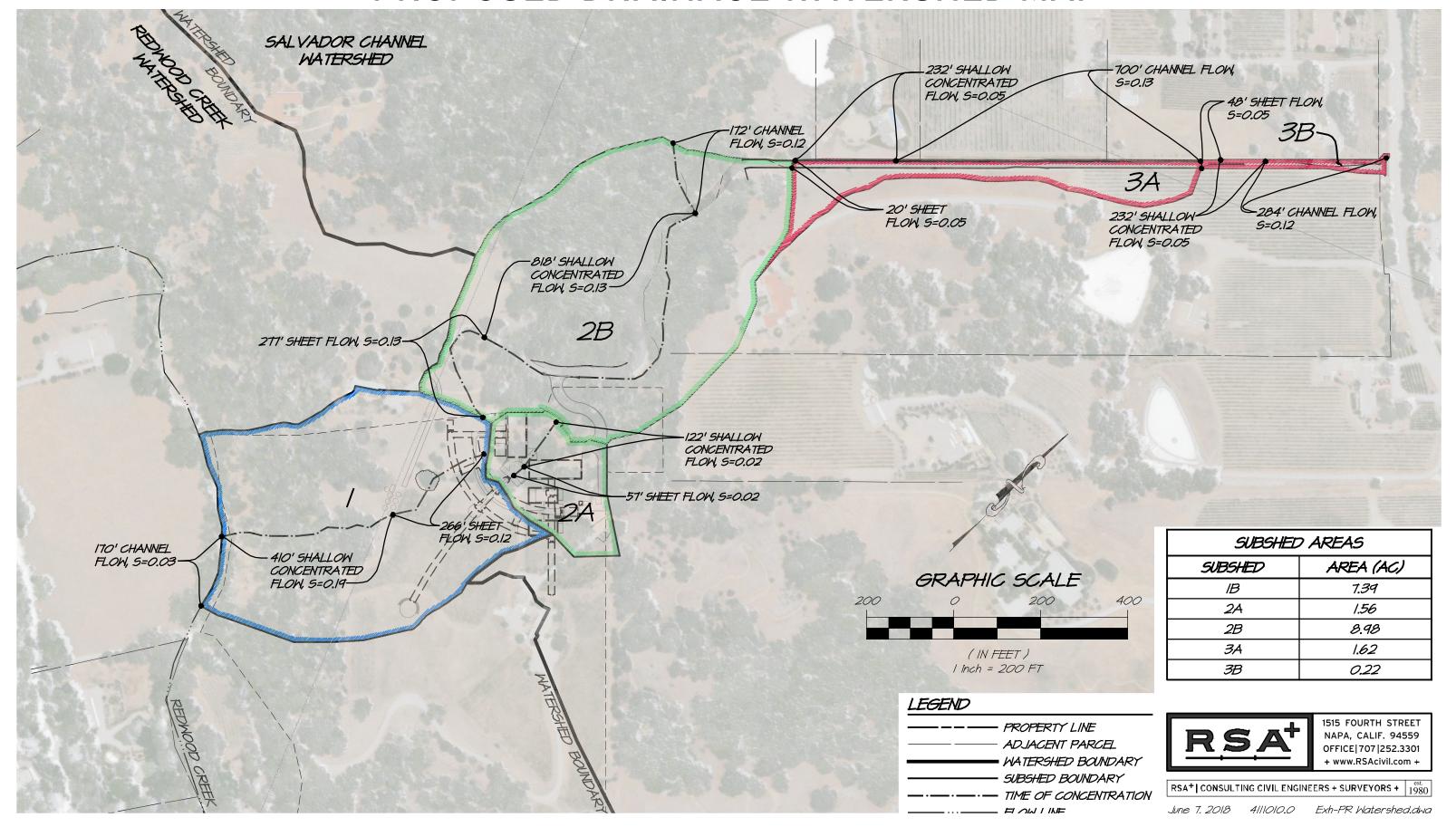
ATTACHMENT 3

Hydrology Calculations

ANTHEM WINERY EXISTING DRAINAGE WATERSHED MAP



ANTHEM WINERY PROPOSED DRAINAGE WATERSHED MAP





NOAA Atlas 14, Volume 6, Version 2 Location name: Napa, California, US* Latitude: 38.3352°, Longitude: -122.3532° Elevation: 381 ft* * source: Google Maps



POINT PRECIPITATION FREQUENCY ESTIMATES

Sanja Perica, Sarah Dietz, Sarah Heim, Lillian Hiner, Kazungu Maitaria, Deborah Martin, Sandra Pavlovic, Ishani Roy, Carl Trypaluk, Dale Unruh, Fenglin Yan, Michael Yekta, Tan Zhao, Geoffrey Bonnin, Daniel Brewer, Li-Chuan Chen, Tye Parzybok, John Yarchoan

NOAA, National Weather Service, Silver Spring, Maryland

PF tabular | PF graphical | Maps & aerials

PF tabular

| PI | PDS-based point precipitation frequency estimates with 90% confidence intervals (in inches) ¹ | | | | | | | | | |
|----------|--|-------------------------------|----------------------------|----------------------------|----------------------------|-------------------------------|----------------------------|----------------------------|----------------------------|----------------------------|
| Duration | | | | Avera | ige recurren | ce interval (y | years) | | | |
| Duration | 1 | 2 | 5 | 10 | 25 | 50 | 100 | 200 | 500 | 1000 |
| 5-min | 0.135 (0.120-0.153) | 0.167 (0.149-0.190) | 0.210 (0.186-0.239) | 0.245 (0.215-0.282) | 0.293 (0.248-0.351) | 0.331 (0.273-0.406) | 0.369 (0.296-0.466) | 0.410 (0.318-0.533) | 0.465 (0.344-0.634) | 0.508 (0.362-0.721) |
| 10-min | 0.193 (0.172-0.219) | 0.240 (0.213-0.272) | 0.301 (0.267-0.343) | 0.351 (0.309-0.404) | 0.421 (0.355-0.503) | 0.474 (0.391-0.581) | 0.530 (0.424-0.667) | 0.587 (0.456-0.764) | 0.666 (0.493-0.909) | 0.728 (0.519-1.03) |
| 15-min | 0.234 (0.208-0.265) | 0.290 (0.258-0.329) | 0.364 (0.323-0.415) | 0.425 (0.373-0.489) | 0.509 (0.430-0.608) | 0.574 (0.473-0.703) | 0.640 (0.513-0.807) | 0.710 (0.551-0.924) | 0.806 (0.596–1.10) | 0.881 (0.627-1.25) |
| 30-min | 0.335 (0.298-0.380) | 0.415 (0.369-0.472) | 0.521 (0.462-0.594) | 0.609 (0.534-0.700) | 0.728 (0.615–0.871) | 0.822 (0.677-1.01) | 0.917 (0.735–1.16) | 1.02 (0.789-1.32) | 1.15 (0.854–1.57) | 1.26 (0.898-1.79) |
| 60-min | 0.485 (0.432-0.551) | 0.602 (0.535-0.684) | 0.756 (0.670-0.862) | 0.883 (0.775-1.02) | 1.06 (0.892–1.26) | 1.19 (0.982-1.46) | 1.33 (1.07–1.68) | 1.47 (1.14–1.92) | 1.67 (1.24–2.28) | 1.83 (1.30-2.60) |
| 2-hr | 0.738 (0.657-0.837) | 0.907 (0.807-1.03) | 1.13 (1.00–1.29) | 1.31 (1.15–1.51) | 1.56 (1.31–1.86) | 1.75 (1.44–2.14) | 1.94 (1.55–2.44) | 2.14 (1.66–2.78) | 2.41 (1.78–3.29) | 2.62 (1.87-3.72) |
| 3-hr | 0.944 (0.841-1.07) | 1.16 (1.03–1.32) | 1.44 (1.28–1.64) | 1.67 (1.47–1.92) | 1.98 (1.67–2.37) | 2.21 (1.83–2.71) | 2.45 (1.97–3.09) | 2.70 (2.10–3.52) | 3.04 (2.25-4.15) | 3.30 (2.35–4.68) |
| 6-hr | 1.40 (1.25–1.59) | 1.73 (1.54–1.97) | 2.15 (1.91–2.45) | 2.49 (2.19–2.87) | 2.95 (2.49–3.53) | 3.30 (2.72-4.05) | 3.66 (2.93-4.61) | 4.02 (3.12–5.23) | 4.51 (3.34–6.15) | 4.88 (3.48-6.93) |
| 12-hr | 1.94 (1.72–2.20) | 2.44 (2.17-2.77) | 3.08 (2.73–3.52) | 3.60 (3.16-4.15) | 4.30 (3.63-5.14) | 4.82 (3.97–5.91) | 5.34 (4.28-6.74) | 5.88 (4.56-7.65) | 6.59 (4.88-8.99) | 7.14 (5.08–10.1) |
| 24-hr | 2.63 (2.37–2.98) | (3.40) (3.06-3.86) | 4.38 (3.93–4.99) | (4.60-5.92) | 6.21 (5.38–7.31) | 6.99 (5.96–8.38) | 7.77 (6.49–9.50) | 8.56 (6.98–10.7) | 9.61 (7.57–12.5) | 10.4 (7.97–13.9) |
| 2-day | 3.42 (3.08-3.88) | 4.43 (3.98-5.03) | 5.72 (5.13–6.50) | 6.74 (6.01–7.73) | 8.12 (7.03–9.55) | 9.15 (7.79–11.0) | 10.2 (8.50–12.4) | 11.2 (9.17–14.1) | 12.6 (9.96–16.4) | 13.7 (10.5–18.3) |
| 3-day | 3.99 (3.59-4.53) | 5.16 (4.63–5.86) | 6.65 (5.96–7.56) | 7.84 (6.98–8.98) | 9.42 (8.17–11.1) | 10.6 (9.05–12.7) | 11.8 (9.87–14.5) | 13.0 (10.6–16.3) | 14.7 (11.6–19.0) | 15.9 (12.2–21.2) |
| 4-day | 4.45 (4.01–5.05) | 5.76 (5.17-6.54) | 7.42 (6.65-8.44) | 8.74 (7.78–10.0) | 10.5 (9.09–12.3) | 11.8 (10.1–14.1) | 13.1 (10.9–16.0) | 14.4 (11.8–18.1) | 16.2 (12.8–21.0) | 17.5 (13.4–23.4) |
| 7-day | 5.47 (4.92-6.21) | 7.12 (6.40-8.08) | 9.17 (8.22–10.4) | 10.8 (9.60–12.3) | 12.9 (11.2–15.1) | 14.4 (12.3–17.3) | 15.9 (13.3–19.5) | 17.4 (14.2–21.8) | 19.4 (15.3–25.1) | 20.8 (16.0–27.8) |
| 10-day | 6.23 (5.61–7.07) | 8.13 (7.31–9.23) | 10.5 (9.39–11.9) | 12.3 (10.9–14.1) | 14.6 (12.7–17.2) | 16.3 (13.9–19.5) | 18.0 (15.0–22.0) | 19.6 (16.0–24.5) | 21.7 (17.1–28.1) | 23.2 (17.8–31.0) |
| 20-day | 8.17 (7.35–9.27) | 10.7 (9.58–12.1) | 13.7 (12.3–15.6) | 16.0 (14.2–18.3) | 18.9 (16.4–22.2) | 20.9 (17.8–25.1) | 22.9 (19.1–28.0) | 24.8 (20.2–31.1) | 27.2 (21.5-35.3) | 29.0 (22.2–38.7) |
| 30-day | 9.86 (8.87–11.2) | 12.8 (11.5–14.5) | 16.3 (14.6–18.6) | 19.0 (16.9–21.7) | 22.3 (19.3–26.3) | 24.7 (21.0-29.5) | 26.9 (22.4-32.9) | 29.0 (23.7–36.3) | 31.7 (25.0-41.1) | 33.7 (25.8-44.9) |
| 45-day | 12.1 (10.9–13.8) | 15.6 (14.0–17.7) | 19.7 (17.6–22.4) | 22.8 (20.3–26.1) | 26.6 (23.0-31.3) | 29.2 (24.9–35.0) | 31.8 (26.5–38.8) | 34.2 (27.9-42.8) | 37.2 (29.3–48.2) | 39.3 (30.1–52.4) |
| 60-day | 14.6 (13.1–16.5) | 18.4 (16.6–20.9) | 23.1 (20.7–26.2) | 26.5 (23.6-30.4) | 30.8 (26.7-36.2) | 33.8 (28.8-40.5) | 36.6 (30.5-44.7) | 39.2 (32.0-49.1) | 42.5 (33.5–55.1) | 44.9 (34.4–59.9) |

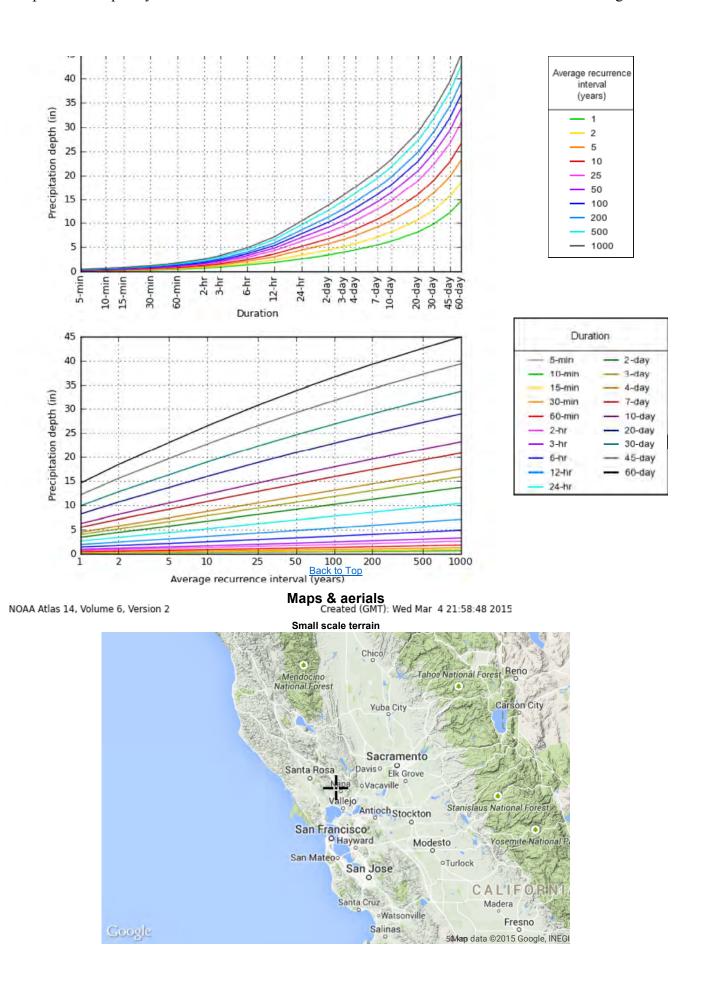
¹ Precipitation frequency (PF) estimates in this table are based on frequency analysis of partial duration series (PDS).

Numbers in parenthesis are PF estimates at lower and upper bounds of the 90% confidence interval. The probability that precipitation frequency estimates (for a given duration and average recurrence interval) will be greater than the upper bound (or less than the lower bound) is 5%. Estimates at upper bounds are not checked against probable maximum precipitation (PMP) estimates and may be higher than currently valid PMP values.

Please refer to NOAA Atlas 14 document for more information.

Back to Top

PF graphical









Back to Top

US Department of Commerce
National Oceanic and Atmospheric Administration
National Weather Service
Office of Hydrologic Development
1325 East West Highway
Silver Spring, MD 20910

Worksheet: Runoff Curve Number

| Project Anthem W | Ву | DJS | Date 6 | 5/7/2018 | |
|-----------------------------------|--|-----------------|-------------|----------------------|----------------------------|
| Location Watershed | 1 | Checked | PSW | Date 6 | 5/7/2018 |
| Subshed Existing Co | onditions | Check one: | ✓ Present | Dev | eloped |
| RUNOFF CURVE NUN | ABER | | | | |
| Soil name and hydrologic group | Cover description | .J., | CN (1) | Area ✓ acres ☐ mi2 | Product of CN x Area |
| (SCS book) | (cover type, treatment and hy condition; percent impervi | | (Table 2-2) | □ % | |
| 133-C | Impervious Area (Roads, roofs, etc | c.) | 98 | 0.06 | 5.88 |
| 133-C | Pasture Land (fair) | | 79 | 2.36 | 186.44 |
| 133-C | Woods (fair) | | 73 | 3.30 | 240.90 |
| 133-C | Row Crop - Straight (Poor) | - | 88 | 1.67 | 146.96 |
| | | | | | |
| | | | | | |
| | | | | | |
| | | | | | |
| | | | | | |
| | | | | | |
| (1) Use only one CN source | per line | | TOTAL: | 7.39 | 580.18 |
| CN (weighted) = - | $\frac{\text{total product}}{\text{total area}} = \frac{580.18}{7.39}$ | _ = | ; USE CN | 79 | |
| | 7.39 | | | | |

Worksheet: Time of Concentration (Tc) or travel time (Tt)

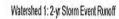
| Worksheet: Time of Concentration (To | By | и ши | | Date | (17/2010 |
|---|--------|-------------|--|------|-------------------|
| Attender willery | | · · | DJS | | 6/7/2018 |
| Location Project Site | Ch | ecked | PSW | Date | 6/7/2018 |
| Subshed Watershed 1 | Che | eck one: | : | nt | Developed |
| Note: Space for as many as two segments per flow type c be used for each worksheet. Include a map, schem or description of flow segments. | 11 h | eck one: | : 🗸 Тс | Т | t through subarea |
| SHEET FLOW (applicable to Tc only) | | | 10 (10 (10 (10 (10 (10 (10 (10 (10 (10 (| | |
| Segment ID | 1 | | | | |
| 1. Surface description (table 3-1) | Range | 9 | | | |
| 2. Manning's roughness coefficient, n (table3-1) | 0.13 | | | | |
| 3. Flow length, L (total L, 300 ft) ft | 266 | | | | |
| 4. Two-year 24-hour rainfall, P2 in | 3.4 | | | | |
| 5. Land slope, s ft/ft | 0.12 | | | | |
| 6. $T_t = \frac{0.007 \text{ (nL)}^{0.8}}{P_2^{0.5} \text{ s}^{0.4}}$ Compute T_t hr | 0.1509 | + | | | 0.1509 |
| SHALLOW CONCENTRATED FLOW | | | | | |
| Segment ID | 2 | | | | |
| 7. Surface description (paved or unpaved) | Unpave | ed | | | |
| 8. Flow length, L | 410 | | | | |
| 9. Watercourse slope, s ft/ft | 0.19 | | | | |
| 10. Average velocity, V (figure 3-1) ft/sec | 7.032 | 9 | | | |
| 11. $T_t = \frac{L}{3600 \text{ V}}$ Compute T_t hr | 0.0162 | + | | | 0.0162 |
| CHANNEL FLOW | | | | | |
| Segment ID | 3 | | | | |
| 12. Cross sectional flow area, a ft ² | 64 | | | | |
| 13. Wetted perimeter, pw | 32.98 | | | | |
| 14. Hydraulic radius, $r = \frac{a}{p_w}$ Compute r ft | 1.940 | 3 | | | |
| 15. Channel slope, s ft/ft | 0.03 | | | | |
| 16. Manning's roughness coefficient, n | 0.03 | | | | |
| 17. $V = \frac{1.49 r^{2/3} s^{-1/2}}{n}$ Compute V . ft/sec | 13.383 | 8 | | - | |
| 18. Flow length, L | 170 | | | | |
| 19. $T_t = \frac{L}{3600 \text{ V}}$ Compute T_t . hr | 0.0035 | + | | = | 0.0035 |
| $_{20.}$ Watershed or subarea $_{1c}$ or $_{1t}$ (add $_{1t}$ in steps 6, 11 an | d 19) | | | . hr | 0.1706 |

