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Traffic Study



Traffic Impact Study for Flora Springs Winery



Prepared for the County of Napa

Submitted by
W-Trans

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Table of Contents

Executive Summary	1
Introduction.....	2
Transportation Setting.....	4
Capacity Analysis	5
Access and Circulation.....	14
Conclusions and Recommendations.....	15
Study Participants and References.....	16

Figure

1. Lane Configurations and Traffic Volumes	3
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Tables

1. Collision Rates at the Study Intersections.....	4
2. Two-Way Stop-Controlled Intersection Level of Service Criteria	5
3. Existing Traffic Volumes	6
4. Existing Peak Hour Intersection Levels of Service	6
5. Existing plus Approved Traffic Volumes.....	7
6. Existing plus Approved Peak Hour Intersection Levels of Service.....	7
7. Future Traffic Volumes	8
8. Future Peak Hour Intersection Levels of Service	8
9. Trip Generation Summary	9
10. Trip Distribution Assumptions.....	9
11. Existing and Existing plus Project Traffic Volumes.....	10
12. Existing and Existing plus Project Peak Hour Intersection Levels of Service	11
13. Existing plus Approved and Existing plus Approved plus Project Traffic Volumes	11
14. Existing plus Approved and Existing plus Approved plus Project Peak Hour Levels of Service.....	12
15. Future and Future plus Project Traffic Volumes	12
16. Future and Future plus Project Peak Hour Intersection Levels of Service	12

Appendices

- A. Collision Rate Calculations
- B. Intersection Level of Service Calculations
- C. Winery Trip Generation Worksheet

Executive Summary

The Flora Springs Winery project is an existing winery with a proposed modification to the Use Permit to increase daily visitation to a maximum of 100 people per day and marketing events as small as 25 people and as large as 350 people. The project's anticipated trip generation includes 42 new daily trips on average during a weekday and 19 new trips on average during a weekend day, with 11 new trips during each of the p.m. peak and weekend midday peak hours.

The study area was established by the County and includes the intersection of SR 29/Zinfandel Lane as well as SR 29 just north and south of Zinfandel Lane. The study intersection experienced a collision rate that is higher than the statewide average during the five-year period reviewed. Most of the collisions were rear-end or broadside types, likely attributable to the congested conditions that occur on SR 29.

Analysis indicates that the study intersection is operating acceptably at LOS C or better during the peak periods under Existing and Existing plus Approved Project conditions. The intersection is expected to operate unacceptably at LOS F under Future conditions and neither the County nor Caltrans have indicated plans to improve the intersection. The project volumes have a less-than-significant impact under all scenarios, including those where the study intersection is operating unacceptably without project-added volumes, as the project-added volumes represent less than one percent of projected future volumes.

Vehicles will continue to access the project via Zinfandel Lane, which terminates into the project driveway. No changes to access or circulation are proposed.

Should the County establish a traffic impact fee prior to the processing of the application to modify the Use Permit, the applicant should pay appropriate mitigation fees to contribute to future infrastructure improvements that could include signalization of SR 29/Zinfandel Lane, although the project's traffic impacts are expected to be less-than-significant.

Introduction

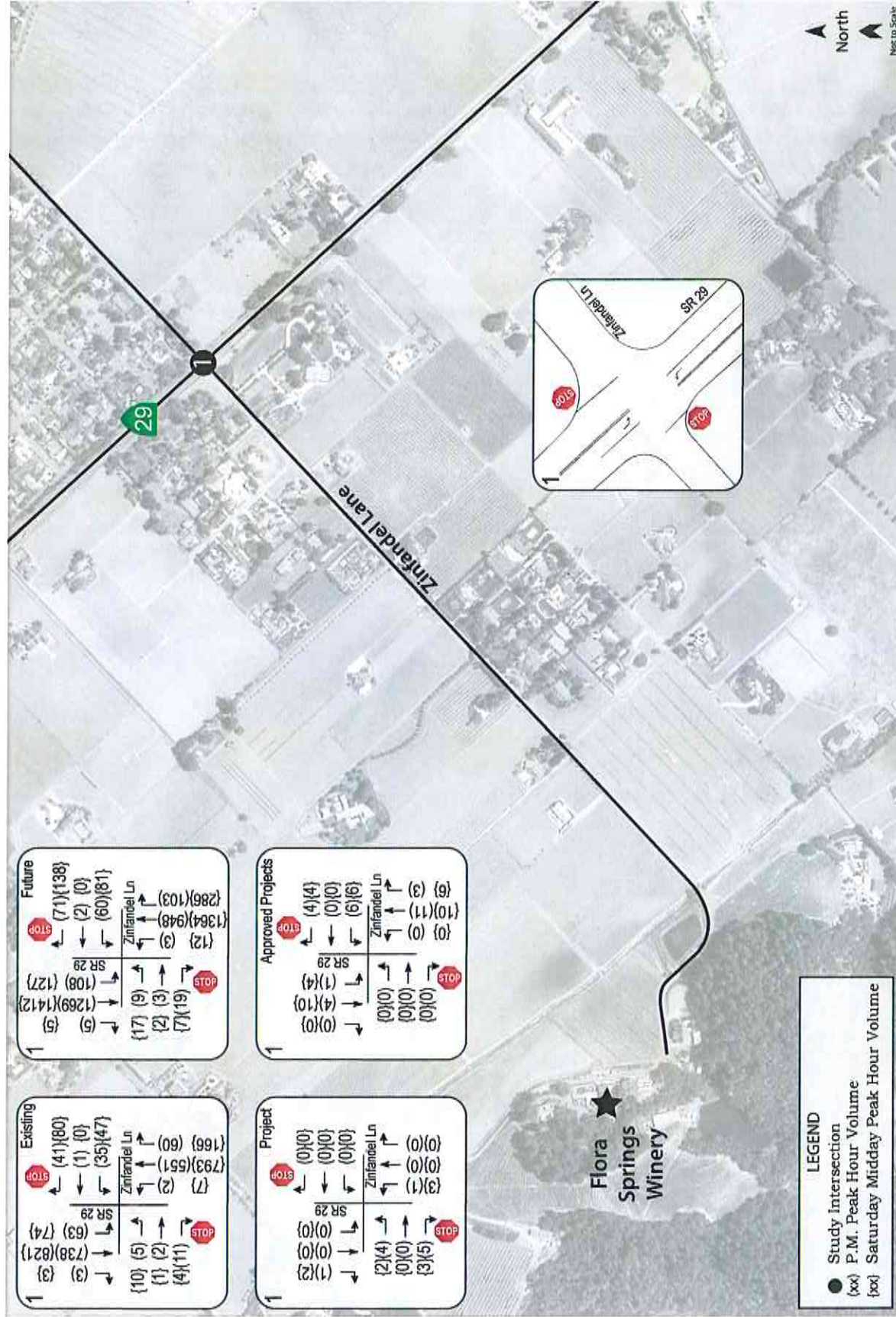
This report presents an analysis of the potential traffic impacts that would be associated with a proposed modification to the Flora Springs Winery Use Permit to increase daily visitation and marketing events. The winery is located at 1978 Zinfandel Lane in the County of Napa. The traffic study was completed in accordance with the criteria established by the County of Napa, reflects a scope of work approved by County of Napa staff, and is consistent with standard traffic engineering techniques.