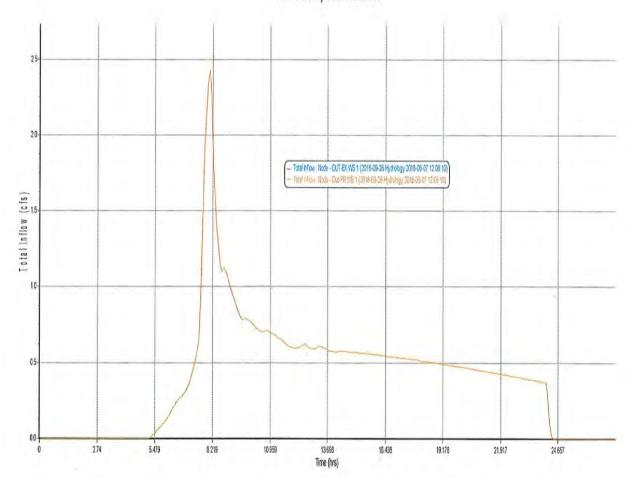
Worksheet: Runoff Curve Number

| Project Anthem W | Ву | DJS | Date 6 | 7/2018 | |
|-----------------------------------|--|------------|-------------|-----------------|----------------------------|
| ^{Location} Watershed | 1 | Checked | PSW | Date 6 | 7/2018 |
| Subshed Proposed (| Conditions | Check one: | Present | ✓ Deve | eloped |
| RUNOFF CURVE NUM | IBER | | | | |
| Soil name and hydrologic group | Cover description (cover type, treatment and hy | dragic | CN (1) | Area acres mi2 | Product of CN x Area |
| (SCS book) | condition; percent impervi | | (Table 2-2) | □ % | |
| 133-C | Impervious Area (Roads, roofs,ect. |) ** | 98 | 0.24 | 23.52 |
| 133-C | Row Crop- Striaght (Poor) | | 88 | 1.75 | 154.00 |
| 133-C | Pasture Land (fair) | | 79 | 2.10 | 165.90 |
| 133-C | Woods (fair) | | 73 | 3.30 | 240.90 |
| | | | | | |
| | | | | | |
| (1) Use only one CN source | рет ше | | TOTAL: | 7.39 | 584.32 |
| CN (weighted) = | $\frac{\text{total product}}{\text{total area}} = \frac{584.32}{7.39}$ | _ = | ; USE CN | 79 | |

Worksheet: Time of Concentration (Tc) or travel time (Tt)

| Project Anthem Winery | Ву | DJS | Date | 6/7/2018 |
|--|------------|-----------|----------|-----------------|
| Location Project Site | Checked | l PSW | Date | 6/7/2018 |
| Subshed Watershed 1 | Check or | ne: Prese | ent | ✓ Developed |
| name Note: Space for as many as two segments per flow type ca be used for each worksheet. Include a map, scheme or description of flow segments. | II hack of | ne: 🗸 Tc | Tt | through subarea |
| SHEET FLOW (applicable to Tc only) | | | | |
| Segment ID | 1 | T | | |
| 1. Surface description (table 3-1) | Range | | | |
| 2. Manning's roughness coefficient, n (table3-1) | 0.13 | | | |
| 3. Flow length, L (total L, 300 ft) ft | 266 | | | |
| 4. Two-year 24-hour rainfall, P2 in | 3.4 | | | |
| 5. Land slope, s ft/ft | 0.12 | | | |
| 6. $T_t = \frac{0.007 \text{ (nL)}^{0.8}}{P_2^{0.5} \text{ s}^{0.4}}$ Compute T_t . hr | 0.1509 | + | | 0.1509 |
| SHALLOW CONCENTRATED FLOW | | | | |
| Segment ID | 2 | | | |
| 7. Surface description (paved or unpaved) | Unpaved | | | |
| 8. Flow length, L ft | 410 | | | |
| 9. Watercourse slope, s ft/ft | 0.19 | | | |
| 10. Average velocity, V (figure 3-1) ft/sec | 7.0329 | | | |
| 11. $T_t = \frac{L}{3600 \text{ V}}$ Compute T_t . hr | 0.0162 | + | | 0.0162 |
| CHANNEL FLOW | | | | |
| Segment ID | 3 | | | |
| 12. Cross sectional flow area, a ft ² | 64 | | | |
| 13. Wetted perimeter, pw ft | 32.98 | | | |
| 14. Hydraulic radius, $r = \frac{a}{p_w}$ Compute r ft | 1.9406 | | | |
| 15. Channel slope, s ft/ft | 0.03 | | | |
| 16. Manning's roughness coefficient, n | 0.03 | | | |
| 17. $V = \frac{1.49 r^{2/3} s^{1/2}}{n}$ Compute V. ft/sec | 13.3838 | | | |
| 18. Flow length, L | 170 | | | |
| 19. $T_t = \frac{L}{3600 V}$ Compute T_t . hr | 0.0035 | + | | 0.0035 |
| $_{20.}$ Watershed or subarea $_{c}$ or $_{t}$ (add $_{t}$ in steps 6, 11 and | d 19) | | . hr | 0.1706 |

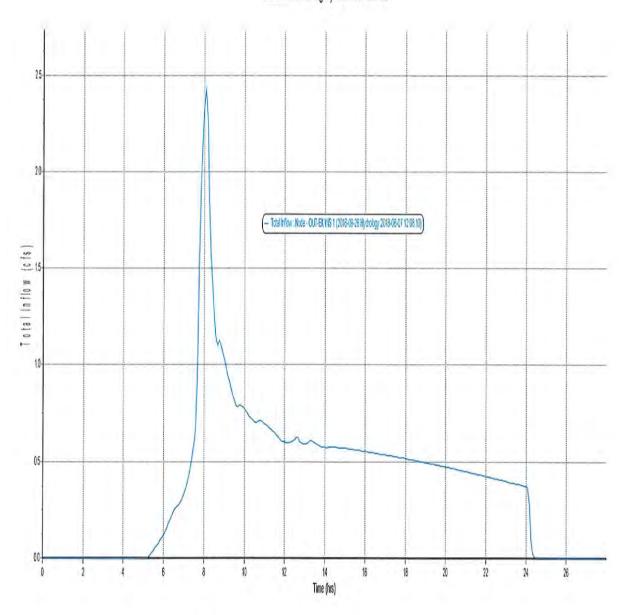




Existing Watershed Runoff Results

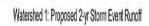
| Total Rainfall (in) | Total Rainfall (in)3.40 |
|------------------------|-------------------------|
| Peak Runoff (cfs)2.43 | Peak Runoff (cfs) |
| Time to Peak (hrs)8.03 | Time to Peak (hrs) |

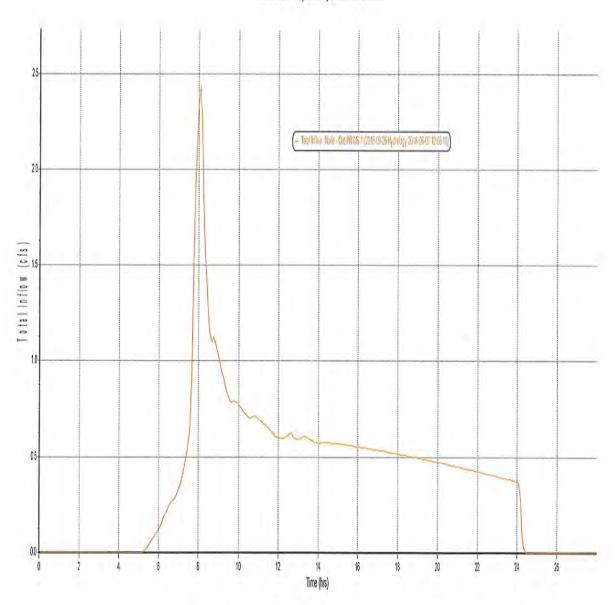
Watershed 1: Existing 2-yr Storm Event Runoff



Existing Watershed Runoff Results

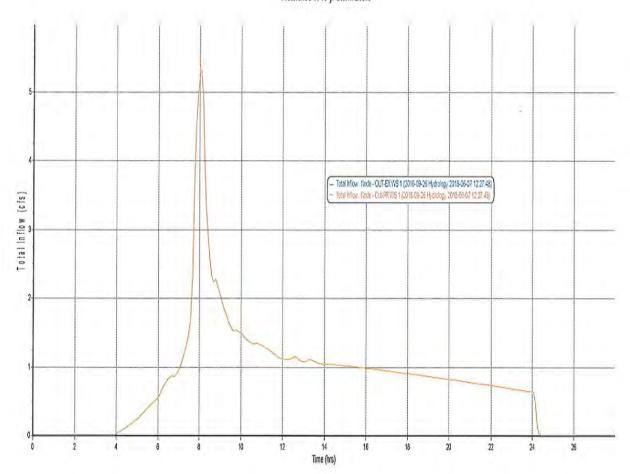
| Watershed | CN | Tc (mins) | Area (Ac.) | Total Rainfall (in.) | Time to Peak (hrs.) | Peak Runoff (cfs) |
|-----------|----|-----------|------------|----------------------|------------------------|-------------------|
| 1 | 79 | 10.24 | 7.39 | 3.40 | 8.03 | 2.43 |





| Watershed | CN | Tc (mins) | Area (Ac.) | Total Rainfall (in.) | Time to Peak (hrs.) | Peak Runoff (cfs) |
|-----------|----|-----------|------------|----------------------|------------------------|----------------------|
| 1 | 79 | 10.24 | 7.39 | 3.40 | 8.03 | 2.43 |

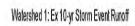


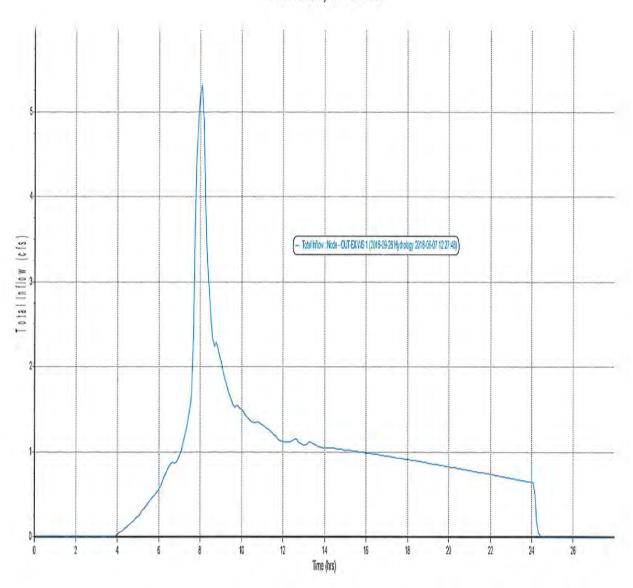


Existing Watershed Runoff Results

Time to Peak (hrs)8.04

| Total Rainfall (in) | 5.17 |
|---------------------|------|
| Peak Runoff (cfs) | 5,32 |
| Time to Peak (hrs) | 8.04 |

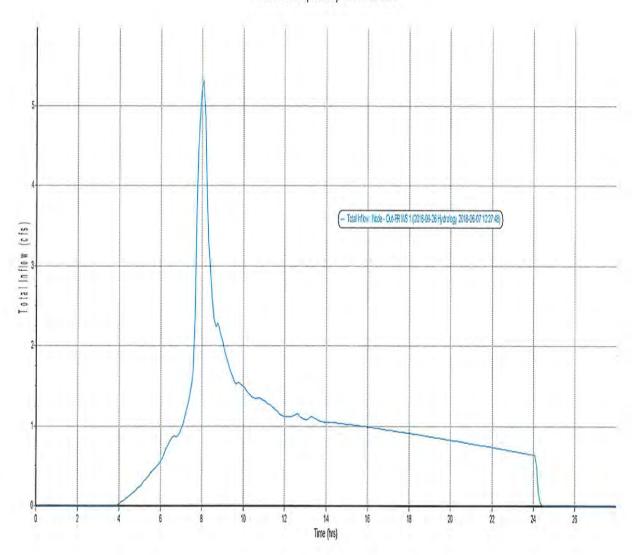




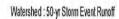
Existing Watershed Runoff Results

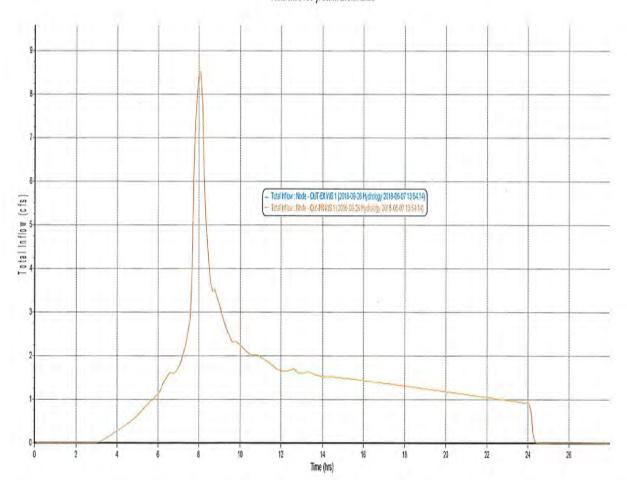
| Watershed | CN | Tc (mins) | Area (Ac.) | Total Rainfall (in.) | Time to Peak (hrs.) | Peak Runoff (cfs) |
|-----------|----|-----------|------------|----------------------|------------------------|----------------------|
| 1 | 79 | 10.24 | 7.39 | 5.17 | 8.06 | 5.32 |

Watershed 1: Proposed 10-yr Storm Event Runoff



| Watershed | CN | Tc (mins) | Area (Ac.) | Total Rainfall (in.) | Time to Peak (hrs.) | Peak Runoff (cfs) |
|-----------|----|-----------|------------|----------------------|------------------------|----------------------|
| 1 | 79 | 10.24 | 7.39 | 5.17 | 8.10 | 5.32 |

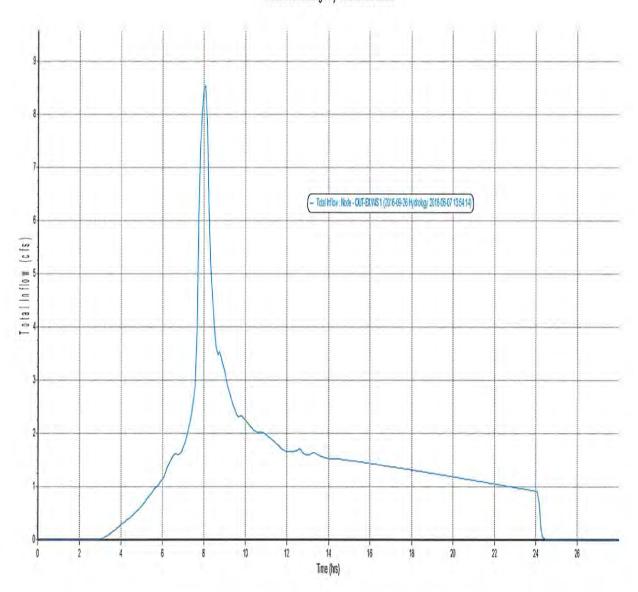




Existing Watershed Runoff Results

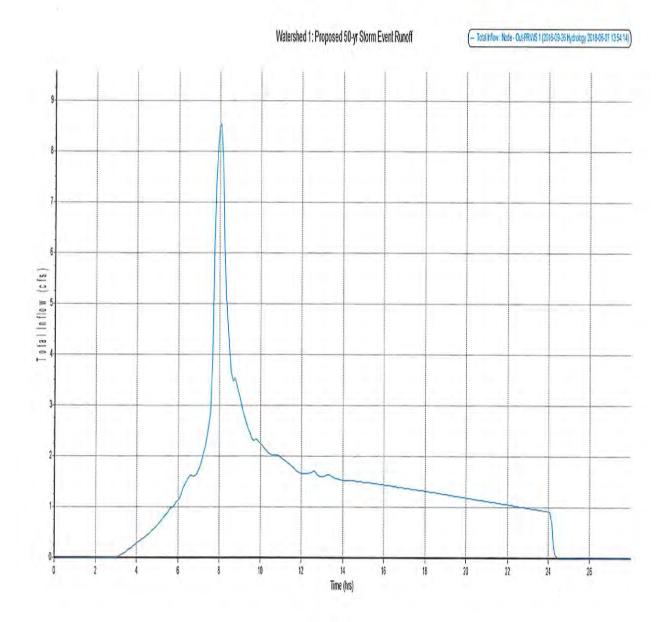
| Total Rainfall (in) | Total Rainfall (in)6.99 |
|-------------------------|-------------------------|
| Peak Runoff (cfs)8.54 | Peak Runoff (cfs)8.54 |
| Time to Peak (hrs) 8.57 | Time to Peak (hrs) 8.08 |

Watershed: Existing 50-yr Storm Event Runoff



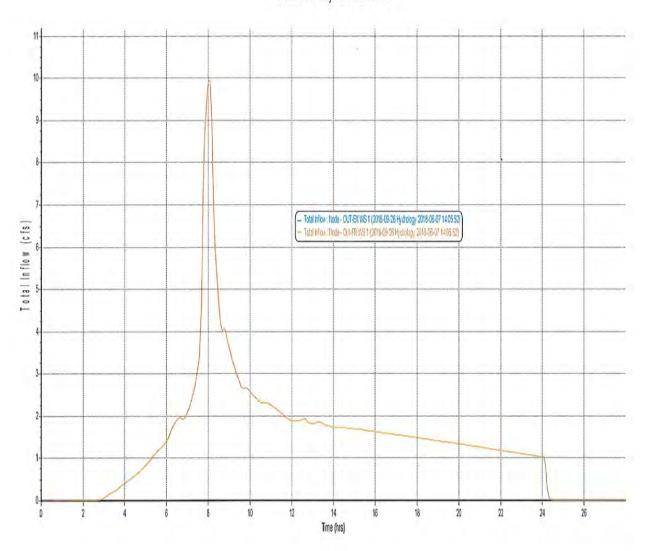
Existing Watershed Runoff Results

| Sub-shed | CN | Tc (mins) | Area (Ac.) | Total Rainfall (in.) | Time to Peak (hrs.) | Peak Runoff (cfs) |
|----------|----|-----------|------------|----------------------|------------------------|----------------------|
| 1 | 79 | 10.24 | 7.39 | 6.99 | 8.08 | 8.54 |



| Watershed | CN | Tc (mins) | Area (Ac.) | Total Rainfall (in.) | Time to Peak (hrs.) | Peak Runoff (cfs) |
|-----------|----|-----------|------------|----------------------|------------------------|----------------------|
| 1 | 79 | 10.24 | 7.31 | 6.99 | 8.10 | 8.54 |

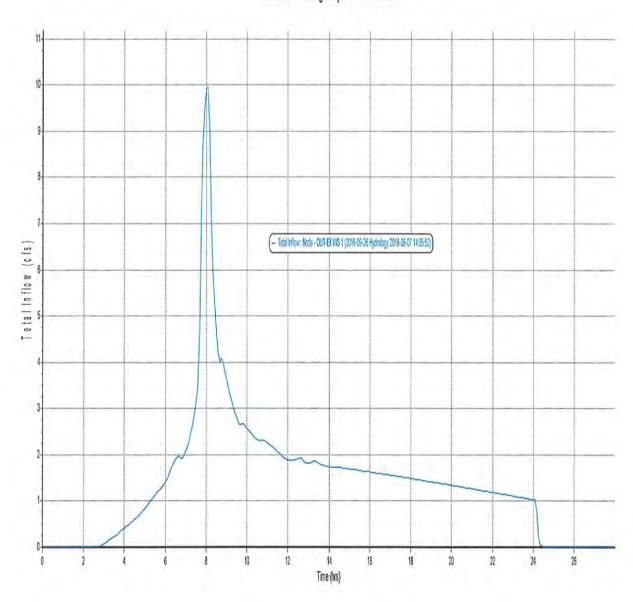
Watershed 1: 100-yr Storm Event Runoff



Existing Watershed Runoff Results

| Total Rainfall (in) | Total Rainfall (in)7.77 |
|-------------------------|-------------------------|
| Peak Runoff (cfs)9.95 | Peak Runoff (cfs)9.95 |
| Time to Peak /hrs\ 8.06 | Time to Peak (hrs) 8 06 |

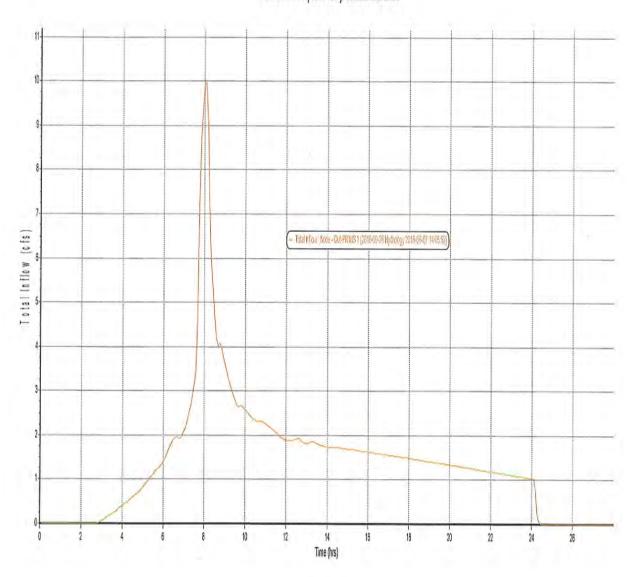
Watershed 1: Existing 100-yr Storm Event Runoff