Prelude

The purpose of a traffic impact study is to provide County staff and policy makers with data that they can use to make an informed decision regarding the potential traffic impacts of a proposed project, and any associated improvements that would be required in order to mitigate these impacts to a level of insignificance as defined by the County's General Plan or other policies. Vehicular traffic impacts are typically evaluated by determining the number of new trips that the proposed use would be expected to generate, distributing these trips to the surrounding street system based on existing travel patterns or anticipated travel patterns specific to the proposed project, then analyzing the impact the new traffic would be expected to have on critical intersections or roadway segments.

Project Profile

The existing Flora Springs Winery is proposing to increase their daily visitation and marketing events while keeping the same 120,000 gallons of production. The project site is located at 1978 Zinfandel Lane, as shown in Figure 1.



Traffic Impact Study for Flora Springs Winery
Figure 1 – Lane Configurations and Traffic Volumes

Transportation Setting

Operational Analysis

Study Area and Periods

The study area consists of the intersection of SR 29/Zinfandel Lane and the segment of SR 29 on either approach to Zinfandel Lane. Operating conditions during the weekday p.m. and weekend midday peak periods were evaluated as these time periods reflect the highest traffic volumes area-wide and for the proposed project. The evening peak hour occurs between 4:00 and 6:00 p.m. and typically reflects the highest level of congestion of the day during the homeward bound commute, while the weekend midday peak occurs between 2:00 and 4:00 p.m.

Study Intersection

SR 29/Zinfandel Lane is a two-way stop controlled intersection with stop signs on the eastbound and westbound Zinfandel Lane approaches. There are left-turn lanes on both approaches of SR 29. The location of the study intersection and the existing lane configuration and controls are shown in Figure 1.

Study Roadway

SR 29 runs north-south with one lane in either direction and a center two-way left-turn lane. The posted speed limit in the area is 45 miles per hour (mph).

Collision History

The collision history for the study area was reviewed to determine any trends or patterns that may indicate a safety issue. Collision rates were calculated based on records available from the California Highway Patrol as published in their Statewide Integrated Traffic Records System (SWITRS) reports. The most current five-year period available is October 1, 2008 through September 30, 2013.

As presented in Table 1, the calculated collision rate for the study intersection was compared to the average collision rate for similar facilities statewide, as indicated in *2010 Collision Data on California State Highways*, California Department of Transportation (Caltrans). The collision rate for the intersection of SR 29/ Zinfandel Lane was 0.58 collisions per million vehicles entering (c/mve) for the five-year study period, which is nearly three times the statewide collision rate for similar facilities in rural areas. Most of the collisions at the intersection were either rear-ends or broadsides, while the primary collision factor in 14 of the 17 crashes was unsafe speed or auto right of way violations. Both types of collisions that predominate at the location could be attributed to the congested conditions that occur during periods of peak flow. The collision rate calculations are provided in Appendix A.

Table 1 – Collision Rate at the Study Intersection

Study Intersection	Number of Collisions (2008-2013)	Calculated Collision Rate (c/mve)	Statewide Average Collision Rate (c/mve)
1. SR 29/Zinfandel Ln	17	0.62	0.23

Note: c/mve = collisions per million vehicles entering; **Bold** text represents a collision rate higher than the statewide average

Capacity Analysis

Intersection Level of Service Methodologies

Level of Service (LOS) is used to rank traffic operation on various types of facilities based on traffic volumes and roadway capacity using a series of letter designations ranging from A to F. Generally, Level of Service A represents free flow conditions and Level of Service F represents forced flow or breakdown conditions. A unit of measure that indicates a level of delay generally accompanies the LOS designation.

The study intersection was analyzed using methodologies published in the *Highway Capacity Manual* (HCM), Transportation Research Board, 2000. This source contains methodologies for various types of intersection control, all of which are related to a measurement of delay in average number of seconds per vehicle.

The Levels of Service for the intersection was analyzed using the "Two-Way Stop-Controlled" intersection capacity method from the HCM. This methodology determines a level of service for each minor turning movement by estimating the level of average delay in seconds per vehicle. Results are presented for individual movements together with the weighted overall average delay for the intersection.

The ranges of delay associated with the various levels of service are indicated in Table 2.

Table 2 – Two-Way Stop-Controlled Intersection Level of Service Criteria

LOS A	Delay of 0 to 10 seconds. Gaps in traffic are readily available for drivers exiting the minor street.
LOS B	Delay of 10 to 15 seconds. Gaps in traffic are somewhat less readily available than with LOS A, but no queuing occurs on the minor street.
LOS C	Delay of 15 to 25 seconds. Acceptable gaps in traffic are less frequent, and drivers may approach while another vehicle is already waiting to exit the side street.
LOS D	Delay of 25 to 35 seconds. There are fewer acceptable gaps in traffic, and drivers may enter a queue of one or two vehicles on the side street.
LOS E	Delay of 35 to 50 seconds. Few acceptable gaps in traffic are available, and longer queues may form on the side street.
LOS F	Delay of more than 50 seconds. Drivers may wait for long periods before there is an acceptable gap in traffic for exiting the side streets, creating long queues.

Reference: *Highway Capacity Manual*, Transportation Research Board, 2000

Traffic Operation Standards

Policy CIR-13 in the *Napa County General Plan* states, "The County seeks to provide a roadway system that maintains current roadway capacities in most locations and is both safe and efficient in terms of providing local access."

Policy CIR-16 of the *Napa County General Plan* provides guidance for roadways, indicating that, "The County shall seek to maintain an arterial Level of Service D or better on all county roadways, except where maintaining this desired level of service would require the installation of more travel lanes than shown on the Circulation Map. SR 29 is shown as a 2-lane Rural Throughway on the Circulation Map (Figure CIR-1). A one-percent criteria for the threshold of significance is used for this analysis because it is well within the range of daily variation in traffic as well as the range of accuracy of travel demand forecast models and therefore not likely to be noticeable to drivers.