Existing Watershed Runoff Results

| Sub-shed | CN | Tc (mins) | Area (Ac.) | Total Rainfall (in.) | Time to Peak (hrs.) | Peak Runoff (cfs) |
|----------|----|-----------|------------|----------------------|------------------------|----------------------|
| 1B | 79 | 10.24 | 7.39 | 7.77 | 8.06 | 9.95 |

Watershed 1: Proposed 100-yr Storm Event Runoff



| Sub-shed | CN | Tc (mins) | Area (Ac.) | Total Rainfall (in.) | Time to Peak (hrs.) | Peak Runoff (cfs) |
|----------|----|-----------|------------|----------------------|------------------------|----------------------|
| 1 | 79 | 10.24 | 7.39 | 7.77 | 8.08 | 9.95 |

Worksheet: Runoff Curve Number

| Project Anthem W | inery | Ву | DJS | Date 9 | /28/2016 |
|-----------------------------------|---|------------|-------------|------------------|----------------------------|
| Location Watershed | 2A | Checked | PSW | Date 9 | /28/2016 |
| Subshed Existing Co | onditions | Check one: | ✓ Present | Dev | reloped |
| RUNOFF CURVE NUM | MBER | | | | |
| Soil name and hydrologic group | Cover description | | CN (1) | Area acres mi2 | Product of CN x Area |
| (SCS book) | (cover type, treatment and hy condition; percent impervi | | (Table 2-2) | | |
| 133-C | Impervious Area (Roads, roofs, etc | o.) | 98 | 0.12 | 11.76 |
| 133-C | Pasture Land (fair) | | 79 | 0.37 | 29.23 |
| 133-C | Row Crops- Straight (poor) | | 88 | 0.22 | 19.36 |
| 133-C | Woods (fair) | | 73 | 0.85 | 62.05 |
| | | | | | |
| | | | | | |
| | | | | | |
| | | | - | | |
| | | | | | |
| | | | | | |
| | · | | | | |
| | | | | | |
| | | | | | |
| | | | | | |
| (1) Use only one CN source | per line | | TOTAL: | 1.56 | 122.40 |
| CN (weighted) =- | total product = 122.40 total area 1.56 | = | ; USE CN | 78 | |
| | total area 1.56 | | | | |

Worksheet: Time of Concentration (Tc) or travel time (Tt)

| Project Anthem Winery | Ву | DJS | Date 9/28/2016 |
|---|-----------|-----------|--------------------|
| Location Project Site | Checked | PSW | Date 9/28/2016 |
| Subshed Watershed 2A | Check one | : Preser | nt Developed |
| Note: Space for as many as two segments per flow type can be used for each worksheet. Include a map, schematic or description of flow segments. | Check one | :: 🗸 Тс | Tt through subarea |
| SHEET FLOW (applicable to Tc only) | | | |
| Segment ID | 1 | 1, | |
| 1. Surface description (table 3-1) | Range | | |
| 2. Manning's roughness coefficient, n (table3-1) | 0.13 | | |
| 3. Flow length, L (total L, 300 ft) ft | 288 | | |
| 4. Two-year 24-hour rainfall, P2 in | 3.4 | | |
| 5. Land slope, s | 0.23 | | |
| 6. $T_t = \frac{0.007 \text{ (nL)}^{0.8}}{P_2^{0.5} \text{ s}^{0.4}}$ Compute T_t hr |).1240 + | - | = 0.1240 |
| SHALLOW CONCENTRATED FLOW | | | |
| Segment ID | | | |
| 7. Surface description (paved or unpaved) | | | |
| 8. Flow length, L | | | |
| 9. Watercourse slope, s ft/ft | | | |
| 10. Average velocity, V (figure 3-1) ft/sec | | | |
| 11. $T_t = \frac{L}{3600 \text{ V}}$ Compute T_t . hr | + | - | = |
| CHANNEL FLOW | | | |
| . Segment ID | | | |
| 12. Cross sectional flow area, a ft ² | | | |
| 13. Wetted perimeter, pw ft | | - | |
| 14. Hydraulic radius, $r = \frac{a}{p_w}$ Compute r ft | | | |
| 15. Channel slope, s ft/ft | | | |
| 16. Manning's roughness coefficient, n | | | |
| 18. Flow length, L ft | | , , | |
| 19. $T_t = \frac{L}{3600 \text{V}}$ Compute T_t . hr | + | | = |
| 20. Watershed or subarea To or Tt (add Tt in steps 6, 11 and 19) | | | . hr 0.1240 |

Worksheet: Runoff Curve Number

| Project Anthem W | inery | Ву | DJS | Date | 9/26/2016 |
|-----------------------------------|---|------------|-------------|-------------|----------------------------|
| Location Watershed 2B Chec | | Checked | PSW | Date o | 9/26/2016 |
| Subshed Existing Co | onditions | Check one: | ✓ Present | De | veloped |
| RUNOFF CURVE NUM | ABER 1 | | | | |
| Soil name and hydrologic group | Cover description | | CN (1) | Area acres | Product of CN x Area |
| (SCS book) | (cover type, treatment and hyd condition; percent impervio | | (Table 2-2) | | |
| 133-C | Impervious Area (Roads, roofs, etc. | .) | 98 | 0.20 | 19.60 |
| 133-C | Pasture Land (fair) | | 79 | 0.79 | 62.41 |
| 133-C | Woods (fair) | | 73 | 1.94 | 141.62 |
| 136-C | Impervious Area (Roads, roofs, etc. | .) | 98 | 0.39 | 38.22 |
| 136-C | Pasture Land (fair) | | 79 | 1.05 | 82.95 |
| 136-C | Woods (fair) | | 73 | 4.61 | 336.53 |
| | | | | | |
| | | | | | |
| | | | | | |
| (1) Use only one CN source | per line | | TOTAL | 8.98 | 681.33 |
| | | | TOTAL: | | 1 |
| CN (weighted) =- | total product = 681.33 total area 8.98 | | ; USE CN | 76 |] |

Worksheet: Time of Concentration (Tc) or travel time (Tt)

| Project Anthem Winery |) 02 42 | By | DIC | Date | 0/00/001/ |
|---|---------|-----------|------------|------------|-------------------|
| 7 .: | | | DJS | | 9/28/2016 |
| Location Project Site | | Checked | PSW | Date | 9/28/2016 |
| Subshed Watershed 2B | | Check one | e: ✓ Prese | nt | Developed |
| Note: Space for as many as two segments per flow type can be used for each worksheet. Include a map, scheme | | Check one | e: 🗸 Tc | □т | t through subarea |
| or description of flow segments. | auc | | | | |
| SHEET FLOW (applicable to Tc only) | | | | | |
| | | 4 | | | |
| Segment ID | | 1 | | | |
| 1. Surface description (table 3-1) | | nge | | | |
| 2. Manning's roughness coefficient, n (table3-1) | | 13 | | | |
| 3. Flow length, L (total L, 300 ft) ft | | 77 | | | |
| 4. Two-year 24-hour rainfall, P2 | | .4 | | | |
| 5. Land slope, s | | 13 | <u> </u> | | |
| 6. $T_t = \frac{0.007 \text{ (nL)}^{0.8}}{P_2^{0.5} \text{ s}^{0.4}}$ Compute T_t . hr | 0.15 | 10 - | F | | 0.1510 |
| SHALLOW CONCENTRATED FLOW | | | | | |
| Γ | | | | | |
| Segment ID | | 2 | | | |
| 7. Surface description (paved or unpaved) | | aved | | | |
| 8. Flow length, L | | 18 | | | |
| 9. Watercourse slope, s ft/ft | | 13 | | | |
| 10. Average velocity, V (figure 3-1) ft/sec | | 174 | | | |
| 11. $T_t = \frac{L}{3600 \text{ V}}$ Compute T_t . hr | 0.03 | 91 + | + | = | 0.0391 |
| CHANNEL FLOW | | | | cue Car | |
| Segment ID | | 3 | | | |
| 12. Cross sectional flow area, a ft ² | 1. | 76 | | | |
| 13. Wetted perimeter, pw ft | 4. | | | | |
| 14. Hydraulic radius, $r = \frac{a}{p_w}$ Compute r ft | 0.3 | | | | |
| 15. Channel slope, s ft/ft | 0. | | | | · |
| 16. Manning's roughness coefficient, n | | 112 | | | |
| 17. $V = \frac{1.49 r^{2/3} s^{1/2}}{n}$ Compute V . ft/sec | | 147 | | | |
| 18. Flow length, L | | 72 | | | |
| 19. $T_t = \frac{L}{3600 \text{ V}}$ Compute T_t . hr | 0.00 | | | | 0.0021 |
| 20. Watershed or subarea T _c or T _t (add T _t in steps 6, 11 and | | | | l . hr | 0.1922 |
| | | | | | |

Worksheet: Runoff Curve Number

| Project Anthem W | inery | Ву | DJS | Date 6 | 5/7/2018 |
|-----------------------------------|--|------------|-------------|----------------------|----------------------------|
| Location Watershed | 2A | Checked | PSW | Date 6 | 5/7/2018 |
| Subshed Proposed C | Conditions | Check one: | Present | ✓ Deve | eloped |
| RUNOFF CURVE NUM | 1BER | | | | |
| Soil name and hydrologic group | Cover descript | | CN (1) | Area ✓ acres ☐ mi2 | Product of CN x Area |
| (SCS book) | (cover type, treatment a condition; percent im | | (Table 2-2) | <u> </u> | |
| 133-C | Impervious Area (Roads, roof | fs, etc.) | 98 | 0.98 | 96.04 |
| 133-C | Pasture Land (fair) | | 79 | 0.46 | 36.34 |
| 133-C | Row Crops- Straight (poor) | | 88 | 0.12 | 10.56 |
| | | | | | |
| | | | | | |
| | | | | | |
| | | | | | |
| | | | | | |
| | | | | | |
| | | | | | |
| | | | | | · |
| | | | | | |
| (1) Use only one CN source | per line | | TOTAL: | 1.56 | 142.94 |
| CN (weighted) = - | $\frac{\text{total product}}{\text{total area}} = \frac{142.94}{1.56}$ | = | ; USE CN | 92 | |

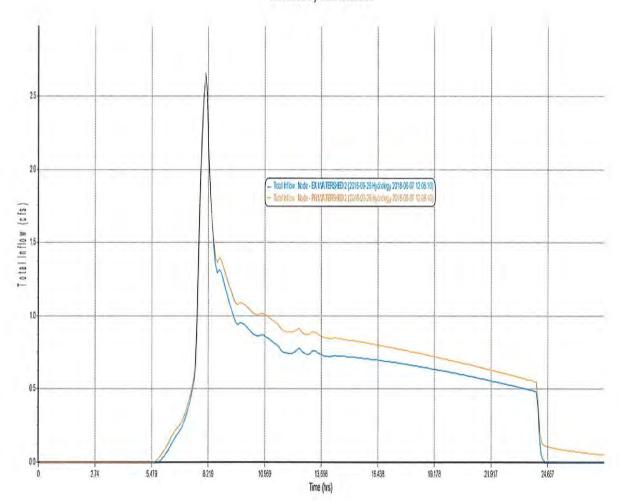
Worksheet: Runoff Curve Number

| Project Anthem W | inery | | Ву | DJS | Date | 9/26/2016 |
|--------------------------------|--------------------|-------------------------------------|------------|-------------|-----------------|----------------------------|
| Location Watershed 2B | | | Checked | PSW | Date | 9/26/2016 |
| Subshed Proposed C | Conditions | | Check one: | Present | √ De | veloped |
| RUNOFF CURVE NUA | MBER | | | | | |
| Soil name and hydrologic group | | ver description treatment and hy | ⁄drogic | CN (1) | Area acres mi2 | Product of CN x Area |
| (SCS book) | | ; percent impervi | | (Table 2-2) | <u></u> % | |
| 133-C | Impervious Area (| Roads, roofs, etc | o.) | 98 | 0.66 | 64.68 |
| 133-C | Pasture Land (fair |) | | 79 | 0.69 | 54.51 |
| 133-C | Row Crops- Straig | ht (poor) | | 88 | 0.09 | 7.92 |
| 133-C | Woods (fair) | | | 73 | 2.02 | 147.46 |
| 136-C | Impervious Area (I | Roads, roofs, etc | c.) | 98 | 0.49 | 48.02 |
| 136-C | Pasture Land (fair | | | 79 | 0.95 | 75.05 |
| 136-C | Woods (fair) | | | 73 | 4.08 | 297.84 |
| | | | | | | |
| (1) Use only one CN source | per line | | | TOTAL: | 8.98 | 695,48 |
| CN (weighted) = - | total product = | 695.48 8.98 | _ = | ; USE CN | 77 |] |

Worksheet: Time of Concentration (Tc) or travel time (Tt)

| Wolfield of Golfeella water (16) | , or draver dar | 10 (11) | | |
|--|-----------------|--------------|------|-------------------|
| Project Anthem Winery | Ву | DJS | Date | 9/28/2016 |
| Location Project Site | Checked | PSW | Date | 9/28/2016 |
| Subshed Watershed 2B | Check or | ne: Prese | nt | ✓ Developed |
| Note: Space for as many as two segments per flow type can be used for each worksheet. Include a map, schemator description of flow segments. | | e: 🗸 Tc | Пт | t through subarea |
| SHEET FLOW (applicable to Tc only) | | | | |
| Segment ID | 1 | 1 | | |
| 1. Surface description (table 3-1) | Range | Smooth Surfa | ace | |
| 2. Manning's roughness coefficient, n (table3-1) | 0.13 | 0.011 | | |
| 3. Flow length, L (total L, 300 ft) ft | 160 | 117 | | • |
| 4. Two-year 24-hour rainfall, P2 in | 3.4 | 3.4 | | |
| 5. Land slope, s ft/ft | 0.13 | 0.02 | | |
| 6. $T_t = \frac{0.007 \text{ (nL)}^{0.8}}{P_2^{0.5} \text{ s}^{0.4}}$ Compute T_t . hr | 0.0973 | + 0.0222 | = | 0.1195 |
| SHALLOW CONCENTRATED FLOW | | | | |
| Segment ID | 2 | | | |
| 7. Surface description (paved or unpaved) | unpaved | | | |
| 8. Flow length, L ft | 818 | | | |
| 9. Watercourse slope, s ft/ft | 0.13 | | | |
| 10. Average velocity, V (figure 3-1) ft/sec | 5.8174 | | | |
| 11. $T_t = \frac{L}{3600 \text{ V}}$ Compute T_t . hr | 0.0391 | + | = | 0.0391 |
| CHANNEL FLOW | | | | |
| Segment ID | 3 | | | |
| 12. Cross sectional flow area, a ft ² | 1.76 | | | |
| 13. Wetted perimeter, pw | 4.71 | | | |
| 14. Hydraulic radius, $r = \frac{a}{p_w}$ Compute r ft | 0.3737 | | | |
| 15. Channel slope, s ft/ft | 0.12 | | | |
| 16. Manning's roughness coefficient, n | 0.012 | | | |
| 17. $V = \frac{1.49 r^{2/3} s^{1/2}}{n}$ Compute V . ft/sec | 22.3147 | | | |
| 18. Flow length, L | 172 | | | |
| 19. $T_t = \frac{L}{3600 \text{ V}}$ Compute T_t . hr | | + | = | 0.0021 |
| 20. Watershed or subarea Tc or Tt (add Tt in steps 6, 11 and | t 19) | | . hr | 0.1607 |

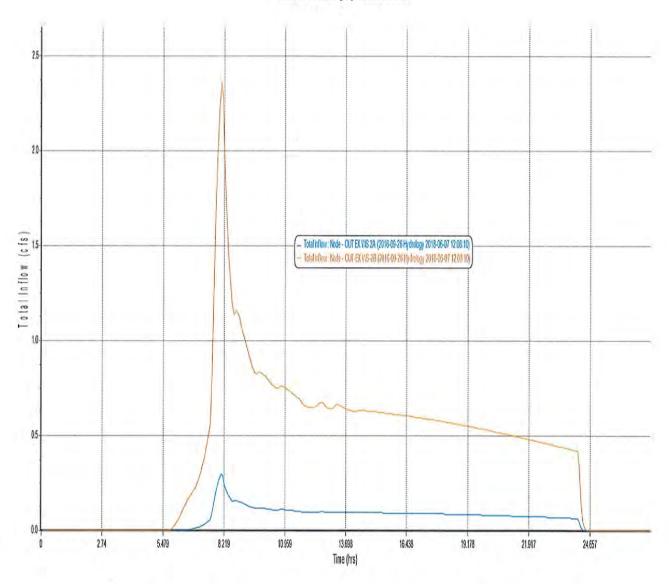




Existing Sub-shed Runoff Results

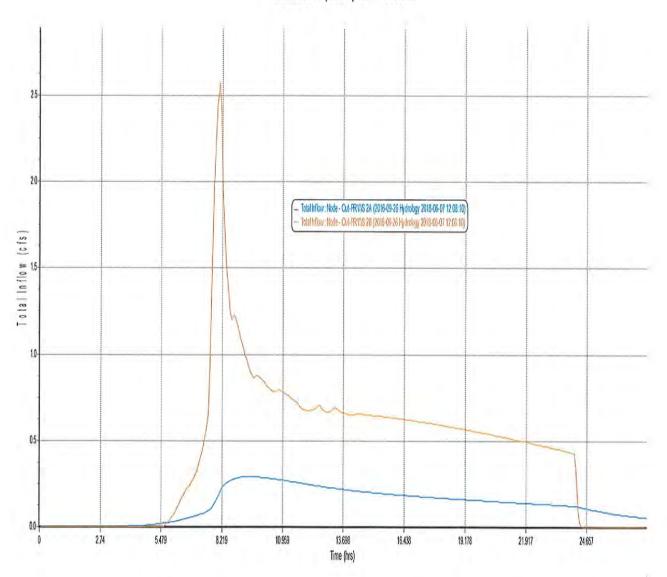
| Total Rainfall (in) | Total Rainfall (in)3.40 |
|------------------------|-------------------------|
| Peak Runoff (cfs)2.65 | Peak Runoff (cfs)2.65 |
| Time to Peak (hrs)8.08 | Time to Peak (hrs) |

Watershed 2: Existing 2-yr Storm Event Runoff



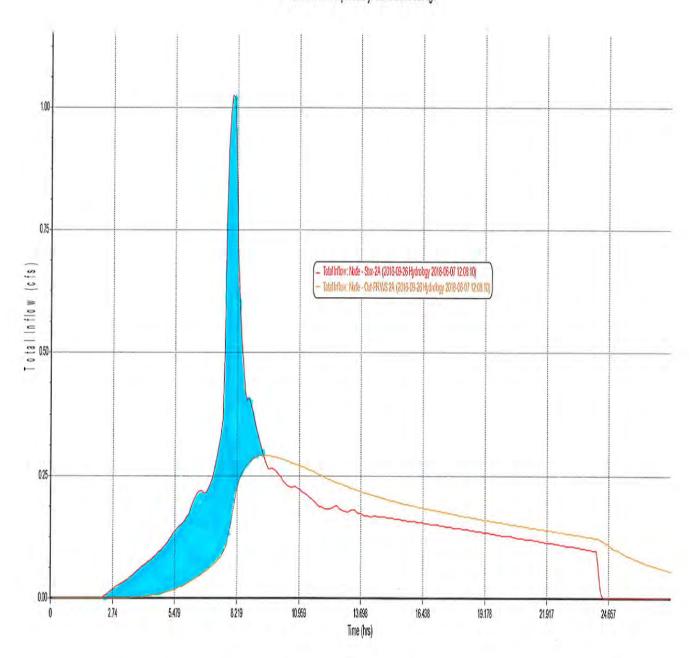
| Sub-shed | CN | Tc (mins) | Area (Ac.) | Total Rainfall (in.) | Time to Peak (hrs.) | Peak Runoff (cfs) |
|----------|----|-----------|------------|----------------------|------------------------|-------------------|
| 2A | 78 | 7.44 | 1.56 | 3.40 | 8.08 | 0.30 |
| 2B | 76 | 11.53 | 8.98 | 3.40 | 8.06 | 2.36 |

Watershed 2: Proposed 2-yr Storm Event Runoff



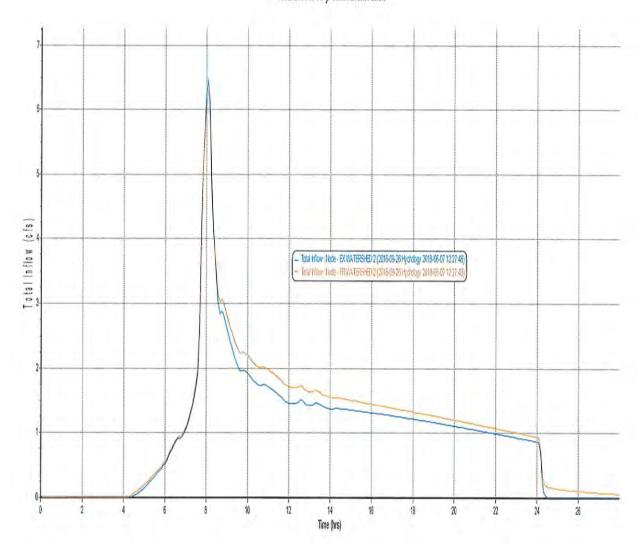
| Sub-shed | CN | Tc (mins) | Area (Ac.) | Total Rainfall (in.) | Time to Peak (hrs.) | Peak Runoff (cfs) |
|----------|----|-----------|------------|----------------------|------------------------|----------------------|
| 2A | 92 | 6.00 | 1.56 | 3.40 | 9.83 | 0.29 |
| 2B | 77 | 9.64 | 8.98 | 3.40 | 8.08 | 2.57 |

Watershed 2: Proposed 2-yr Storm Event Storage



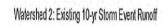
Storage......5,380 cu. ft.

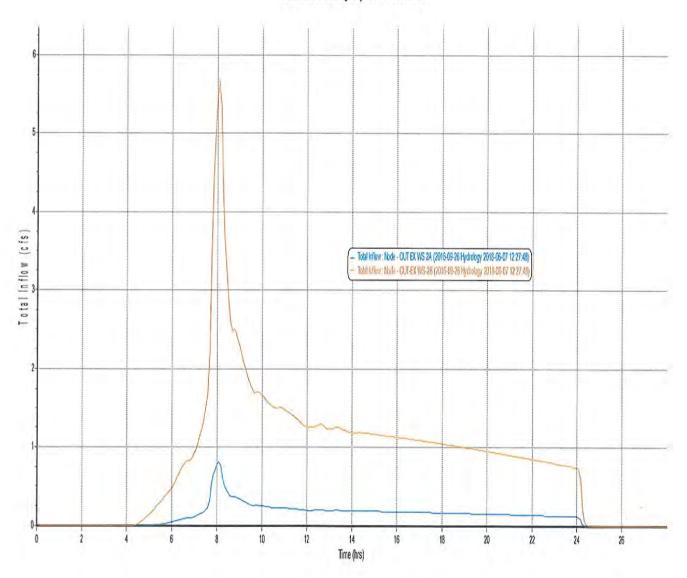
Watershed 2:10-yr Storm Event Runoff



Existing Sub-shed Runoff Results

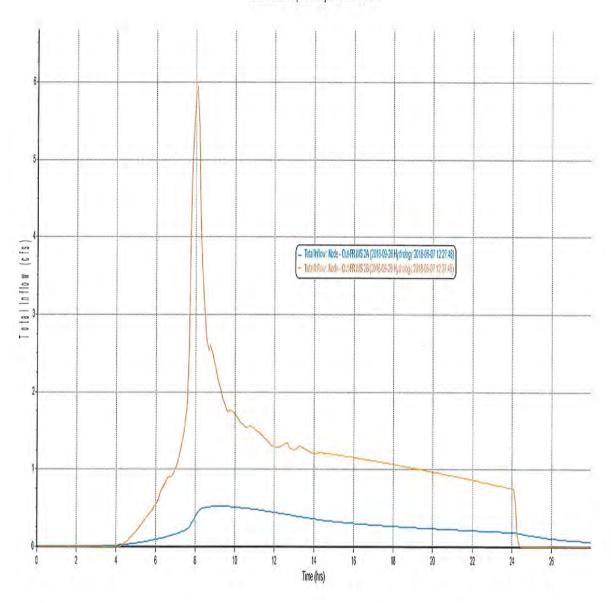
| Total Rainfall (in) | Total Rainfall (in)5.17 |
|---------------------|-------------------------|
| Peak Runoff (cfs) | Peak Runoff (cfs)6.29 |
| Time to Peak (hrs) | Time to Peak (hrs) 8 11 |





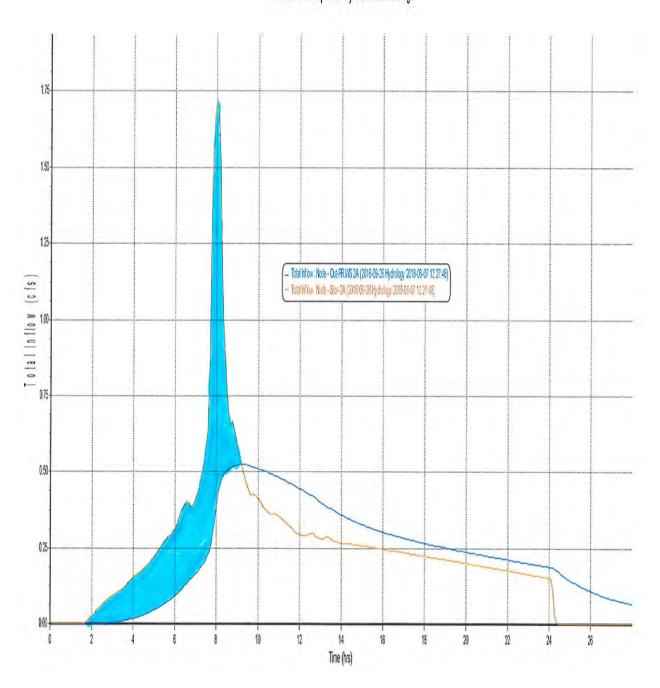
| Sub-shed | CN | Tc (mins) | Area (Ac.) | Total Rainfall (in.) | Time to Peak (hrs.) | Peak Runoff (cfs) |
|----------|----|-----------|------------|----------------------|------------------------|----------------------|
| 2A | 78 | 7.44 | 1.56 | 5.17 | 8.08 | 0.81 |
| 2B | 76 | 11.53 | 8.98 | 5.17 | 8.06 | 5.65 |

Watershed 2: Proposed 10-yr Storm Event Runoff

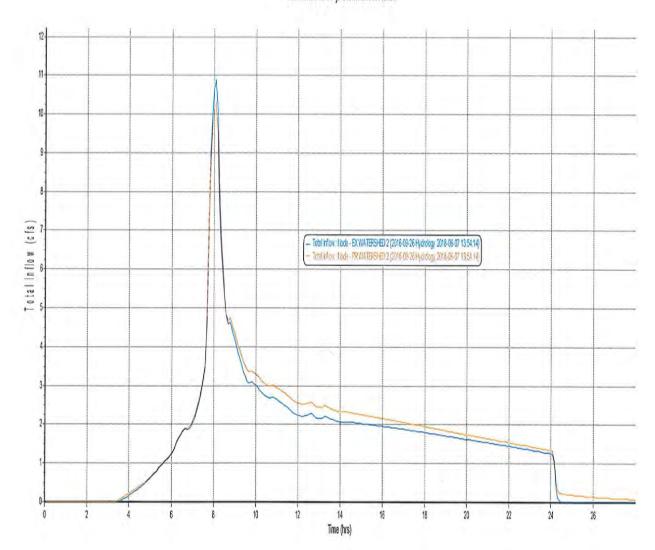


| Sub-shed | CN | Tc (mins) | Area (Ac.) | Total Rainfall (in.) | Time to Peak (hrs.) | Peak Runoff (cfs) |
|----------|----|-----------|------------|----------------------|------------------------|----------------------|
| 2A | 92 | 6.00 | 1.56 | 5.17 | 10.08 | 0.52 |
| 2B | 77 | 9.64 | 8.98 | 5.17 | 8.08 | 5.94 |

Watershed 2: Proposed 10-yr Storm Event Storage



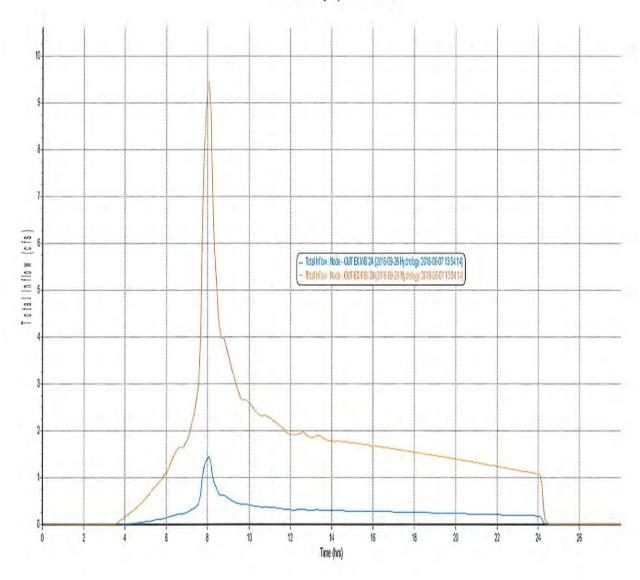
Watershed 2: 50-yr Storm Event Runoff



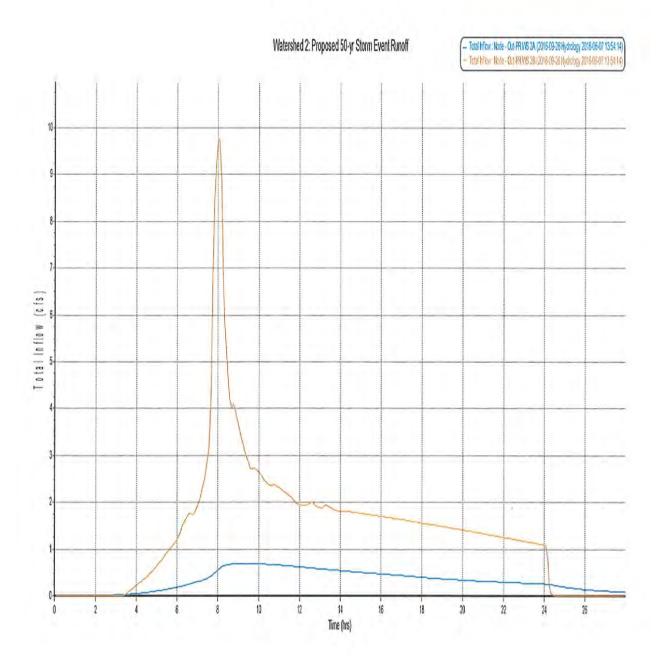
Existing Sub-shed Runoff Results

| Total Rainfall (in) | Total Rainfall (in)6.99 |
|-------------------------|-------------------------|
| Peak Runoff (cfs) | Peak Runoff (cfs)10.32 |
| Time to Peak (hrs) 8.08 | Time to Peak (hrs) 8 11 |

Watershed 2: Existing 50-yr Storm Event Runoff

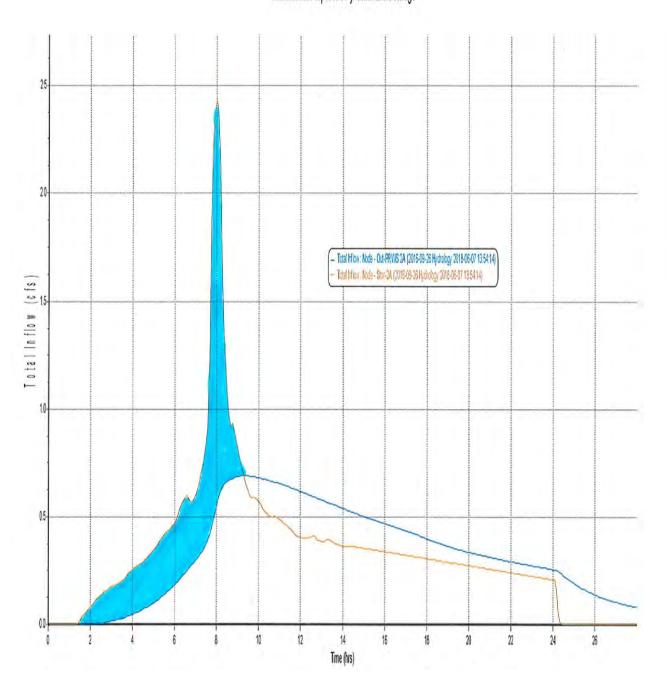


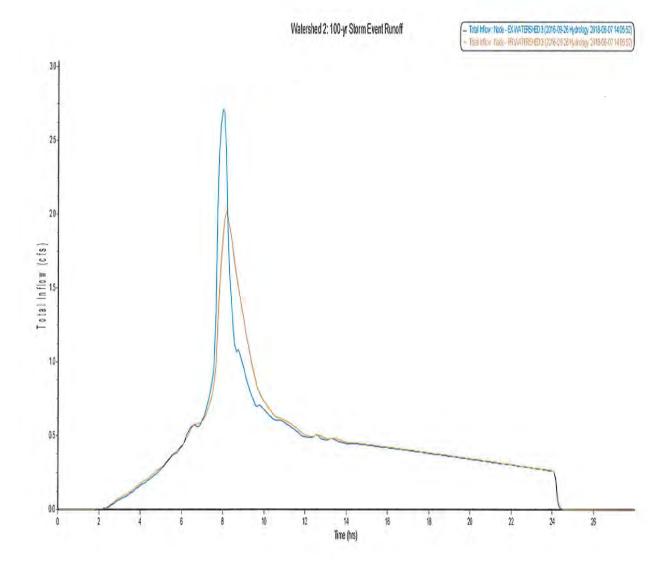
| Sub-shed | CN | Tc (mins) | Area (Ac.) | Total Rainfall (in.) | Time to Peak (hrs.) | Peak Runoff (cfs) |
|----------|----|-----------|------------|----------------------|------------------------|----------------------|
| 2A | 78 | 7.44 | 1.56 | 6.99 | 8.08 | 1.44 |
| 2B | 76 | 11.53 | 8.98 | 6.99 | 8.06 | 9.44 |



| Sub-shed | CN | Tc (mins) | Area (Ac.) | Total Rainfall (in.) | Time to Peak (hrs.) | Peak Runoff (cfs) |
|----------|----|-----------|------------|----------------------|------------------------|----------------------|
| 2A | 92 | 6.00 | 1.56 | 6.99 | 10.08 | 0.69 |
| 2B | 77 | 9.64 | 8.98 | 6.99 | 8.08 | 9.77 |

Watershed 2: Proposed 50-yr Storm Event Storage

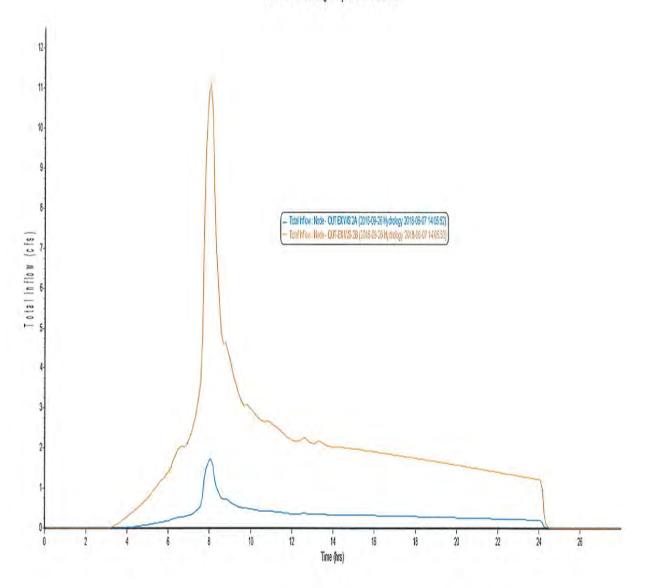




Existing Sub-shed Runoff Results

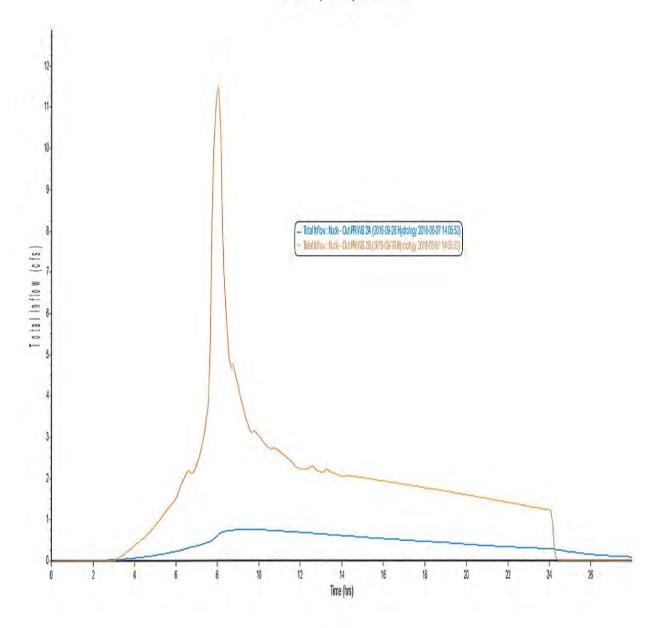
| Total Rainfall (in) | Total Rainfall (in)7.77 |
|-------------------------|-------------------------|
| Peak Runoff (cfs)12.84 | Peak Runoff (cfs)12.0 |
| Time to Peak (hrs) 8 05 | Time to Peak /hrs\ |

Watershed 2: Existing 100-yr Storm Event Runoff



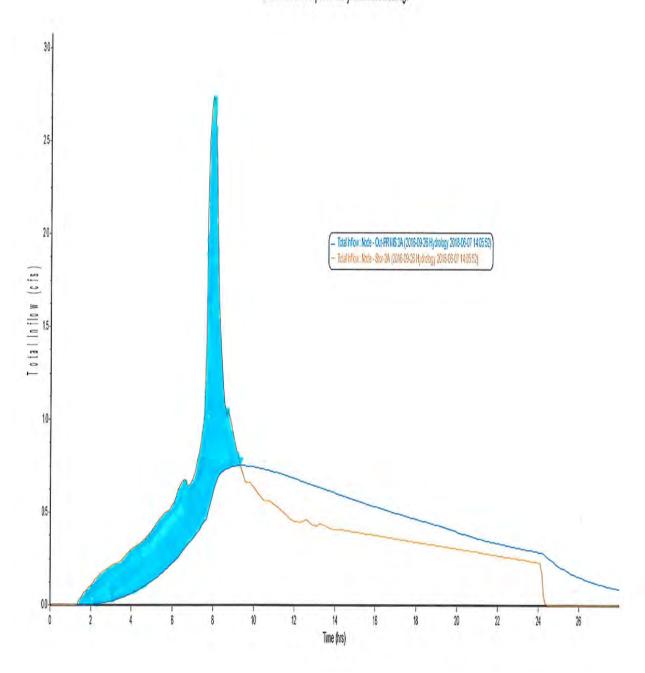
| Sub-shed | CN | Tc (mins) | Area (Ac.) | Total Rainfall (in.) | Time to Peak (hrs.) | Peak Runoff (cfs) |
|----------|----|-----------|------------|----------------------|------------------------|-------------------|
| 2A | 78 | 7.44 | 1.56 | 7.77 | 8.00 | 1.72 |
| 2B | 76 | 11.53 | 8.98 | 7.77 | 8.02 | 11.12 |

Watershed 2: Proposed 100-yr Storm Event Runoff



| Sub-shed | CN | Tc (mins) | Area (Ac.) | Total Rainfall (in.) | Time to Peak (hrs.) | Peak Runoff (cfs) |
|----------|----|-----------|------------|----------------------|------------------------|----------------------|
| 2A | 92 | 6.00 | 1.56 | 7.77 | 10.33 | 0.75 |
| 2B | 77 | 9.64 | 8.98 | 7.77 | 8.02 | 11.47 |