While the traffic study relies on the County's LOS D standard, if operation falls below this threshold, an increase which is less than one-percent of cumulative volumes is considered less-than-significant.

Policy CIR-18 of the *Napa County General Plan* states, "Traffic safety and adequate local access will be priorities on roadway segments and at signalized intersections where Level of Service D or better cannot be achieved. Therefore, proposed capital improvements and development projects in these areas shall be evaluated to determine their effect on safety or local access. Projects that improve safety, improve local access, or alleviate congestion will be prioritized."

Existing Conditions

The Existing Conditions scenario provides an evaluation of current operation based on existing traffic volumes during the weekday p.m. and weekend midday peak periods. This condition does not include project-generated traffic volumes. Volume data was collected on Saturday, June 27, 2015 and Tuesday June 30, 2015. The existing traffic volumes on SR 29 are summarized in Table 3.

Table 3 – Existing Traffic Volumes								
Study Segment	Weekday		Weekday PM Peak		Saturday		Weekend MD Peak	
	SB	NB	SB	NB	SB	NB	SB	NB
SR 29 North of Zinfandel Ln	8,040	5,970	804	597	9,878	9,713	898	883
SR 29 South of Zinfandel Ln	7,840	6,130	784	613	9,592	10,626	872	966

Intersection Levels of Service

Under existing conditions, the study intersection is operating acceptably at LOS A overall during the weekday p.m. peak period and LOS C during the weekend midday peak period. The westbound approach is operating at LOS F during both peak periods, while the eastbound approach operates at LOS E during the weekend midday peak period. These results indicate excessive delay on the westbound approach, though the overall LOS C operation indicates that this is considered an acceptable condition. The existing traffic volumes are shown in Figure 1. A summary of the intersection level of service calculations is contained in Table 4, and copies of the Level of Service calculations are provided in Appendix B.

Table 4 – Existing Peak Hour Intersection Levels of Service				
Study Intersection Approach	PM Peak		Midday Peak	
	Delay	LOS	Delay	LOS
SR 29/Zinfandel Ln	3.2	A	17.0	C
Westbound Approach	48.4	E	**	F
Eastbound Approach	28.9	D	114.0	F

Notes: Delay is measured in average seconds per vehicle; LOS = Level of Service; Results for minor approaches to two-way stop-controlled intersections are indicated in *italics*;
 ** = delay greater than 120 seconds

Existing plus Approved Conditions

Existing plus Approved volumes were developed to include trips from other approved and pending projects that would add traffic to the segment of SR 29 within the vicinity of Flora Springs Winery. As directed by County staff, the following projects were included to evaluate Existing plus Approved conditions:

- **Accendo Cellars** – 50,000 gallon winery on Zinfandel Lane, east of SR 29 with up to 32 visitors per day and three employees
- **WHL Winery** – 10,000 gallon winery on Whitehall Lane, west of SR 29 with no tasting room and 10 employees
- **LMR Rutherford Estate Winery** – 100,000 gallon winery on SR 29, between Mee Lane and SR 128 (Rutherford Road) with 50 visitors and eight employees

The Existing plus Approved traffic volumes on SR 29 are summarized in Table 5.

Study Segment	Weekday		Weekday PM Peak		Saturday		Weekend MD Peak	
	SB	NB	SB	NB	SB	NB	SB	NB
SR 29 North of Zinfandel Ln	8,080	6,130	808	613	10,043	9,878	913	898
SR 29 South of Zinfandel Ln	7,920	6,280	792	628	9,779	10,813	889	983

Under Existing plus Approved conditions, the study intersection is expected to continue operating acceptably at LOS A overall during the weekday p.m. peak period and LOS C during the weekend midday peak period. The westbound approach will continue to experience high delays during both peak periods and delay is expected to increase on the eastbound approach to drop to LOS F operation during the weekend midday peak period. A summary of the intersection level of service calculations is contained in Table 6, volumes are shown in Figure 1 and copies of the Level of Service calculations are provided in Appendix B.

Study Intersection Approach	PM Peak		Midday Peak	
	Delay	LOS	Delay	LOS
SR 29/Zinfandel Ln	4.1	A	24.0	C
Westbound Approach	59.2	F	**	F
Eastbound Approach	29.8	D	**	F

Notes: Delay is measured in average seconds per vehicle; LOS = Level of Service; Results for minor approaches to two-way stop-controlled intersections are indicated in *italics*; ** = delay greater than 120 seconds

Future Conditions

Segment volumes for the horizon year of 2030 are typically obtained from the County's gravity demand model. However, at SR 29 near Zinfandel Lane, the model shows a 148 percent increase between 2010 and 2030 volumes, which does not appear reasonable or achievable. In order to arrive at more reasonable future volumes, an average was taken of the percent increase per year based on published Caltrans volumes between 1993 and 2007 and translated to a growth factor over 20 years. It is noted that volumes from 2008 on were not used as this was when volumes started substantially decreasing due to the recession. Using this method, there was an average of 2.76

percent growth in traffic volumes per year in the area of SR 29/Zinfandel Lane, which equates to a growth factor of 1.72 for 20 years. The future traffic volumes on SR 29 arrived at by applying this growth factor are summarized in Table 7.

Table 7 – Future Traffic Volumes

Study Segment	Weekday		Weekday PM Peak		Saturday		Weekend MD Peak	
	SB	NB	SB	NB	SB	NB	SB	NB
SR 29 North of Zinfandel Ln	13,830	10,270	1,383	1,027	16,995	16,709	1,545	1,519
SR 29 South of Zinfandel Ln	13,480	10,540	1,348	1,054	16,500	18,282	1,500	1,662

Under the anticipated Future volumes, the study intersection is expected to operate unacceptably at LOS F overall as well on both the eastbound and westbound stop-controlled approaches. Future volumes are shown in Figure 1 and operating conditions are summarized in Table 8.

Table 8 – Future Peak Hour Intersection Levels of Service

Study Intersection <i>Approach</i>	PM Peak		Midday Peak	
	Delay	LOS	Delay	LOS
SR 29/Zinfandel Ln	103.1	F	**	F
<i>Westbound Approach</i>	**	F	**	F
<i>Eastbound Approach</i>	**	F	**	F

Notes: Delay is measured in average seconds per vehicle; LOS = Level of Service; Results for minor approaches to two-way stop-controlled intersections are indicated in *italics*; ** = delay greater than 120 seconds; **Bold** text = deficient operation

Recommendation – The County and Caltrans should consider installing a traffic signal at the intersection of SR 29/Zinfandel Lane to avoid unacceptable operation of this intersection in the future. This project could be included in the County's Traffic Impact Fee program at such time as one is established.