Watershed 2A: Proposed 100-yr Storm Event Storage



| ^{roject} Anthem Winery | | | Ву | DJS | Date ç | 9/28/2016 |
|--|----------------------|------------------------------------|------------|-----------|--|----------------------------|
| Location Watershee | ocation Watershed 3A | | | PSW | Date g | 9/28/2016 |
| Subshed Existing Conditions | | | Check one: | ✓ Present | De ^v | veloped |
| RUNOFF CURVE NU | MBER | | | | | |
| Soil name and hydrologic group (SCS book) | | er description reatment and hyd | drogic | CN (1) | Area acres mi2 | Product of CN x Area |
| | | percent impervio | | 1 | <u></u> % | |
| 133-C | Impervious Area (Ro | oads, roofs, etc | .) | 98 | 0.24 | 23.52 |
| 133-C | Pasture Land (fair) | | | 79 | 1.38 | 109.02 |
| | | | | | and the second s | |
| | | | | | | |
| | | | | | | |
| | | | | | | |
| | | | | | | |
| | | | | | Alaman and a second a second and a second an | |
| | | | | | | |
| | | | | | | |
| | | | | | | |
| | | | | | | |
| | | | | | · · · · · · · · · · · · · · · · · · · | |
| | | | | | | |
| (1) Use only one CN source | e per line | | | TOTAL: | 1.62 | 132.54 |
| CN (weighted) = | total product = | 132.54 1.62 | 81.81 | ; USE CN | 82 |] |
| Name of the Control o | | 1.02 | | | | |

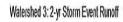
Worksheet: Time of Concentration (Tc) or travel time (Tt)

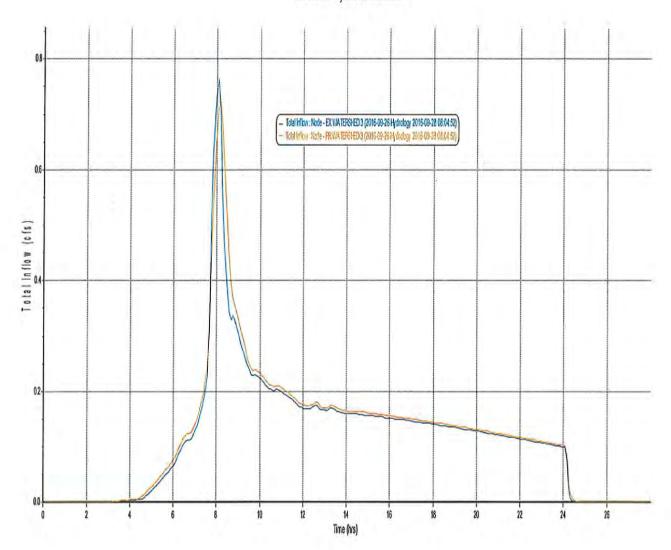
| Project Anthem Winery | By | | Date 0/20/2016 |
|---|-------------|---|--------------------|
| THILLETT WHELY | | DJS | 9/20/2010 |
| Location Project Site | Checked | PSW | Date 9/28/2016 |
| Subshed Watershed 3A | Check one | e: 🗸 Prese | nt Developed |
| Note: Space for as many as two segments per flow type ca be used for each worksheet. Include a map, schema or description of flow segments. | II hack one | e: 🗸 Tc | Tt through subarea |
| SHEET FLOW (applicable to Tc only) | | | |
| Segment ID | 1 | | |
| 1. Surface description (table 3-1) | Range | | |
| 2. Manning's roughness coefficient, n (table3-1) | 0.13 | | |
| 3. Flow length, L (total L, 300 ft) | 277 | | |
| 4. Two-year 24-hour rainfall, P2 in | 3.4 | | |
| 5. Land slope, s ft/ft | 0.13 | | |
| 6. $T_t = \frac{0.007 \text{ (nL)}^{0.8}}{P_2^{0.5} \text{ s}^{0.4}}$ Compute T_t . hr | 0.1510 | - | = 0.1510 |
| SHALLOW CONCENTRATED FLOW | | | |
| Command ID | 2 | 3 | |
| Segment ID | UNPAVED | PAVED | |
| 7. Surface description (paved or unpaved) 8. Flow length, L | 108 | 10 | |
| 9. Watercourse slope, s ft/ft | 0.13 | 0.13 | |
| 10. Average velocity, V (figure 3-1) ft/sec | 5.8174 | 7.3294 | · |
| 11. $T_t = \frac{L}{3600 \text{ V}}$ Compute T_t . hr | 0.0052 + | 0.0004 | = 0.0055 |
| CHANNEL FLOW | | | |
| | | | |
| Segment ID | | | |
| 12. Cross sectional flow area, a ft ² | | | |
| 13. Wetted perimeter, pw ft | | | |
| 14. Hydraulic radius, $r = \frac{a}{p_w}$ Compute r ft | | • | |
| 15. Channel slope, s ft/ft | | | |
| 16. Manning's roughness coefficient, n | | | |
| 17. $V = \frac{1.49 r^{2/3} s^{1/2}}{n}$ Compute V . ft/sec | | | |
| 18. Flow length, L | | | |
| 19. $T_t = \frac{L}{3600 \text{ V}}$ Compute T_t . hr | + | - | |
| 20. Watershed or subarea T_c or T_t (add T_t in steps 6, 11 and | | | . hr 0.1565 |

| ^{Project} Anthem Winery | | | Ву | DJS | Date 9 | 9/28/2016 |
|-----------------------------------|---------------------|-----------------------------------|------------|-------------|----------------------|----------------------------|
| ^{Location} Watershed | cation Watershed 3B | | | PSW | Date 9 | 9/28/2016 |
| Subshed Existing Co | onditions | | Check one: | ✓ Present | ☐ Dev | veloped |
| RUNOFF CURVE NUN | MBER | | | Erick C | | |
| Soil name and hydrologic group | | er description reatment and hy | odrogic | CN (1) | Area ✓ acres ☐ mi2 | Product of CN x Area |
| (SCS book) | | percent impervi | | (Table 2-2) | <u> </u> % | |
| 133-C | Impervious Area (R | oads, roofs, etc | 5.) | 98 | 0.10 | 9.80 |
| 133-C | Pasture Land (fair) | | | 79 | 0.12 | 9.48 |
| | | | | | | |
| | | | | | | |
| · | | | | | | |
| | | | | | | |
| | | | | | | |
| | | | | | | |
| | | | | | | |
| | | | | | | |
| | | | | | | |
| | | | | | | |
| | | | | | | |
| | | | | | · | |
| | | | | | | |
| | | | | | | |
| | | | | | | |
| | | | | | | |
| | | | | | | |
| (1) Use only one CN source | per line | • | | TOTAL: | 0.22 | 19.28 |
| CN (weighted) =- | total product = _ | 19.28 | 87.64 | ; USE CN | 88 | |
| CIA (Meistillen) | total area | 0,22 | | - | | |

| Project Anthem W | Ву | DJS | Date 9 | 9/26/2016 | |
|-----------------------------------|---|-----------|-------------|--------------------|----------------------------|
| Location Watershed 3A | | Checked | PSW | Date 9 | 9/26/2016 |
| Subshed Proposed C | Proposed Conditions (C | | | ✓ Dev | /eloped |
| RUNOFF CURVE NUM | ABER | | | | |
| Soil name and hydrologic group | Cover description | | CN (1) | Area ✓ acres mi2 | Product of CN x Area |
| (SCS book) | (cover type, treatment and hy condition; percent impervi | | (Table 2-2) | | |
| 133-C | Impervious Area (Roads, roofs, et | c.) | 98 | 0.38 | 37.24 |
| 133-C | Pasture Land (fair) | | 79 | 1.24 | 97.96 |
| | | | | | |
| | | | | | |
| | | | • | | |
| × | | | | | |
| | | | | | |
| | | | | | |
| | | | | | |
| | | | | | |
| | | | | | |
| | | | | | |
| | | | | | |
| | i i | | | | |
| | | | | | |
| | | | | | |
| | | | | | |
| (1) Use only one CN source | | | | · | |
| (1) Use only one Civ source | per mie | | TOTAL: | 1.62 | 135.20 |
| CN (weighted) =- | total product = 135.20 | _ = 83.46 | ; USE CN | 83 |] |
| | total area 1.62 | | | | |

| ^{Project} Anthem Winery | | | Зу | DJS | Date 9 | /28/2016 |
|---|--|------------|---------|-----------------------|---------------------------|----------------------------|
| Location Watershed 3B | | | Checked | PSW | Date 9 | /28/2016 |
| Subshed Proposed C | C | Check one: | Present | ✓ Dev | eloped | |
| RUNOFF CURVE NUN | ABER | | | | | |
| Soil name and hydrologic group (SCS book) | Cover description (cover type, treatment and hydrogic | | | CN (1) (Table 2-2) | Area ✓ acres ☐ mi2 ☐ % | Product of CN x Area |
| 133-C | condition; percent Impervious Area (Roads, ro | | 3) | 98 | 0.12 | 11.76 |
| 133-C | Pasture Land (fair) | | | 79 | 0.10 | 7.90 |
| | | | | | | |
| | | | | | | |
| | | | | | | |
| | | | | | | |
| | | | | | | |
| | | | | | | |
| | | | | | | |
| | | | | | | |
| <u></u> | | | | | | |
| | | | | | - | |
| | | | | | | |
| | | | | | · | |
| | | | | | | |
| | | | | | | |
| | | | | | | |
| (1) Use only one CN source | per line | | | TOTAL: | 0.22 | 19.66 |
| | total product 19. | .66 | 89.36 | ; USE CN | 89 | |
| CN (weighted) = | $\frac{\text{total product}}{\text{total area}} = \frac{19.}{0.2}$ | | | | | |

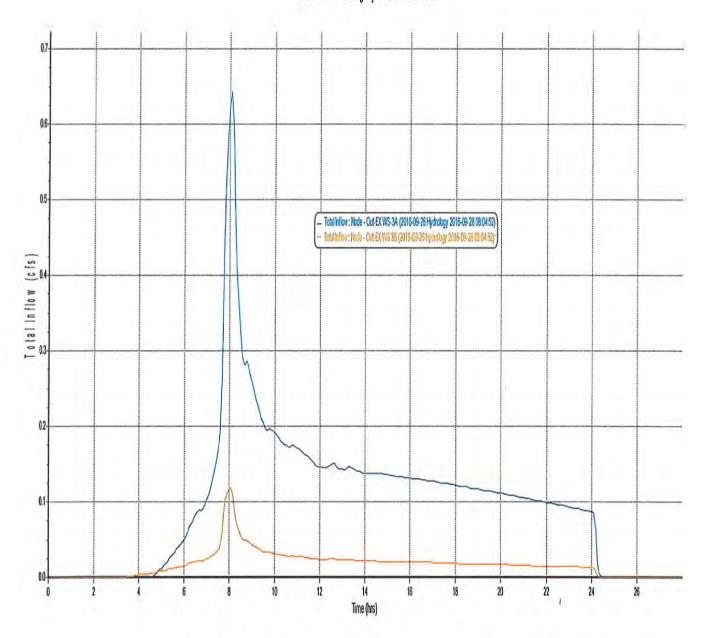




Existing Sub-shed Runoff Results

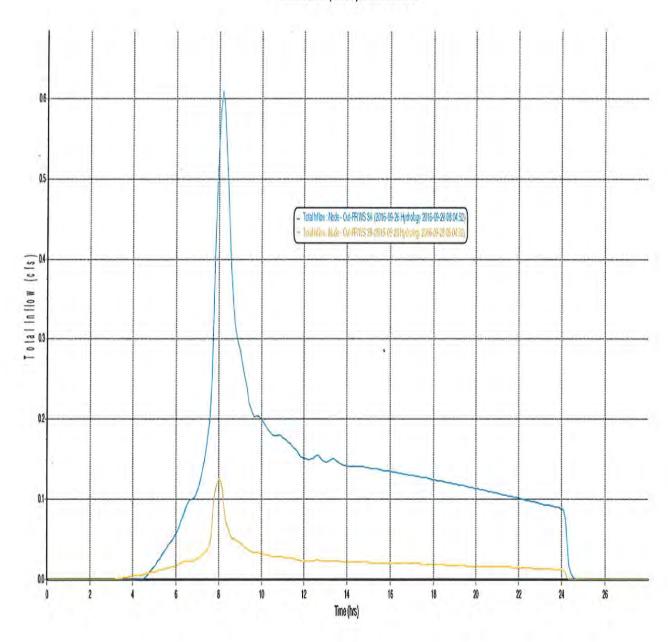
| Total Rainfall (in) | Total Rainfall (in)3.40 |
|-----------------------|-------------------------|
| Peak Runoff (cfs)0.76 | Peak Runoff (cfs)0.72 |
| Time to Peak (hrs) | Time to Peak (hrs) |

Watershed 3: Existing 2-yr Storm Event Runoff



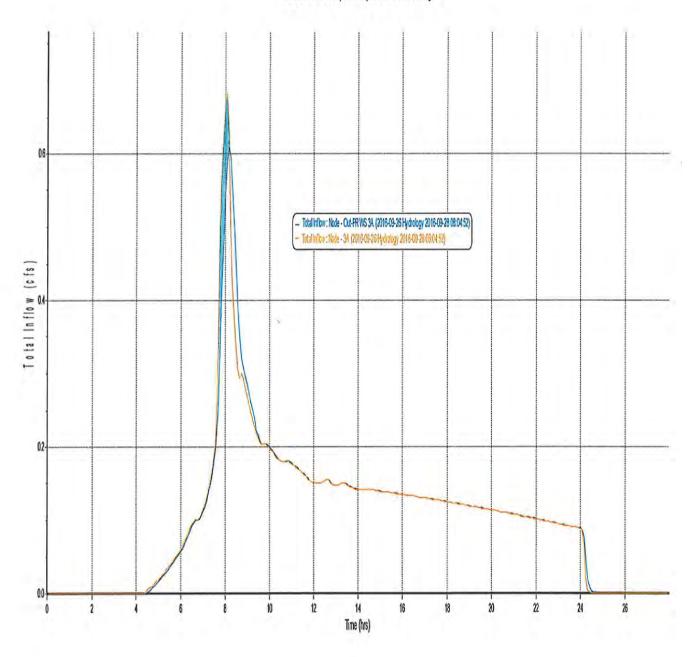
| Sub-shed | CN | Tc (mins) | Area (Ac.) | Total Rainfall (in.) | Time to Peak (hrs.) | Peak Runoff (cfs) |
|----------|----|-----------|------------|----------------------|------------------------|----------------------|
| 3A | 82 | 9.39 | 1.62 | 3.40 | 8.08 | 0.64 |
| 3B | 88 | 6.00 | 0.22 | 3.40 | 8.07 | 0.12 |

Watershed 3: Proposed 2-yr Storm Event Runoff



| Sub-shed | CN | Tc (mins) | Area (Ac.) | Total Rainfall (in.) | Time to Peak (hrs.) | Peak Runoff (cfs) |
|----------|----|-----------|------------|----------------------|------------------------|-------------------|
| 3A | 83 | 6.00 | 1.62 | 6.99 | 8.17 | 0.61 |
| 3B | 89 | 6.00 | 0.22 | 6.99 | 8.00 | 0.13 |

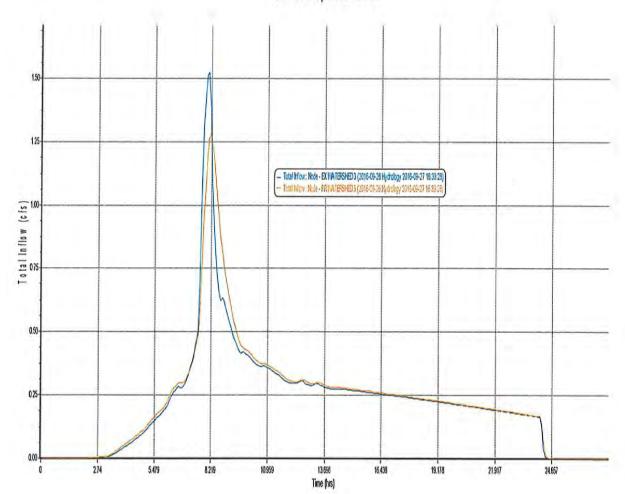
Watershed 3A: Proposed 2-yr Storm Event Storage



1

Storage......246 cu. ft.

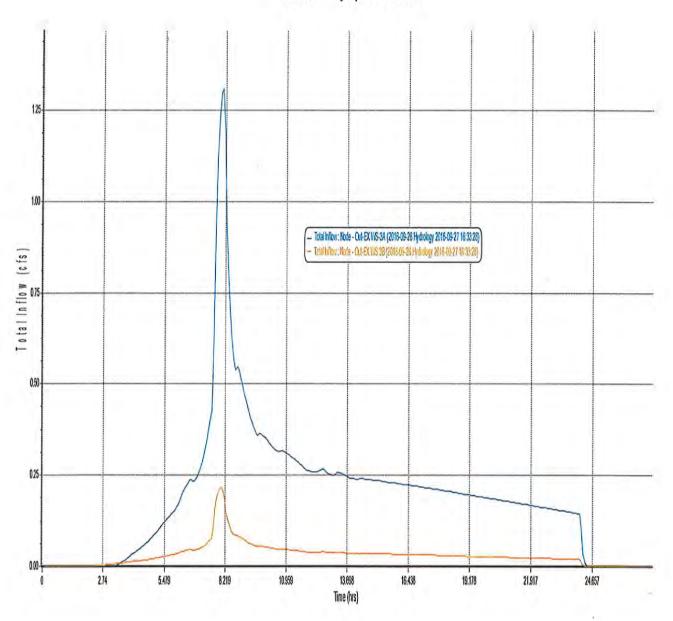




Existing Sub-shed Runoff Results

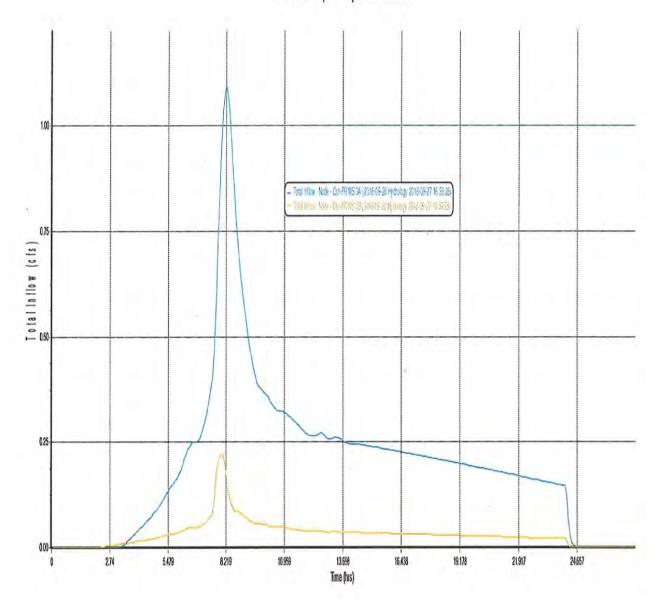
| Total Rainfall (in) | Total Rainfall (in)5.17 |
|-------------------------|-------------------------|
| Peak Runoff (cfs)1.52 | Peak Runoff (cfs)1.28 |
| Time to Peak (hrs) 8.08 | Time to Peak /hrs) 8 17 |

Watershed 3: Existing 10-yr Storm Event Runoff



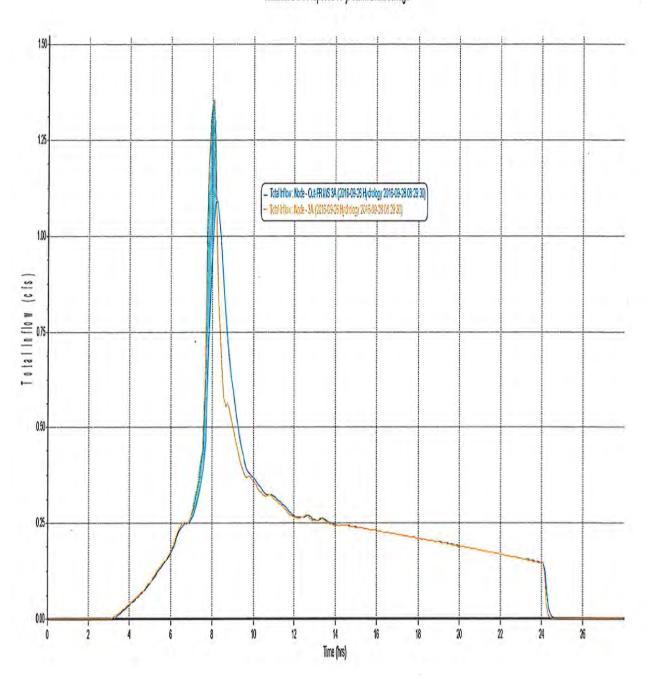
| Sub-shed | CN | Tc (mins) | Area (Ac.) | Total Rainfall (in.) | Time to Peak (hrs.) | Peak Runoff (cfs) |
|----------|----|-----------|------------|----------------------|------------------------|----------------------|
| 3A | 82 | 9.39 | 1.62 | 5.17 | 8.08 | 1.31 |
| 3B | 88 | 6.00 | 0.22 | 5.17 | 8.00 | 0.22 |