Project Description

The project consists of a Use Permit Modification application to update the number of special events and tasting room visitors. Flora Springs Winery proposes to modify their use permit to allow a maximum of 100 visitors per day, two lunchtime events and two wine club events for up to 50 people, and two dinnertime events for up to 25 people per week. The application also includes three wine club release events for up to 250 guests, one trilogy wine club release event for up to 350 guests and one wine auction event for up to 60 guests annually. It is anticipated that some of the existing part-time staff would be converted to full-time staff with the additional guests and events.

Trip Generation

The anticipated trip generation for a proposed project is typically estimated using standard rates published by the Institute of Transportation Engineers (ITE) in *Trip Generation Manual*, 9th Edition, 2012. However, the publication contains no such information for a winery. Therefore, the County of Napa's Winery Traffic Information/Trip Generation Sheet was used to determine the anticipated traffic generated both the production that is already

permitted at the site and with the proposed changes to the Use Permit. The trip generation worksheet is provided in Appendix C.

As the County of Napa's Winery Traffic Information/Trip Generation Sheet does not include guidance on inbound versus outbound trips, it was assumed that 75 percent of trip ends at the winery would be outbound during the weekday p.m. peak hour since most of the trips would be associated with employees and customers leaving at closure of the winery. For the weekend midday peak hour it was assumed that inbound and outbound trip ends would be evenly split.

As indicated in Table 9, the proposed change to the Flora Springs Winery's Use Permit is expected to result in an average of 42 additional trip ends on weekdays and 19 on weekend days, including 11 additional trip ends each during the weekday p.m. and weekday midday peak periods.

Table 9 – Trip Generation Summary

Land Use	Daily		Weekday PM Peak Hour			Weekend MD Peak Hour		
	Weekday	Weekend	Trips	In	Out	Trips	In	Out
Existing								
Employees	45	9	14	3	11	3	2	1
Tasting Visitors	52	52	24	7	17	32	16	16
Truck Traffic	2	0	0	0	0	0	0	0
Subtotal	99	61	38	10	28	35	18	17
Proposed								
Employees	51	9	16	4	12	3	1	2
Tasting Visitors	77	71	33	8	25	43	22	21
Truck Traffic	2	0	0	0	0	0	0	0
Subtotal	130	80	49	12	37	46	23	23
Net New Trips	42	19	11	2	9	11	5	6

Note: Trip generation as estimated above does not include special events

Trip Distribution

The pattern used to allocate new project trips to the street network was based on turning movements at the intersection of SR 29 and Zinfandel Lane. The applied distribution assumptions and resulting trips are shown in Table 10.

Table 10 – Trip Distribution Assumptions

Route	Percent	Daily Trips	Weekend Trips	PM Trips	Midday Trips
to/from SR 29 North	40%	17	8	4	4
to/from SR 29 South	55%	23	10	6	6
to/from Zinfandel Ln East	5%	2	1	1	1
TOTAL	100%	42	19	11	11

Special Events

Three different sized wine-marketing and agricultural promotion events are proposed as part of the Use Permit modification, including two 25-person events and four 50-person events per week, and one 60-person event, three 250-person events, and one 350-person event per year. It was assumed that a maximum-sized 350-person event would require a staff of 18 in addition to any winery staff that would assist with the event. Using the County-established occupancy of 2.8 persons per vehicle for guests and solo occupancy for staff, a maximum-sized 350-person event would be expected to generate 290 trip ends on the west leg of Zinfandel Lane, including 145 inbound trips and 145 outbound trips. This once-per-year event is proposed to take place between 11:00 a.m. to 10:00 p.m., with staff arriving an hour or more prior to the start of the event and leaving an hour or more after its conclusion. Guests would generate 125 trips for arrival and 125 for departure, with these trips generally occurring over more than an hour's time. Distributed evenly to the north and south, the maximum-sized special event would be expected to generate a maximum of 67 trips during any single hour on each of the segments of SR 29 to the north and south of Zinfandel Lane. Given that this and the other large special events are infrequent, are not part of typical daily operation, and often occur outside the peak period for traffic, together with the minimal increase in traffic associated with all but the very largest, special event traffic was not included in the daily trip generation and resulting intersection operation analysis.

Intersection Operation

Existing plus Project Conditions

Volumes on SR 29 north and south of Zinfandel Lane both without and with project trips included are shown in Table 11. As can readily be seen, the 11 hourly trips result in a maximum of 4 additional trips on any single segment of SR 29.

Table 11 – Existing and Existing plus Project Traffic Volumes

Study Segment	Existing Conditions				Existing plus Project Conditions			
	Weekday PM Peak		Weekend Midday Peak		Weekday PM Peak		Weekend Midday Peak	
	SB	NB	SB	NB	SB	NB	SB	NB
SR 29 North of Zinfandel Ln	804	597	898	883	805	601	900	886
SR 29 South of Zinfandel Ln	784	613	872	966	788	615	875	969

Upon the addition of project-related traffic to the Existing volumes, the intersection is expected to continue operating acceptably overall at LOS A during the weekday p.m. peak period and LOS C during the weekend midday peak period. The westbound approach is expected to continue operating at LOS F during both peak periods and the eastbound approach is expected to continue operating at LOS E during the weekend midday peak period; no changes in service level are projected as a result of the project. These results are summarized in Table 12. Project traffic volumes are shown in Figure 1.

Table 12 – Existing and Existing plus Project Peak Hour Intersection Levels of Service

Study Intersection Approach	Existing Conditions				Existing plus Project			
	PM Peak		Midday Peak		PM Peak		Midday Peak	
	Delay	LOS	Delay	LOS	Delay	LOS	Delay	LOS
SR 29/Zinfandel Ln	3.2	A	17.0	C	3.5	A	17.9	C
<i>Westbound Approach</i>	48.4	E	**	F	49.9	E	**	F
<i>Eastbound Approach</i>	28.9	D	114.0	F	32.0	D	114.9	F

Notes: Delay is measured in average seconds per vehicle; LOS = Level of Service; Results for minor approaches to two-way stop-controlled intersections are indicated in *italics*; ** = delay greater than 120 seconds; **Bold** text = deficient operation

Finding – The study intersections are expected to continue operating acceptably at the same levels of service upon the addition of project-generated traffic.

Existing plus Approved plus Project Conditions

Existing plus Approved volumes on SR 29 north and south of Zinfandel Lane are shown in Table 13 along with the projected volumes upon adding the traffic associated with the proposed Use Permit modification.