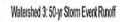
Watershed 3: Proposed 10-yr Storm Event Runoff

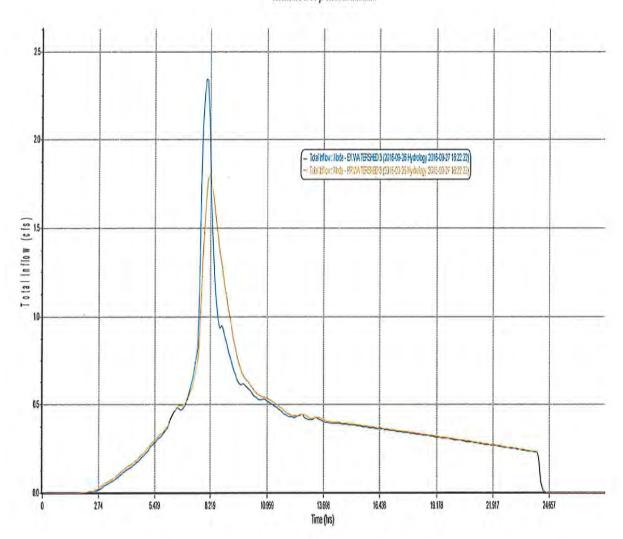


| Sub-shed | CN | Tc (mins) | Area (Ac.) | Total Rainfall (in.) | Time to Peak (hrs.) | Peak Runoff (cfs) |
|----------|----|-----------|------------|----------------------|------------------------|-------------------|
| 3A | 83 | 6.00 | 1.62 | 6.99 | 8.17 | 1.09 |
| 3B | 89 | 6.00 | 0.22 | 6.99 | 8.00 | 0.22 |

Watershed 3A: Proposed 10-yr Storm Event Storage



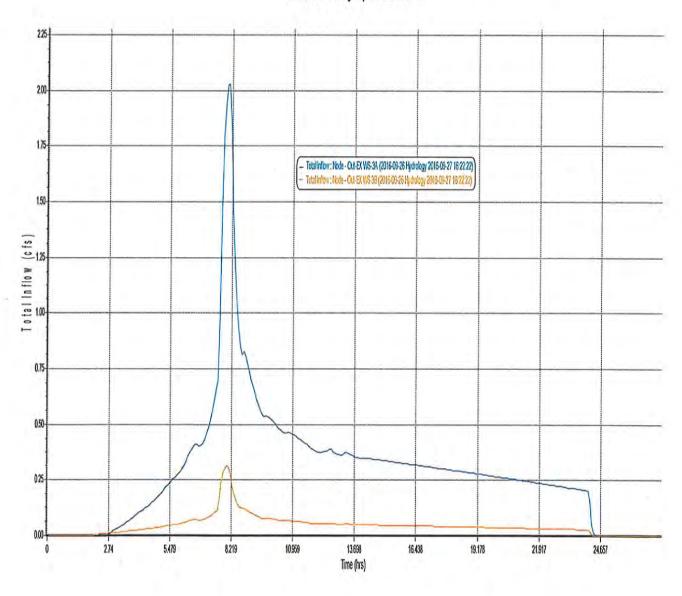




Existing Sub-shed Runoff Results

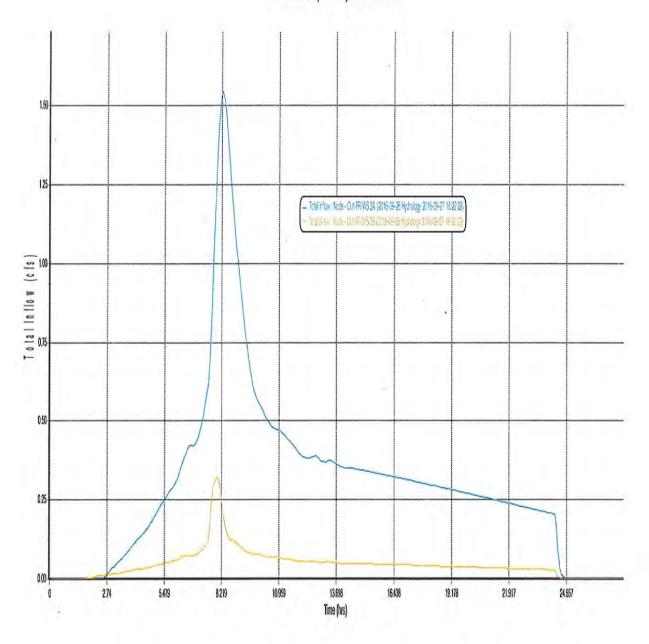
| Total Rainfall (in) | Total Rainfall (In) |
|-----------------------|-------------------------|
| Peak Runoff (cfs)2.34 | Peak Runoff (cfs)1.81 |
| Time to Peak (hrs) | Time to Peak (hrs) 8.17 |

Watershed 3: Existing 50-yr Storm Event Runoff



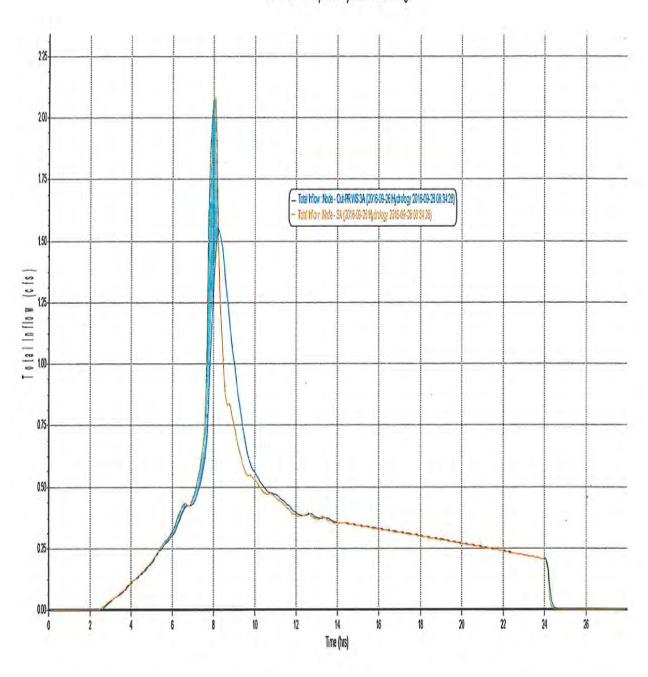
| Sub-shed CN 3A 82 | | Tc (mins) | Area (Ac.) | Total Rainfall (in.) | Time to Peak (hrs.) | Peak Runoff (cfs) |
|----------------------|----|-----------|------------|----------------------|------------------------|----------------------|
| | | 9.39 | 1.62 | 6.99 | 8.08 | 2.03 |
| 3B | 88 | 6.00 | 0.22 | 6.99 | 8.00 | 0.32 |

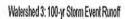
Watershed 3: Proposed 50-yr Storm Event Runoff

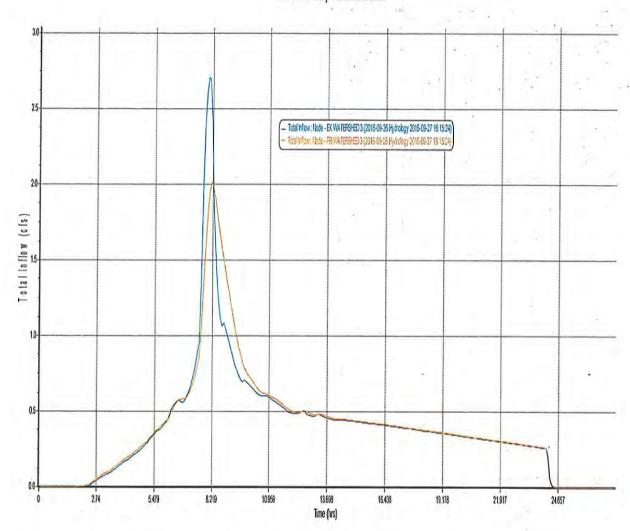


| Sub-shed | CN | Tc (mins) | Area (Ac.) | Total Rainfall (in.) | Time to Peak (hrs.) | Peak Runoff (cfs) |
|----------|----|-----------|------------|----------------------|------------------------|----------------------|
| 3A 83 | | 6.00 | 1.62 | 6.99 | 8.17 | 1.54 |
| 3B | 89 | 6.00 | 0.22 | 6.99 | 8.00 | 0.32 |

Watershed 3A: Proposed 50-yr Storm Event Storage



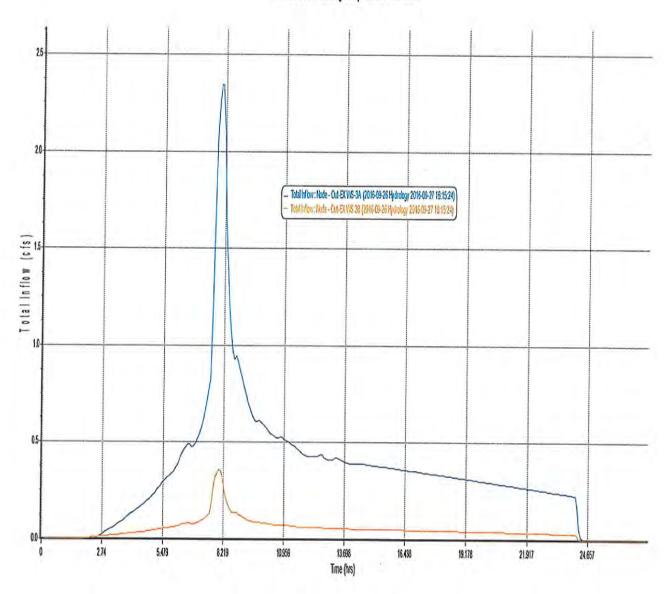




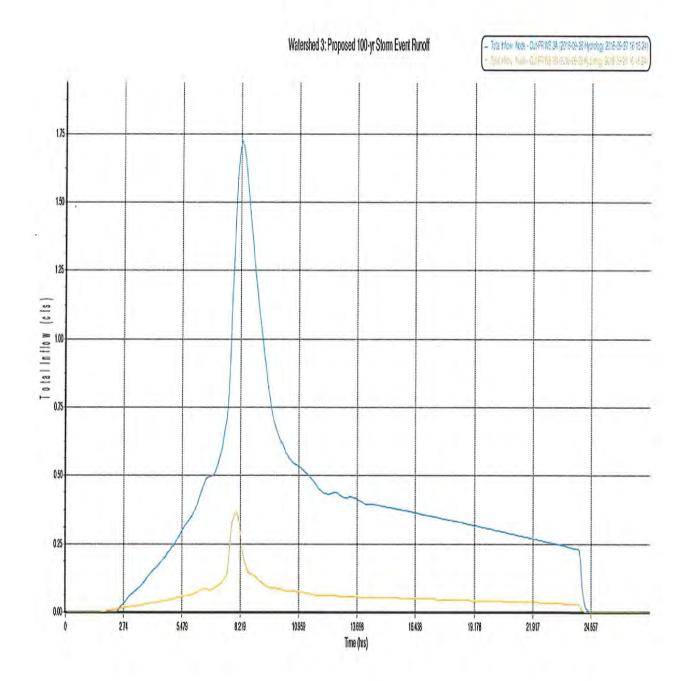
Existing Sub-shed Runoff Results

| Total Rainfall (In) | Total Rainfall (in)7.77 |
|------------------------|-------------------------|
| Peak Runoff (cfs)2.71 | Peak Runoff (cfs)2.02 |
| Time to Peak (hrs)8.00 | Time to Peak (hrs) |

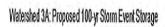
Watershed 3: Existing 100-yr Storm Event Runoff

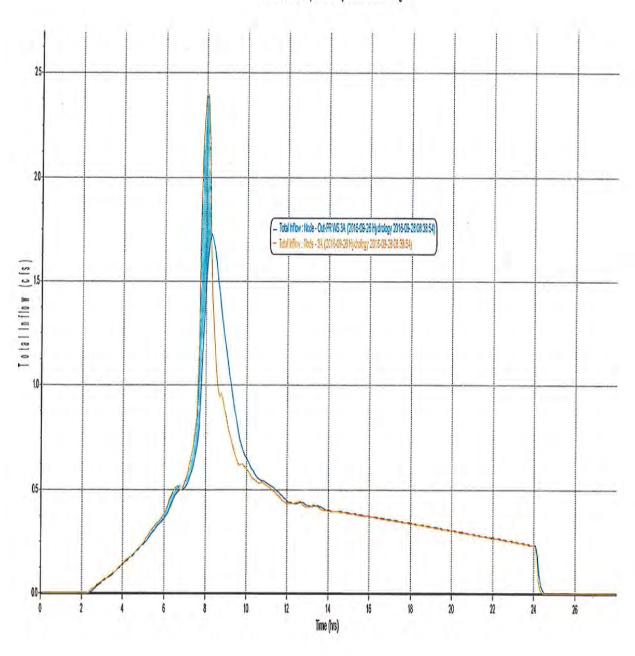


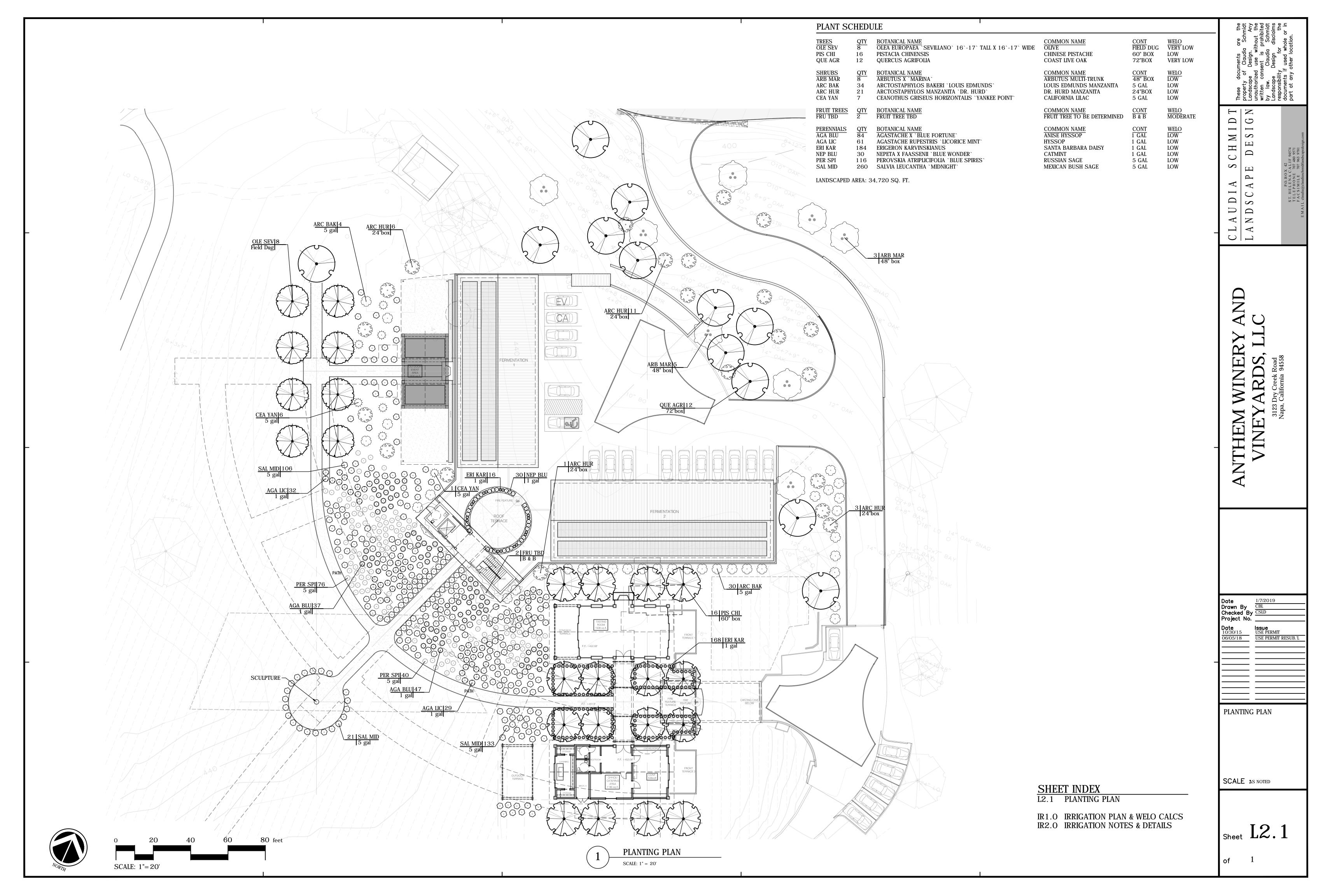
| Sub-shed | CN | Tc (mins) | Area (Ac.) | Total Rainfall (in.) | Time to Peak (hrs.) | Peak Runoff (cfs) |
|----------|----|-----------|------------|----------------------|------------------------|----------------------|
| 3A | 82 | 9.39 | 1.62 | 7.77 | 8.00 | 2.35 |
| 3B | 88 | 6.00 | 0.22 | 7.77 | 8.08 | 0.36 |

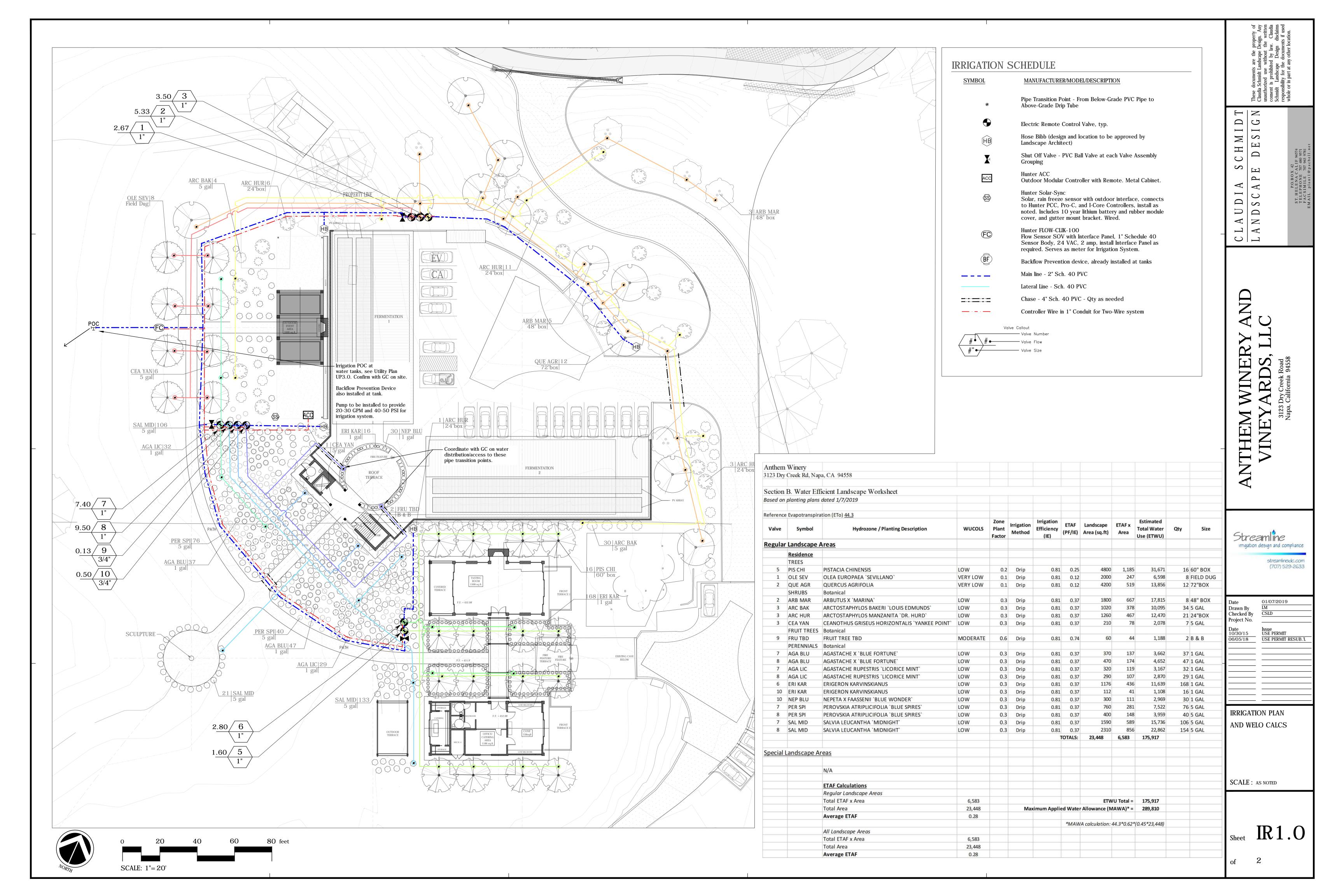


| Sub-shed | CN | Tc (mins) | Area (Ac.) | Total Rainfall (in.) | Time to Peak (hrs.) | Peak Runoff (cfs) |
|----------|----|-----------|------------|----------------------|------------------------|----------------------|
| 3A | 83 | 6.00 | 1.62 | 7.77 | 8.17 | 1.72 |
| 3B | 89 | 6.00 | 0.22 | 7.77 | 8.00 | 0.36 |







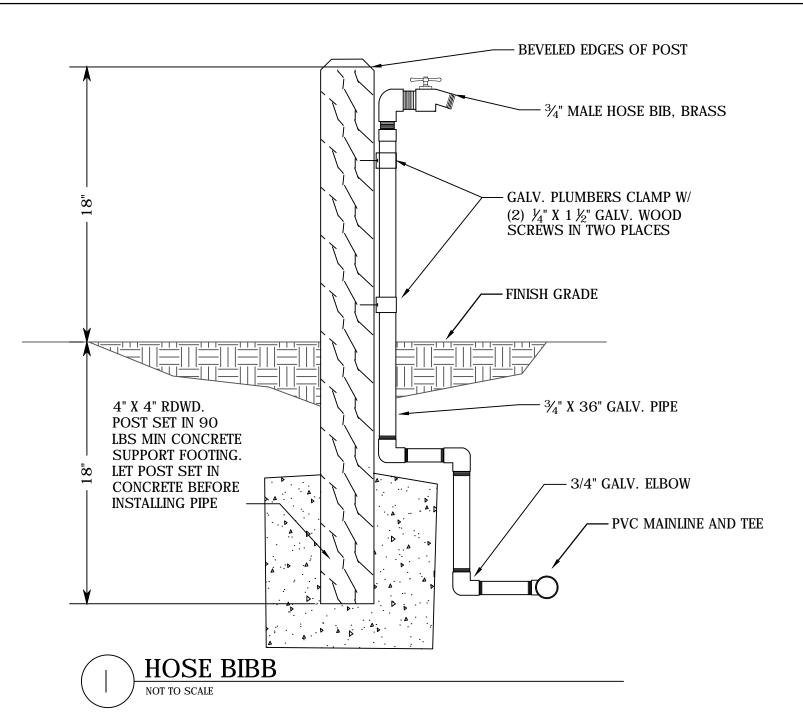


Irrigation Notes

- 1. These irrigation drawings are diagrammatic and indicative of the work to be installed. All piping, valves and other irrigation components may be shown within paved areas for graphic clarity only and are to be installed within planting areas. Due to the scale of the drawings, it is not possible to indicate all offsets, fittings, sleeves, conduit and other items which may be required. Notify any coordinate irrigation contract work with applicable contractors for the location and installation of pipe, conduit or sleeves.
- 2. The intent of this irrigation system is to conserve resources while providing a minimum amount of water required to sustain good plant health.
- 3. It is the responsibility of the maintenance contractor and/or owner to program the irrigation controller to provide the minimum amount of water needed to sustain good plant health. This includes making adjustments to the irrigation program for seasonal weather changes, plant material, water requirements, mounds, slopes, sun, shade, wind exposure and growth over time.
- 4. It is the responsibility of a licensed electrical contractor to provide 120 volt A.C. (2.5 amp demand per controller) electrical service to the controller location. It is the responsibility of the irrigation contractor to coordinate the electrical service stub-out to the controller. Provide proper grounding per controller manufacturer's instructions and in accordance with local codes.
- 5. Provide the irrigation controller with its own independent low voltage common ground
- 6. Unless otherwise indicated, well contractor to provide a dedicated 2" service line with a minimum of 25 GPM and 40 PSI with a separate meter or sub-meter.
- 7. Irrigation controller shall be weather- or moisture-based controller that automatically adjusts irrigation in response to changes in plants' needs as weather conditions change. Weather-based controllers without integral rain sensors or communication systems that account for local rainfall shall have a separate rain sensor which communicates with the controller. Soil moisture-based controllers are not required to have rain sensor input.
- 8. Install new batteries in the irrigation controller to retain program information during power failures. Owner/maintenance contractor to check twice annually and replace when necessary, no less frequent than two years.
- 9. Schedule a meeting which includes the maintenance contractor, the owner (or owner's representative) and the irrigation contractor at the site for instruction on the proper programming and operation of the irrigation controller. Irrigation contractor to provide owner's manual and as-built plans.
- 10. Splicing of low voltage wires is permitted in valve boxes only. Leave a 24" long, 1" diameter coil of excess wire at each splice. Tape wires together every ten feet. Do not tape wires together where contained within sleeving or conduit.
- 11. Install black plastic valve boxes with non-hinged cover marked "irrigation"
- 12. Hose bib and irrigation valve locations are diagrammatic. Install remote control valve boxes 12" from walk, curb, lawn header board, building or landscape feature. At multiple valve box groups, install each box 12" apart. Hose bib and valve box locations to be approved by landscape architect.
- 13. A ball/gate valve shall be installed to isolate each irrigation valve or group of valves located together. Gate valve size shall be the same as the main line.
- 14. Flush and adjust irrigation outlets and nozzles for optimum performance and to not allow overspray onto walks, roadways and/or buildings. Select the best degree of the arc and radius to fit the existing site conditions and throttle the flow control at each valve to obtain the optimum operating pressure for each control zone.
- 15. Set sprinkler heads perpendicular to finish grade.
- 16. Locate emitter outlets and bubblers on uphill side of plant or tree.
- 17. At locations where low sprinkler head drainage will cause erosion and/or excess water, install a pop-up body with integral check valve. Install a spring loaded check valve on bubbler and emitter risers where required.
- 18. Where it is necessary to excavate adjacent to existing trees, use caution to avoid injury to trees and tree roots. Excavate by hand in areas where 2 inch diameter and larger roots occur. Backfill trenches adjacent to tree within 24 hours. Where this is not possible, shade the side of the trench adjacent to the tree with wet burlap or
- 19. The sprinkler system design is based on the minimum operating pressure shown on the irrigation drawings. Verify water pressure prior to construction. Report any difference between the water pressure indicated on the drawings and the actual pressure reading at the irrigation point of connection to the owner's authorized representative.
- 20. Pipe sizing shown on the drawings is typical. As changes in layout occur during staking and construction the size may need to be adjusted accordingly.
- 21. The irrigation contractor shall be responsible for minor changes in the irrigation layout due to obstructions not shown on the irrigation drawings such as lights, fire hydrants, signs, electrical enclosures, unforeseen underground utilities or boulders, etc.

"I have complied with the criteria of the Model Water Efficient Landscape Ordinance and applied them accordingly for the efficient use of water in the irrigation design plan."

-Lindsay Merget, Streamline Irrigation Design and Compliance

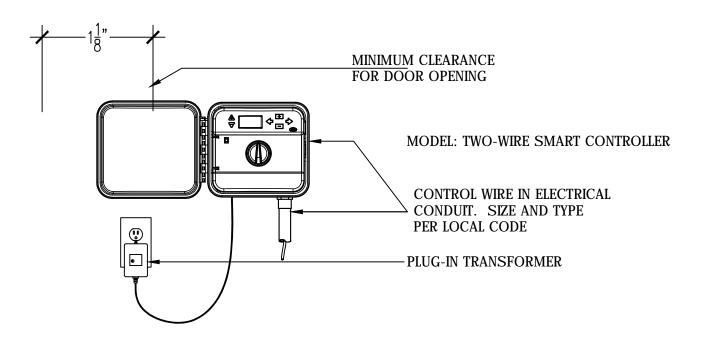


| | | EMITTER | CHART - 1 | NETAFIM I | PC 1 GPH | EMITTER | | |
|-------------------|-------|---------|-------------------|-----------|----------|---------|---------|-----------|
| Container Size | 1 gal | 5 gal | 15 gal/24" box | 36" box | 48" box | 60" box | 72" box | Field Dug |
| Qty Emitters | 1 | 2 | 4 | 6 | 10 | 14* | 20* | 20* |

*Distribute emitters on 2 drip rings

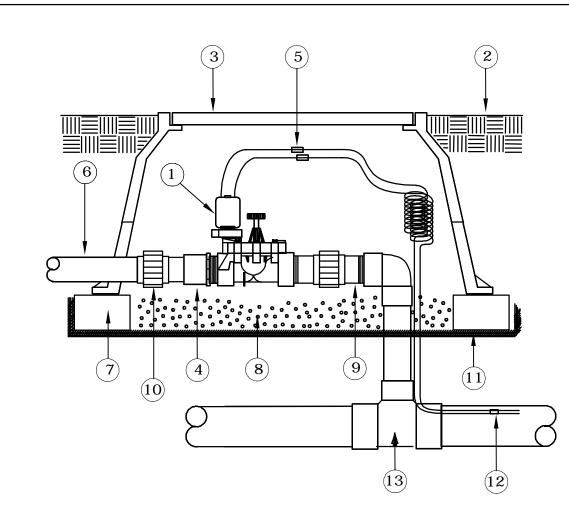






MOUNT CONTROLLER WITH LCD SCREEN AT EYE LEVEL. PLUG-IN TRANSFORMER SHALL BE CONNECTED TO GROUNDED 110 VAC OUTLET.





(1) Irritrol 700-1/-LS, 700-1.5/-LS, 700-2/-LS electric control valve or equal

(2) Finish Grade

(3) Valve Box, black

4 PVC Male Adapter

(5) Waterproof wire connectors

6 PVC lateral line-angle to proper depth with 450 ells

(7) Common red brick (optional, 4 required)

(8) 3/8" Gravel, 1-1/2" deep

(9) PVC Schedule 80 nipple (6" long)

(10) PVC Schedule 80 Union (11) Hardware Cloth for gopher protection, typ.

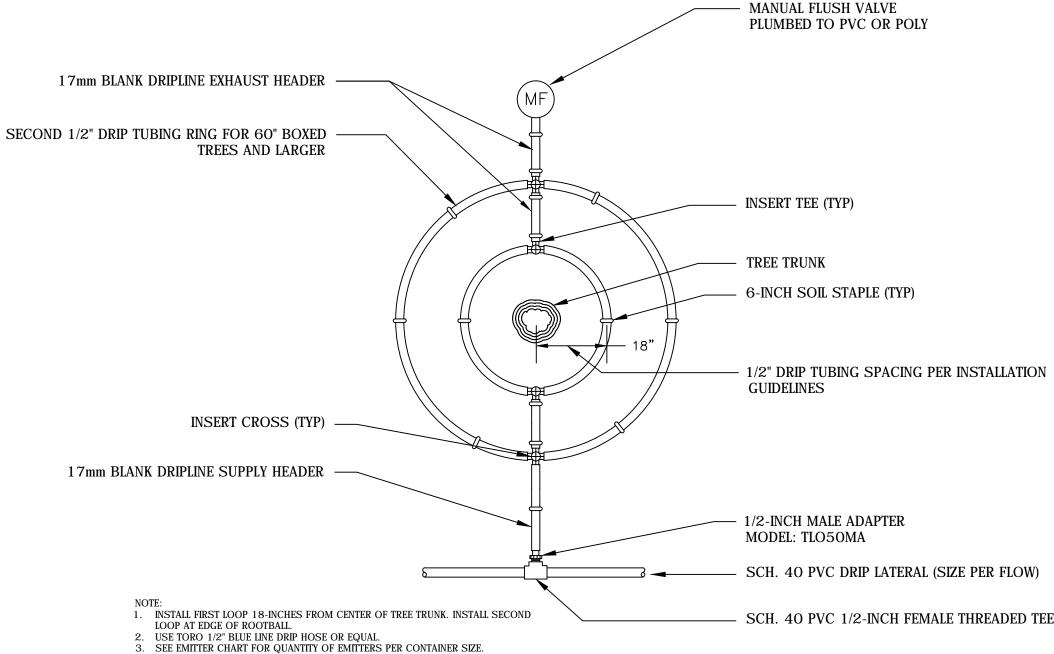
(12) Common and control wires to controller

(13) Pressure main line piping with main line litting (depth as required)

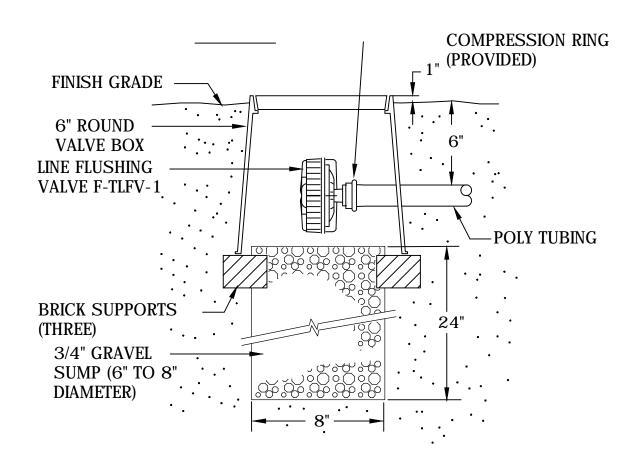
1. This drawing is provided for reference only. Individual project requirements and local codes may dictate differences in installation procedure that are not identified here. 2. Refer to product literature for additional installation and adjustment information.

VALVE ASSEMBLY

NOT TO SCALE



TREE RING DETAIL FOR 24" BOXED TREES AND LARGER





0 U \mathcal{C} \sim \forall Π

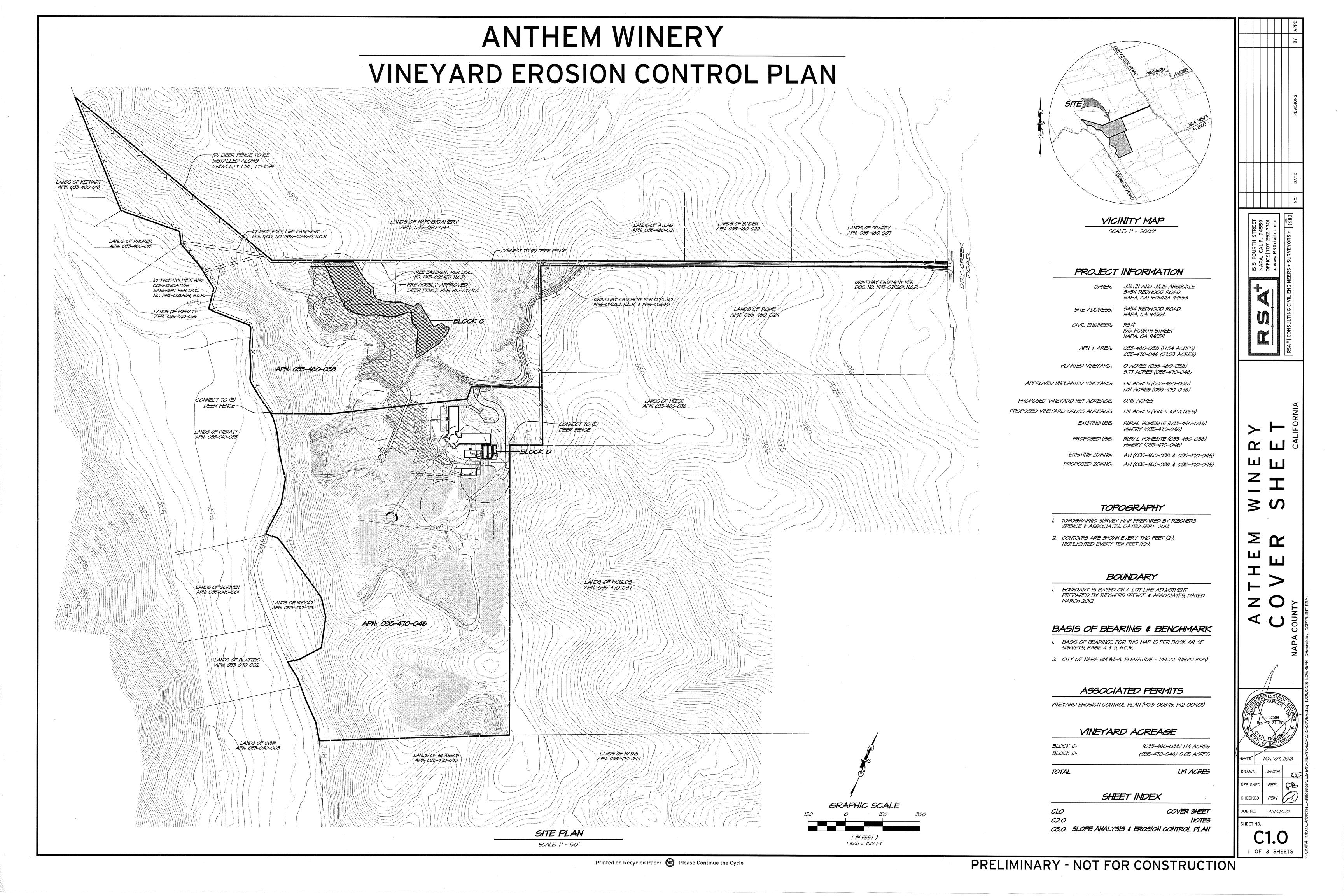
S $A \mid X$

Streamline irrigation design and compliance streamlineidc.com (707) 529-2633

| Date Drawn By Checked By Project No. | 01/07/2019 LM CSLD |
|---|--|
| Date 10/30/15 06/05/18 | Issue USE PERMIT USE PERMIT RESUB.'L |
| | |

IRRIGATION NOTES AND DETAILS

SCALE: AS NOTED



NARRATIVE SUPPLEMENT TO THE EROSION CONTROL PLAN

NATURE AND PURPOSE OF ACTIVITY

VINEYARD PLANTING AND VINEYARD AVENUES WILL BE DEVELOPED ON 1.30 ACRES LOCATED ON TWO PARCELS OF 44.63 TOTAL ACRES. NO GRADING ON SLOPES 30% OR GREATER WILL TAKE PLACE.

GENERAL SITE DESCRIPTION

THE AREAS TO BE PLANTED RANGE IN SLOPE FROM 1% TO 15% AVERAGING 14%, AND CONSIST OF THE SOILS DESCRIBED BELOW. THE PROJECT AREA CONSISTS OF RUDERAL GRASS LAND AND SPARSELY SCATTERED OAK WOODLAND. THE PROJECT LIES IN THE SALVADOR CHANNEL WATERSHED.

NATURAL FEATURES ON-SITE OR AFFECTED BY PROJECT

NO WETLANDS OR WATER BODIES, INCLUDING STREAMS OR WATERCOURSES REQUIRING SETBACKS ARE LOCATED IN THE VICINITY OF THE PROPOSED VINEYARD DEVELOPMENT.

LOCATION SOURCE OF WATER FOR IRRIGATION OR OTHER USES

SEE TIER I WAA PROVIDED WITH THE USE PERMIT SUBMITTAL.

SOIL TYPES

THE USDA-SCS NAPA COUNTY SOIL SURVEY MAPS THE PROJECT SITE AS: 133 - FAGAN CLAY LOAM, 30% TO 50% SLOPES.

CRITICAL AREAS

THERE ARE NO IDENTIFIED AREAS OF EROSION OR SLOPE STABILITY.