Table 13 – Existing plus Approved and Existing plus Approved plus Project Traffic Volumes

Study Segment	Existing plus Approved Conditions				Existing plus Approved plus Project			
	Weekday PM Peak		Weekend Midday Peak		Weekday PM Peak		Weekend Midday Peak	
	SB	NB	SB	NB	SB	NB	SB	NB
SR 29 North of Zinfandel Ln	808	613	913	898	809	617	915	901
SR 29 South of Zinfandel Ln	792	628	889	983	796	630	892	986

With project-related traffic added to Existing plus Approved volumes, the intersection is expected to operate acceptably overall at LOS A during the weekday p.m. peak period, though it will drop to LOS D overall during the weekend midday peak period. Because operation remains at the LOS C/D threshold (0.2 seconds into the LOS D range), this operation would continue to be acceptable under the Caltrans standards applied. The westbound approach is expected to continue operating at LOS F during both peak periods, with the eastbound approach at LOS F during the weekend midday peak period. These results are summarized in Table 14.

Table 14 – Existing plus Approved and Existing plus Approved plus Project Peak Hour Levels of Service

Study Intersection Approach	Existing plus Approved Conditions				Existing plus Approved plus Project			
	PM Peak		Midday Peak		PM Peak		Midday Peak	
	Delay	LOS	Delay	LOS	Delay	LOS	Delay	LOS
SR 29/Zinfandel Ln	4.1	A	24.0	C	4.4	A	25.2	D
Westbound Approach	59.2	F	**	F	61.4	F	**	F
Eastbound Approach	29.8	D	**	F	33.2	D	**	F

Notes: Delay is measured in average seconds per vehicle; LOS = Level of Service; Results for minor approaches to two-way stop-controlled intersections are indicated in *italics*; ** = delay greater than 120 seconds; **Bold** text = deficient operation

Finding – The study intersections are expected to continue operating acceptably at LOS A during the weekday p.m. peak period and at the LOS C/D threshold during the weekend midday peak period upon the addition of project-generated traffic.

Future plus Project Conditions

Future volumes on SR 29 north and south of Zinfandel Lane are shown in Table 15, both without project traffic added and with it.

Table 15 – Future and Future plus Project Traffic Volumes

Study Segment	Future Conditions				Future plus Project Conditions			
	Weekday PM Peak		Weekend Midday Peak		Weekday PM Peak		Weekend Midday Peak	
	SB	NB	SB	NB	SB	NB	SB	NB
SR 29 North of Zinfandel Ln	1,383	1,027	1,545	1,519	1,384	1,031	1,547	1,522
SR 29 South of Zinfandel Ln	1,348	1,054	1,500	1,662	1,353	1,055	1,503	1,665

Upon the addition of project-generated traffic to the anticipated Future volumes, the study intersection is expected to continue to operate unacceptably at LOS F overall as well on the eastbound and westbound approaches. The Future plus Project operating conditions are summarized in Table 16.

Table 16 – Future and Future plus Project Peak Hour Levels of Service

Study Intersection Approach	Future Conditions				Future plus Project			
	PM Peak		Midday Peak		PM Peak		Midday Peak	
	Delay	LOS	Delay	LOS	Delay	LOS	Delay	LOS
SR 29/Zinfandel Ln	103.1	F	**	F	110.3	F	**	F
Westbound Approach	**	F	**	F	**	F	**	F
Eastbound Approach	**	F	**	F	**	F	**	F

Notes: Delay is measured in average seconds per vehicle; LOS = Level of Service; Results for minor approaches to two-way stop-controlled intersections are indicated in *italics*; ** = delay greater than 120 seconds; **Bold** text = deficient operation

Finding – The study intersection will continue operating unacceptably with project traffic added, at the same Levels of Service as without it. Under Future conditions, project trips contribute 0.42 percent of total trips through the intersection during the weekday p.m. peak period and 0.32 percent of total trips during the weekend midday peak period, both of which are less than the 1 percent threshold at which the increase is considered significant. The impact is therefore *less-than-significant*.

Recommendation – The applicant should pay appropriate traffic impact fees if established prior to completion of this application.

Access and Circulation

Site Access

Flora Springs Winery is currently accessed by a private driveway that connects to Zinfandel Lane where it terminates. Because of the alignment there is no need for sight distance analysis as there would be no right or left turns; all visitors and employees continue to drive straight on Zinfandel Lane until it turns into the project driveway.

At the intersection of SR 29/Zinfandel Lane, there are existing left-turn lanes on both the northbound and southbound SR 29 approaches.

On-Site Circulation

The site layout is not changing with the addition of tasting room visitors or special events. A field review of the site indicates adequate space for vehicle and truck circulation.

Conclusions and Recommendations

Conclusions

- The proposed changes to the Use Permit would generate a net increase of 42 daily trips, including 11 weekday p.m. peak period trips and 11 weekend midday peak period trips.
- The study intersection of SR 29/Zinfandel Lane is expected to continue to operate acceptably under Existing and Existing plus Approved conditions without and with project-added trips.
- The intersection is expected to operate unacceptably under Future conditions and Future plus Project conditions. Because project trips account for less than 1 percent of future volumes, the impact is considered less-than-significant.
- The largest special event would be one 350-person event per year with approximately 18 special event employees.
- There is no change in site access or site circulation.

Recommendations

- Should the County establish a traffic impact fee prior to the processing of the application to modify the Use Permit, the applicant should pay appropriate mitigation fees to contribute to future infrastructure improvements that could include signalization of SR 29/Zinfandel Lane.

Study Participants and References

Study Participants

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Technician/Graphics	Deborah J. Mizell
Editing/Formatting	Angela McCoy

References

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NAX098



Appendix A

Collision Rate Calculations

Intersection Collision Rate Calculations

Flora Springs Winery

Intersection # 1: SR 29 & Zinfandel Ln
 Date of Count: Tuesday, June 30, 2015

Number of Collisions: 17
 Number of Injuries: 4
 Number of Fatalities: 0
 ADT: 15,100
 Start Date: October 1, 2008
 End Date: September 30, 2013
 Number of Years: 5

Intersection Type: Four-Legged
 Control Type: Stop & Yield Controls
 Area: Rural

$$\text{collision rate} = \frac{\text{Number of Collisions} \times 1 \text{ Million}}{\text{ADT} \times 365 \text{ Days per Year} \times \text{Number of Years}}$$

$$\text{collision rate} = \frac{17}{15,100} \times \frac{1,000,000}{365 \times 5}$$

	Collision Rate	Fatality Rate	Injury Rate
Study Intersection	0.62 c/mve	0.0%	23.5%
Statewide Average*	0.23 c/mve	2.0%	40.4%

ADT = average daily total vehicles entering intersection
 c/mve = collisions per million vehicles entering intersection
 * 2010 Collision Data on California State Highways, Caltrans