EROSION CALCULATIONS:

| | USLE L | AYOUT | AND PRACTI | ICE AL | TERNATIVES | A=(1 | RYKALSYCHF | <i>'</i> | - Province - | |
|---|----------------------|-------|---------------|-----------|--|---------|---|---|---|------|
| FOR: | PRE-PROJECT ANALYS | 515 | | | | | | | | |
| SOIL TYPE: | FAGAN 133 | T=3 | | | | | | | | |
| USER: | P BLAKE, CPESC | | <u> </u> | ···· | *************************************** | | | | -,-,-,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,, | |
| DATE: | 8/27/2018 | | | | Manuel Control of the | | | *************************************** | | |
| | | | TRANSECT I | | TRANSECT 2 | | TRANSECT 3 | | TRANSECT 4 | |
| # / ACRES: | | 1 | BLOCK C | 1 | BLOCK C | 1 | BLOCK C | / | BLOCK D | |
| FACTOR: | DESCRIPTION | #/ | /DESCRIBE | #2 | /DESCRIBE | #3 | /DESCRIBE | #4 | DESCRIBE | #5 |
| R** | RAINFALL | 54 | | 54 | *************************************** | 54 | | 54 | | |
| K | SOIL EROSIVENESS | 0.28 | | 0.28 | V - V - V - V - V - V - V - V - V - V - | 0.28 | | 0.28 | | |
| *************************************** | SLOPE LENGTH (FT) | 150 | | 160 | | 220 | | 60 | | |
| 5 | GRADIENT | 15.0 | | 15.0 | | 12.0 | *************************************** | 13.0 | | |
| L5 | CALCULATED LS | 2.90 | | 2.99 | | 2.59 | VVIII 3 800 AV | 1.51 | | 0.00 |
| C | COVER | 0.042 | * | 0.042 | * | 0.042 | * | 0.042 | * | |
| P | PRACTICE | / | VERTICAL | / | VERTICAL | 1 | VERTICAL | / | VERTICAL | |
| | | | | | | | J | | | |
| A | SOIL LOSS, TONS/ACRE | 1.84 | | 1.90 | | 1.64 | | 0.96 | | 0.00 |
| | SOIL LOSS, TONS | 1.84 | | 1.90 | | 1.64 | | 0.96 | | 0.00 |
| | A=(R)(K)(LS)(C)(P) | | | | | | | | | |
| | | | - | F | ER TABLE 5 | | | | | |
| | | | *NO CANOF | Y, 60% | COVER: 100% G | RASS, 1 | DLE PASTURE | | | |
| | | | _**NOAA ATLAS | 5 14, VOL | UME 6, VERSIOI | V2 27 | R 6 HR = 1.73" | | | |
| | | | F | ?=16.55 x | · 1.73 | 37 = 54 | | | | |

| | | US | AL SUIL | JAYE. | R" SPREAD | JOHEL | -/ | | | |
|--------------------------|----------------------|-------|-----------------------|-------|-----------------|--|---------------------------------------|------|---------------------|---------|
| | USLE L | AYOUT | AND PRACTI | CE AL | TERNATIVES | A=(| RYKXLSXCXP |) | 7711110100000000000 | |
| FOR: | POST-PROJECT ANALY | 1515 | | | | | | | | |
| SOIL TYPE: | FAGAN 133 | T=3 | | | 1 1 1 1 1 1 1 1 | | · · · · · · · · · · · · · · · · · · · | | | ******* |
| USER: | P BLAKE, CPESC | | I | | | ************************************** | | | | |
| DATE: | 8/27/2018 | | | | | | | | | |
| VIP-100-010-01-1-1-1-1-1 | | I | TRANSECT I | | TRANSECT 2 | | TRANSECT 3 | | TRANSECT 4 | |
| # / ACRES: | . 1/ | / | BLOCK C | / | BLOCK C | 1 | BLOCK C | / | BLOCK D | |
| FACTOR: | DESCRIPTION | #/ | /DESCRIBE | #2 | /DESCRIBE | #3 | <i>IDESCRIBE</i> | #4 | DESCRIBE | #5 |
| R** | RAINFALL | 54 | | 54 | | 54 | | 54 | | |
| K | SOIL EROSIVENESS | 0.28 | | 0.28 | | 0.28 | | 0.28 | | |
| | SLOPE LENGTH (FT) | 150 | | 160 | | 220 | | 60 | | |
| 5 | GRADIENT | 15.0 | | 15.0 | | 12.0 | | 13.0 | | |
| L5 | CALCULATED LS | 2.90 | | 2.99 | | 2.59 | | 1.51 | | 0.00 |
| C | COVER | 0.04 | * | 0.04 | * | 0.04 | * | 0.04 | * | |
| P | PRACTICE | / | VERTICAL | / | VERTICAL | 1 | VERTICAL | / | VERTICAL | |
| A | SOIL LOSS, TONS/ACRE | 1.75 | · y walking war halfe | 1.81 | | 1.56 | | 0.91 | | 0.00 |
| | SOIL LOSS, TONS | 1.75 | | 1.81 | | 1.56 | | 0.91 | | 0.00 |
| | A=(R)(K)(LS)(C)(P) | | | | | | | | | |

RESULT: SLIGHT SOIL LOSS DECREASE, (5%, BLOCK C / 88%, BLOCK D)

ANY/ALL PROPOSED EROSION CONTROL METHODS INCLUDING, BUT NOT LIMITED TO:

A) ALL DRAINAGE SYSTEMS AND FACILITIES, WALLS, CRIBBING OR OTHER EROSION PROTECTION DEVICES TO BE CONSTRUCTED WITH, OR AS A PART OF THE PROPOSED WORK.

FIBER ROLL SEDIMENT BARRIERS ARE TO BE INSTALLED AS A TEMPORARY EROSION CONTROL MEASURE, AT THE INDICATED LOCATIONS.

B) PROPOSED VEGETATIVE EROSION CONTROL MEASURES INCLUDING MAINTENANCE OF PLANT MATERIAL...

PRIOR TO OCTOBER 15TH OF THE DEVELOPMENT YEAR, ALL DISTURBED AREAS SHALL BE SEEDED AS DESCRIBED IN THE COVER CROP PROGRAM SECTION. THE COVER CROP WILL BE MANAGED EACH YEAR SUCH THAT ANY AREAS WHICH HAVE LESS THAN TOX VEGETATION COVER WILL BE RESEEDED UNTIL ADEQUATE COVERAGE IS ACHIEVED.

AVENUES SHALL BE PLANTED, ALSO AS DESCRIBED IN THE COVER CROP PROGRAM. THE COVER CROP WILL BE MANAGED EACH YEAR SUCH THAT ANY AREAS WHICH HAVE LESS THAN 15% VEGETATIVE COVER WILL BE RESEEDED UNTIL ADEQUATE COVERAGE IS ACHIEVED. ALL AVENUES SHALL BE MOWED ONLY AND SHALL NOT BE DISCED. IN YEAR ONE OF DEVELOPMENT, ALL CLEARED LAND, INCLUDING VINEYARD AVENUES WILL BE STRAW MULCHED FOLLOWING SEEDING OF THE COVER CROP. STRAW WILL BE APPLIED AT A RATE OF 3,000 LBS. PER ACRE.

NARRATIVE SUPPLEMENT TO THE EROSION CONTROL PLAN CONTINUED

STORM WATER STABILIZATION MEASURES TO HANDLE ANY INCREASED PEAK RATES OF RUNOFF FROM THE DEVELOPMENT OF THE SITE THAT WOULD RESULT IN FLOODING OR CHANNEL DEGRADATION DOWNSTREAM.
INCLUDING CALCULATIONS OF ESTIMATED INCREASED RUNOFF AND/OR EXPLANATION OF WHY AN INCREASE IS/IS NOT EXPECTED:

TR-55 CALCULATIONS - NRCS EFM CN'S USED

| | | * | STORM DAT | TA | | |
|--------|------------|--------------|------------|------------|-------------|--------|
| R | AINFALL DI | EPTH (IN INC | HES) BY RA | AINFALL RE | TURN PERIOL | 7 |
| 2 YEAR | 5 YEAR | IO YEAR | 25 YEAR | 50 YEAR | 100 YEAR | I YEAR |
| 3.40 | 4.38 | 5./7 | 6.21 | 6.99 | 7.77 | 0.00 |

* NOAA ATLAS 14, VOLUME 6, VERSION 2

PRE-PROJECT

03/2015

| SUB-AREA LAND USE AND CURVE NUMBER DETAILS | | | | | | | |
|--|------------------------------------|--------------------------|-----------------------|-----------------|--|--|--|
| SUB-AREA IDENTIFIER | LAND USE | HYDROLOGIC SOIL GROUP | SUB-AREA AREA (AC) | CURVE NUMBER | | | |
| IA | PASTURE, GRASSLAND OR RANGE (FAIR) | C | 1.2 | 79 | | | |
| | TOTAL AREA / WEIGHTED CURVE NUMBER | | 1.2 | 79 | | | |

| SUB-AREA DATA | | | | | | | |
|---------------|------------------------------------|--------|-----------|-----|-----|--|--|
| NAME | DESCRIPTION | REACH | AREA (AC) | RCN | TC | | |
| IA | PASTURE, GRASSLAND OR RANGE (FAIR) | OUTLET | 1.2 | 79 | 0.1 | | |

POST-PROJECT

| SUB-AREA LAND USE AND CURVE NUMBER DETAILS | | | | | | | |
|--|------------------------------------|--------------------------|-----------------------|-----------------|--|--|--|
| SUB-AREA IDENTIFIER | LAND USE | HYDROLOGIC SOIL GROUP | SUB-AREA AREA (AC) | CURVE NUMBER | | | |
| IA | PASTURE, GRASSLAND OR RANGE (FAIR) | C | 1.2 | 79 | | | |
| | TOTAL AREA / WEIGHTED CURVE NUMBER | | 1.2 | 79 | | | |

| | SUB-A | REA DATA | | · | |
|------|--|----------|-----------|-----|-----|
| NAME | DESCRIPTION | REACH | AREA (AC) | RCN | TC |
| IA | 10 10 10 10 10 10 10 10 10 10 10 10 10 1 | OUTLET | 1.2 | 79 | 0.1 |

PRE-PROJECT

| PEAK FLOW (CFS) BY RAINFALL PERIOD | | | | | | |
|------------------------------------|------|------|------|------|------|------|
| | | | | | | |
| SUBAREAS IA | 0.40 | 0.66 | 0.88 | 1.18 | 1.41 | 1.64 |

POST-PROJECT

| WATERSHED PEAK TABLE | | | | | | |
|------------------------------------|--------|--------|---------|---------|---------|----------|
| PEAK FLOW (CFS) BY RAINFALL PERIOD | | | | | | |
| SUB-AREA OR REACH IDENTIFIER | 2 YEAR | 5 YEAR | IO YEAR | 25 YEAR | 50 YEAR | IOO YEAR |
| SUBAREAS IA | 0.40 | 0.66 | 0.88 | 1.18 | 1.41 | 1.64 |

PLANTING OF THE VINEYARD WITH 15% NO-TILL COVER CROPS, AS COMPARED WITH THE PRE-PROJECT CONDITION OF OPEN GRASSLAND RESULTS IN THE FOLLOWING CHANGES IN RUNOFF, BY STORM EVENT:

| 2-YR STORM | 5-YR STORM | IO-YR STORM | 25-YR STORM | 50-YR STORM | 100-YR STORM |
|------------|------------|-------------|------------------|------------------|------------------|
| O CFS | O CFS | O CFS | O CFS | O CFS | O CFS |

NET RUNOFF RESULTS:

RUNOFF DECREASES MODESTLY WITH THE VINEYARD DEVELOPMENT AND THERE ARE THUS NO NEEDS TO ATTENUATE STORM RUNOFF WITH STORAGE DEVICE LAND TREATMENTS. RUNOFF QUANTITIES APPLY TO BOTH THE REDWOOD CREEK AND SALVADOR CREEK WATERSHEDS.

IMPLEMENTATION SCHEDULE:

THE PROPOSED VEGETATION CLEARING, EARTH MOVING/GRADING, AND CONSTRUCTION/PLANTING SCHEDULE:

AFTER APRIL 15:

COMMENCE LAND CLEARING AND ESTABLISHMENT OF VINEYARD AND IRRIGATION SYSTEM.

PRIOR TO OCTOBER 15:

SEED AND MULCH THE COVER CROP AND INSTALL FIBER ROLLS.

ESTIMATED COST:

TOTAL COST FOR IMPLEMENTATION OF THE DESCRIBED EROSION CONTROL MEASURES: \$6,000.00

ROAD SERVING VINEYARDS:

NOTE - NO NEW ROADS WILL BE CONSTRUCTED TO SERVICE NEW VINEYARDS. NO CULVERTS OR CREEK / SWALE CROSSINGS ARE IN PLAY. VINEYARD AVENUES WILL NOT REQUIRE GRADING, ALLOWING FOR NATURAL SHEET FLOW TO CONTINUE, AS PER PRE-PROJECT CONDITIONS.

EROSION CONTROL PLAN NOTES

I. GRADING ON THE SITE WILL BE LIMITED TO THE EXCAVATION SHOWN ON THE PLAN.

2. FIBER ROLL SEDIMENT BARRIERS WILL BE INSTALLED PRIOR TO ANY GRADING ON THE SITE AND WILL BE OPERABLE DURING THE RAINY SEASON, OCTOBER 15 TO APRIL 15.

3. ALL MOVEMENT OF EARTH SHALL COMPLY WITH THE SPECIFICATIONS CONTAINED IN THE NAPA COUNTY GRADING ORDINANCE
AND THE EROSION CONTROL PLAN.

4. CHANGES TO THIS EROSION AND SEDIMENT CONTROL PLAN TO MEET FIELD CONDITIONS WILL BE MADE ONLY WITH THE APPROVAL OF/OR AT THE DIRECTION OF THE DIRECTOR OF PUBLIC WORKS.

5. BETWEEN OCTOBER I5 AND APRIL I, ALL PAVED AREAS WILL BE KEPT CLEAR OF EARTH MATERIAL AND DEBRIS. THE SITE WILL BE MAINTAINED SO THAT A MINIMUM OF SEDIMENT-LADEN RUNOFF LEAVES THE SITE.

6. THE CONTRACTOR WILL INFORM ALL CONSTRUCTION SITE WORKERS ABOUT THE MAJOR PROVISIONS OF THE EROSION AND SEDIMENT CONTROL PLAN AND SEEK THEIR COOPERATION IN AVOIDING THE DISTURBANCE OF THESE CONTROL MEASURES.

MAINTENANCE NOTES

I. DURING THE RAINY SEASON (OCTOBER 15 TO APRIL 15) ALL SEDIMENT BARRIERS WILL BE INSPECTED AND REPAIRED AT THE END OF EACH WORKING DAY AND, IN ADDITION, AFTER EACH STORM.

2. SEEDED AREAS WILL BE REPAIRED, RESEEDED AND MULCHED AS SOON AS POSSIBLE IF DAMAGED FOR ANY REASON.

COVER CROP PROGRAM

03/2015

<u>YEAR I</u>: CEREAL BARLEY WILL BE PLANTED AT A RATE OF 90 LBS PER ACRE. STRAW MULCH WILL BE APPLIED OVER SEED AT A RATE OF 3,000 LBS PER ACRE. THE ENTIRE DISTURBED SOIL AREA WILL BE TREATED. GRANULAR FERTILIZER WILL BE APPLIED AT A RATE THAT WILL YIELD 40 UNITS OF NITROGEN AND 60 UNITS OF PHOSPHOROUS.

YEAR 2: A NO-TILL COVER CROP WILL BE PLANTED AS FOLLOWS:

VINEYARD - 25 LBS PER ACRE PLANTING RATE:
40% BLANDO BROME, (BROMUS HORDEACEUS)
20% ZORRO ANNUAL FESCUE, (VULPIA MYUROS, VAR. HIRSUTA)
15% CALIFORNIA BROME, (BROMUS CARINATUS)
10% CRIMSON CLOVER, (TRIFOLIUM INCARNATUM)
15% ROSE CLOVER, (TRIFOLIUM HIRTUM)

AVENUES: FAWN TALL FESCUE PLANTED AT 25 LBS PER ACRE

3RD LEAF: TO ADDRESS THE LOSS OF PURPLE NEEDLEGRASS, THE COVER CROP WILL BE SUPPLEMENTED WITH THE ADDITIONAL SOWING OF PURPLE NEEDLEGRASS SEED IN THE 3RD LEAF STAGE OF VINEYARD DEVELOPMENT. SEED WILL BE DRILLED INTO THE STUBBLE OF THE NO-TILL COVER CROP THAT WAS ESTABLISHED IN YEAR 2 OF VINEYARD DEVELOPMENT. MITIGATION TIMING AT THIS STAGE OF VINEYARD GROWTH WILL MINIMIZE THE NEGATIVE IMPACTS OF SOWING A PERENNIAL GRASS COVER CROP BEFORE VINES ARE FULLY ESTABLISHED. DE-AWNED NEEDLEGRASS SEED WILL BE SEWN AT A RATE OF 35 LBS PER ACRE.

<u>VINE ROW STRIP</u>: A I FOOT WIDE MOWED OR MULCHED STRIP WILL BE MAINTAINED FOR WEED AND COVER CONTROL IN FEBRUARY OF EACH SEASON FOLLOWING PLANTING OF VINES.

GENERAL NOTES

10/2014

CONTRACTOR AGREES THAT HE SHALL ASSUME SOLE AND COMPLETE RESPONSIBILTY FOR JOB SITE CONDITIONS DURING THE COURSE OF CONSTRUCTION OF THIS PROJECT, INCLUDING SAFETY OF ALL PERSONS AND PROPERY. THIS REQUIREMENT SHALL APPLY CONTINUOUSLY AND NOT BE LIMITED TO NORMAL WORKING HOURS. THE CONTRACTOR SHALL DEFEND, INDEMNIFY AND HOLD THE OWNERS AND THE ENGINEER HARMLESS FROM ALL LIABILITY, REAL OR ALLEGED, IN CONNECTION WITH THE PERFORMANCE OF WORK ON THE PROJECT; EXCEPTING FOR LIABILITY ARISING FROM THE SOLE NEGLIENCE OF THE OWNERS OF THE ENGINEER.

2. SHOULD ANY CONTRACTOR OR SUBCONTRACTOR FIND ANY DEFICIENCIES, ERRORS, CONFLICTS OR OMISSIONS IN THESE PLANS AND SPECIFICATIONS OR SHOULD HE BE IN DOUBT AS TO THEIR MEANING OR INTENT, HE SHALL NOTIFY THE ENGINEER FOR A WRITTEN CLARIFICATION, ADDENDUM, ETC. SHOULD HE FAIL TO DO SO BEFORE SUBMITTING A PROPOSAL, HE CANNOT CLAIM ADDITIONAL COMPENSATION FOR WORK REQUIRED TO COMPLETE THE PROJECT.

3. WRITTEN DIMENSIONS ALWAYS TAKE PRECEDENCE OVER SCALED DIMENSIONS. IF THERE IS A CONFLICT, NOTIFY THE ENGINEER AND OBTAIN A CLARIFICATION. NO DEVIATIONS OR SUBSTITUTIONS SHALL BE ALLOWED WITHOUT OBTAINING WRITTEN APPROVAL FROM THE ENGINEER.

4. ALL WORKMANSHIP AND MATERIALS FOR BOTH ONSITE AND OFFSITE IMPROVEMENTS SHALL CONFORM TO THE STANDARD SPECIFICATIONS AND STANDARD PLANS. THE ONSITE IMPROVEMENTS SHALL BE INSPECTED BY THE PUBLIC WORKS INSPECTOR.

5. CONTRACTOR SHALL BE FULLY RESPONSIBLE FOR BEING FAMILIAR WITH THE PROVISIONS AND REQUIREMENTS CONTAINED IN THE NAPA COUNTY STANDARD SPECIFICATIONS. CONTRACTOR SHALL HAVE A COPY AVAILABLE AT THE JOB SITE AT ALL TIMES.

6. CONTRACTOR SHALL NOTIFY UNDERGROUND SERVICE (U.S.A.) AT I-800-642-2444 PRIOR TO START OF ANY CONSTRUCTION.

CONTRACTOR SHALL BE RESPONSIBLE FOR THE VERIFICATION OF ALL EXISTING UTILITIES IN THE FIELD. LOCATIONS OF UTILITIES AND UNDERGROUND FACILITIES SHOWN ARE APPROXIMATE AND FOR GENERAL INFORMATION ONLY.

8. ALL MATERIAL SHALL BE FURNISHED AND INSTALLED BY THE CONTRACTOR UNLESS OTHERWISE NOTED.

9. SURVEYOR SHALL LAYOUT SITE IMPROVEMENTS AS SHOWN. PRIOR TO SETTING GRADING AND LAYOUT STAKES, THE SURVEYOR SHALL CHECK THE VERIFICATION TIE AND ADVISE THE ENGINEER IF THE LAYOUT DIMENSION VARIES BY MORE THAN 0.2 FEET.

IO. ALL MOVEMENT OF EARTH WILL COMPLY WITH NAPA COUNTY GRADING ORDINANCE SPECIFICATIONS AND THIS EROSION CONTROL PLAN. 1515 FOURTH STREET

NAPA, CALIF. 94559

OFFICE|707|252.3301

+ www.RSAcivil.com +

ENGINEERS + SURVEYORS + 1980

1515 FC
NAPA,
OFFICE
+ www.

WINERY FS



DATE NOV 07, 2018
DRAWN JFWDB

DESIGNED PRB CHECKED PSW

4111010.0

SHEET NO.

JOB NO.

2 OF 3 SHEETS