Appendix B

Intersection Level of Service Calculations

PM Peak Hour - Existing plus Project Conditions
 Flora Springs Winery Use Permit Modification
 County of Napa

Trip Generation Report

Forecast for PM Project

Zone #	Subzone	Amount	Units	Rate		Trips		Total % Of Trips Total
				In	Out	In	Out	
1	Flora Spring	1.00	Winery	2.00	9.00	2	9	11 100.0
	Zone 1 Subtotal					2	9	11 100.0
TOTAL								
						2	9	11 100.0

Saturday Midday Peak Hour - Existing Conditions
 Flora Springs Winery Use Permit Modification
 County of Napa

Trip Generation Report

Forecast for MD Project

Zone #	Subzone	Amount	Units	Rate		Trips		Total % Of Trips Total
				In	Out	In	Out	
1	Flora Spring	1.00	Winery	5.00	6.00	5	6	11 100.0
	Zone 1 Subtotal					5	6	11 100.0
TOTAL								
						5	6	11 100.0

Level Of Service Computation Report
2000 HCM Unsignalized Method (Future Volume Alternative)
Intersection #1 SR 29/Zinfandel

Average Delay (sec/veh): 3.5 Worst Case Level Of Service: E[49.9]

Street Name: SR 29 Zinfandel
Approach: North Bound South Bound East Bound West Bound
Movement: L - T - R L - T - R L - T - R L - T - R
Control: Uncontrolled Uncontrolled Stop Sign Stop Sign
Rights: Include Include Include Include
Lanes: 1 0 0 1 0 1 0 0 1 0 0 0 1 0 0 0

Volume Module: >> Count Date: 30 Jun 2015 << 16:30-17:30
Base Vol: 2 551 60 63 738 3 5 2 11 35 1 41
Growth Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
Initial Bse: 2 551 60 63 738 3 5 2 11 35 1 41
Added Vol: 1 0 0 0 1 4 0 5 0 0 0 0
PasserByVol: 0 0 0 0 0 0 0 0 0 0 0 0
Initial Fut: 3 551 60 63 738 4 9 2 16 35 1 41
User Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
PHF Adj: 0.96 0.96 0.96 0.96 0.96 0.96 0.96 0.96 0.96 0.96 0.96
PHF Volume: 3 574 63 66 769 4 9 2 17 36 1 43
Reduct Vol: 0 0 0 0 0 0 0 0 0 0 0 0
FinalVolume: 3 574 63 66 769 4 9 2 17 36 1 43

Critical Gap Module:
Critical Gap: 4.1 xxxx xxxxxx 4.1 xxxx xxxxxx 7.1 6.5 6.2 7.1 6.5 6.2
FollowUpTime: 2.2 xxxx xxxxxx 2.2 xxxx xxxxxx 3.5 4.0 3.3 3.5 4.0 3.3

Capacity Module:
Conflict Vol: 773 xxxx xxxxxx 636 xxxx xxxxxx 1535 1545 771 1523 1516 605
Potential Cap.: 851 xxxx xxxxxx 957 xxxx xxxxxx 96 116 403 98 121 501
Move Cap.: 851 xxxx xxxxxx 957 xxxx xxxxxx 82 107 403 87 112 501
Volume/Cap: 0.00 xxxx xxxxxx 0.07 xxxx xxxxxx 0.11 0.02 0.04 0.42 0.01 0.09

Level Of Service Module:
2May95thQ: 0.0 xxxx xxxxxx 0.2 xxxx xxxxxx xxxx xxxxxx xxxx xxxxxx
Control Del: 9.2 xxxx xxxxxx 9.0 xxxx xxxxxx xxxx xxxxxx xxxx xxxxxx
LOS by Move: A A A A A A A A A A A A
Movement: LT - LTR - RT LT - LTR - RT LT - LTR - RT LT - LTR - RT
Shared Cap.: xxxx xxxx xxxx xxxx xxxx xxxx 161 xxxx xxxx 157 xxxx
SharedQueue:xxxx xxxx xxxx xxxx xxxx xxxx 0.6 xxxx xxxx 2.5 xxxx
Shrd ConDel:xxxx xxxx xxxx xxxx xxxx xxxx 32.0 xxxx xxxx 49.9 xxxx
Shared LOS: *
ApproachDel: xxxxxx xxxxxx xxxxxx 32.0 49.9
ApproachLOS: *

Note: Queue reported is the number of cars per lane.

Level Of Service Computation Report
2000 HCM Unsignalized Method (Future Volume Alternative)
Intersection #1 SR 29/Zinfandel

Average Delay (sec/veh): 17.9 Worst Case Level Of Service: F[259.5]

Street Name: SR 29 Zinfandel
Approach: North Bound South Bound East Bound West Bound
Movement: L - T - R L - T - R L - T - R L - T - R
Control: Uncontrolled Uncontrolled Stop Sign Stop Sign
Rights: Include Include Include Include
Lanes: 1 0 0 1 0 1 0 0 1 0 0 0 1 0 0 0

Volume Module: >> Count Date: 27 Jun 2015 << 14:45-15:45
Base Vol: 7 793 166 74 821 3 10 1 4 47 0 80
Growth Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
Initial Bse: 7 793 166 74 821 3 10 1 4 47 0 80
Added Vol: 3 0 0 0 2 0 0 0 3 0 0 0
PasserByVol: 0 0 0 0 0 0 0 0 0 0 0 0
Initial Fut: 10 793 166 74 821 5 12 1 7 47 0 80
User Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
PHF Adj: 0.98 0.98 0.98 0.98 0.98 0.98 0.98 0.98 0.98 0.98 0.98
PHF Volume: 10 809 169 76 838 5 12 1 7 48 0 82
Reduct Vol: 0 0 0 0 0 0 0 0 0 0 0 0
FinalVolume: 10 809 169 76 838 5 12 1 7 48 0 82

Critical Gap Module:
Critical Gap: 4.1 xxxx xxxxxx 4.1 xxxx xxxxxx 7.1 6.5 6.2 7.1 6.5 6.2
FollowUpTime: 2.2 xxxx xxxxxx 2.2 xxxx xxxxxx 3.5 4.0 3.3 3.5 4.0 3.3

Capacity Module:
Conflict Vol: 843 xxxx xxxxxx 979 xxxx xxxxxx 1946 1990 840 1910 1908 894
Potential Cap.: 802 xxxx xxxxxx 713 xxxx xxxxxx 49 61 368 52 69 343
Move Cap.: 802 xxxx xxxxxx 713 xxxx xxxxxx 34 54 368 46 61 343
Volume/Cap: 0.01 xxxx xxxxxx 0.11 xxxx xxxxxx 0.36 0.02 0.02 1.04 0.00 0.24

Level Of Service Module:
2May95thQ: 0.0 xxxx xxxxxx 0.4 xxxx xxxxxx xxxx xxxxxx xxxx xxxxxx
Control Del: 9.5 xxxx xxxxxx 10.6 xxxx xxxxxx xxxx xxxxxx xxxx xxxxxx
LOS by Move: A A B
Movement: LT - LTR - RT LT - LTR - RT LT - LTR - RT LT - LTR - RT
Shared Cap.: xxxx xxxx xxxx xxxx xxxx xxxx 52 xxxx xxxx 101 xxxx
SharedQueue:xxxx xxxx xxxx xxxx xxxx xxxx 1.4 xxxx xxxx 9.0 xxxx
Shrd ConDel:xxxx xxxx xxxx xxxx xxxx xxxx 115 xxxx xxxx 259 xxxx
Shared LOS: *
ApproachDel: xxxxxx xxxxxx 114.9 259.5
ApproachLOS: *

Note: Queue reported is the number of cars per lane.

PM Peak Hour - Existing plus Approved
Flora Springs Winery Use Permit Modification
County of Napa

Trip Generation Report

Forecast for PM Approved

Zone #	Subzone	Amount	Units	Rate		Trips		Total % Of Trips Total
				In	Out	In	Out	
2		1.00	Accendo Cellar	3.00	10.00	3	10	13 26.0
	Zone 2 Subtotal					3	10	13 26.0
3		1.00	WHL Winery	3.00	9.00	3	9	12 24.0
	Zone 3 Subtotal					3	9	12 24.0
4		1.00	Rutherford Est	6.00	19.00	6	19	25 50.0
	Zone 4 Subtotal					6	19	25 50.0
TOTAL								
							12 38	50 100.0

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Saturday Midday Peak Hour - Existing plus Approved
Flora Springs Winery Use Permit Modification
County of Napa

Trip Generation Report

Forecast for MD Approved

Zone #	Subzone	Amount	Units	Rate		Trips		Total % Of Trips Total
				In	Out	In	Out	
2		1.00	Accendo Cellar	9.00	9.00	9	9	18 25.7
	Zone 2 Subtotal					9	9	18 25.7
3		1.00	WHL Winery	9.00	9.00	9	9	18 25.7
	Zone 3 Subtotal					9	9	18 25.7
4		1.00	Rutherford Est	17.00	17.00	17	17	34 48.6
	Zone 4 Subtotal					17	17	34 48.6
TOTAL								
							35 35	70 100.0

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PM Peak Hour - Existing plus Approved
Flora Springs Winery Use Permit Modification
County of Napa

Level of Service Computation Report
2000 HCM Unsignalized Method (Future Volume Alternative)

Intersection #1 SR 29/Zinfandel

Average Delay (sec/veh): 4.1 Worst Case Level of Service: F [59.2]

Street Name: SR 29 Zinfandel

Approach:	North Bound	South Bound	East Bound	West Bound
Movement:	L - T - R	L - T - R	L - T - R	L - T - R
Control:	Uncontrolled	Uncontrolled	Uncontrolled	Uncontrolled
Rights:	Include	Include	Include	Include
Lanes:	1 0 0 1 0	1 0 0 1 0	0 0 1 0 0	0 0 1 0 0

Volume Module: >> Count Date: 30 Jun 2015 << 16:30-17:30

Base Vol:	2 551 60 63 738 3 5 2 11 35 1 41
Growth Adj:	1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
Initial Bse:	2 551 60 63 738 3 5 2 11 35 1 41
Added Vol:	0 11 3 1 4 0 0 0 0 0 0 0 4
PasserByVol:	0 0 0 0 0 0 0 0 0 0 0 0 0
Initial Fut:	2 562 63 64 742 3 5 2 11 41 1 45
User Adj:	1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
PHF Adj:	0.96 0.96 0.96 0.96 0.96 0.96 0.96 0.96 0.96 0.96 0.96
PHF Volume:	2 585 66 67 773 3 5 2 11 43 1 47
Reduct Vol:	0 0 0 0 0 0 0 0 0 0 0 0 0
FinalVolume:	2 585 66 67 773 3 5 2 11 43 1 47

Critical Gap Module:

Critical Gap:	4.1 xxxxx xxxxxx	4.1 xxxxx xxxxxx	7.1 6.5 6.2	7.1 6.5 6.2
FollowUpTime:	2.2 xxxxx xxxxxx	2.2 xxxxx xxxxxx	3.5 4.0 3.3	3.5 4.0 3.3

Capacity Module:

Conflict Vol:	776 xxxxx xxxxxx	651 xxxxx xxxxxx	1554 1563	774 1537 1532	618
Potent Cap.:	849 xxxxx xxxxxx	945 xxxxx xxxxxx	93 113 401	96 118 493	493
Move Cap.:	849 xxxxx xxxxxx	945 xxxxx xxxxxx	79 105 401	86 109 493	493
Volume/Cap:	0.00 xxxxx xxxxx	0.07 xxxxx xxxxx	0.07 0.02 0.03	0.49 0.01 0.10	0.10

Level of Service Module:

2Way95thQ:	0.0 xxxxx xxxxxx	0.2 xxxxx xxxxxx	xxxx xxxxx xxxxxx	xxxx xxxxx xxxxxx
Control Del:	9.3 xxxxx xxxxx	9.1 xxxxx xxxxxx	xxxx xxxxx xxxxx	xxxx xxxxx xxxxx
LOS by Move:	A	A	*	*
Movement:	LT - LTR - RT	LT - LTR - RT	LT - LTR - RT	LT - LTR - RT
Shared Cap.:	xxxx xxxxx xxxxx	xxxx xxxxx xxxxx	164 xxxxx xxxxx	151 xxxxx
SharedQueue:	xxxx xxxxx xxxxx	xxxx xxxxx xxxxx	0.4 xxxxx xxxxx	3.2 xxxxx
Shrd ConDel:	xxxx xxxxx xxxxx	xxxx xxxxx xxxxx	29.8 xxxxx xxxxx	59.2 xxxxx
Shared LOS:	*	*	D	F
ApproachDel:	xxxxxx	xxxxxx	29.8	59.2
ApproachLOS:	*	*	D	F

Note: Queue reported is the number of cars per lane.

Saturday Midday Peak Hour - Existing plus Approved
Flora Springs Winery Use Permit Modification
County of Napa

Level of Service Computation Report
2000 HCM Unsignalized Method (Future Volume Alternative)

Intersection #1 SR 29/Zinfandel

Average Delay (sec/veh): 24.0 Worst Case Level of Service: F [337.8]

Street Name: SR 29 Zinfandel

Approach:	North Bound	South Bound	East Bound	West Bound
Movement:	L - T - R	L - T - R	L - T - R	L - T - R
Control:	Uncontrolled	Uncontrolled	Uncontrolled	Uncontrolled
Rights:	Include	Include	Include	Include
Lanes:	1 0 0 1 0	1 0 0 1 0	0 0 1 0 0	0 0 1 0 0

Volume Module: >> Count Date: 27 Jun 2015 << 14:45-15:45

Base Vol:	7 793 166 74 821 3 10 1 4 47 0 80
Growth Adj:	1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
Initial Bse:	7 793 166 74 821 3 10 1 4 47 0 80
Added Vol:	0 10 6 4 10 0 0 0 0 0 0 4
PasserByVol:	0 0 0 0 0 0 0 0 0 0 0 0
Initial Fut:	7 803 172 78 831 3 10 1 4 53 0 84
User Adj:	1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
PHF Adj:	0.98 0.98 0.98 0.98 0.98 0.98 0.98 0.98 0.98 0.98 0.98
PHF Volume:	7 819 176 80 848 3 10 1 4 54 0 86
Reduct Vol:	0 0 0 0 0 0 0 0 0 0 0 0
FinalVolume:	7 819 176 80 848 3 10 1 4 54 0 86

Critical Gap Module:

Critical Gap:	4.1 xxxxx xxxxxx	4.1 xxxxx xxxxxx	7.1 6.5 6.2	7.1 6.5 6.2
FollowUpTime:	2.2 xxxxx xxxxxx	2.2 xxxxx xxxxxx	3.5 4.0 3.3	3.5 4.0 3.3

Capacity Module:

Conflict Vol:	851 xxxxx xxxxxx	985 xxxxx xxxxxx	1973 2018	849 1933 1932	907
Potent Cap.:	796 xxxxx xxxxxx	703 xxxxx xxxxxx	47 59 364	50 67 337	337
Move Cap.:	796 xxxxx xxxxxx	703 xxxxx xxxxxx	32 52 364	45 59 337	337
Volume/Cap:	0.01 xxxxx xxxxx	0.11 xxxxx xxxxx	0.32 0.02 0.01	1.21 0.00 0.25	0.25

Level of Service Module:

2Way95thQ:	0.0 xxxxx xxxxxx	0.4 xxxxx xxxxxx	xxxx xxxxx xxxxxx	xxxx xxxxx xxxxxx
Control Del:	9.6 xxxxx xxxxx	10.8 xxxxx xxxxxx	xxxx xxxxx xxxxx	xxxx xxxxx xxxxx
LOS by Move:	A	B	*	*
Movement:	LT - LTR - RT	LT - LTR - RT	LT - LTR - RT	LT - LTR - RT
Shared Cap.:	xxxx xxxxx xxxxx	xxxx xxxxx xxxxx	xxxx xxxxx xxxxx	44 xxxxx xxxxx
SharedQueue:	xxxx xxxxx xxxxx	xxxx xxxxx xxxxx	xxxx xxxxx xxxxx	1.2 xxxxx xxxxx
Shrd ConDel:	xxxx xxxxx xxxxx	xxxx xxxxx xxxxx	xxxx xxxxx xxxxx	127 xxxxx xxxxx
Shared LOS:	*	*	F	F
ApproachDel:	xxxxxx	xxxxxx	126.6	337.8
ApproachLOS:	*	*	F	F

Note: Queue reported is the number of cars per lane.

PM Peak Hour - Existing plus Approved plus Project
Flora Springs Winery Use Permit Modification
County of Napa

Trip Generation Report

Forecast for PM Project

Zone #	Subzone	Amount	Units	Rate		Trips		Total % Of	
				In	Out	In	Out	Trips	Total
1	Flora Spring	1.00	Winery	2.00	9.00	2	9	11	18.0
	Zone 1 Subtotal					2	9	11	18.0
TOTAL						2	9	11	18.0

Saturday Midday Peak Hour - Existing plus Approved plus Project
Flora Springs Winery Use Permit Modification
County of Napa

Trip Generation Report

Forecast for MD Project

Zone #	Subzone	Amount	Units	Rate		Trips		Total % Of	
				In	Out	In	Out	Trips	Total
1	Flora Spring	1.00	Winery	5.00	6.00	5	6	11	13.6
	Zone 1 Subtotal					5	6	11	13.6
TOTAL						5	6	11	13.6

PM Peak Hour - Existing plus Approved plus Project
Flora Springs Winery Use Permit Modification
County of Napa

Trip Generation Report

Forecast for PM Approved

Zone #	Subzone	Amount	Units	Rate		Trips		Total % Of Trips Total
				In	Out	In	Out	
2	1.00 Accendo Cellar	3.00	10.00	3	10	3	10	13 21.3
	Zone 2 Subtotal			3	10	3	10	13 21.3
3	1.00 WHL Winery	3.00	9.00	3	9	3	9	12 19.7
	Zone 3 Subtotal			3	9	3	9	12 19.7
4	1.00 Rutherford Est	6.00	19.00	6	19	6	19	25 41.0
	Zone 4 Subtotal			6	19	6	19	25 41.0
TOTAL								
							12 38	50 82.0

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Saturday Midday Peak Hour - Existing plus Approved plus Project
Flora Springs Winery Use Permit Modification
County of Napa

Trip Generation Report

Forecast for MD Approved

Zone #	Subzone	Amount	Units	Rate		Trips		Total % Of Trips Total
				In	Out	In	Out	
2	1.00 Accendo Cellar	9.00	9.00	9	9	9	9	18 22.2
	Zone 2 Subtotal			9	9	9	9	18 22.2
3	1.00 WHL Winery	9.00	9.00	9	9	9	9	18 22.2
	Zone 3 Subtotal			9	9	9	9	18 22.2
4	1.00 Rutherford Est	17.00	17.00	17	17	17	17	34 42.0
	Zone 4 Subtotal			17	17	17	17	34 42.0
TOTAL								
							35 35	70 86.4

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PM Peak Hour - Future plus Project Conditions
Flora Springs Winery Use Permit Modification
County of Napa

Level of Service Computation Report
2000 HCM Unsignalized Method (Future Volume Alternative)

Intersection #1 SR 29/Zinfandel

Average Delay (sec/veh): 110.3 Worst Case Level of Service: F(2020.4)

Street Name: SR 29 Zinfandel

Approach: North Bound South Bound East Bound West Bound

Movement: L - T - R L - T - R L - T - R L - T - R

Control: Uncontrolled Uncontrolled Uncontrolled Uncontrolled

Rights: Include Include Include Include

Lanes: 1 0 0 1 0 1 0 0 1 0 0 0 1 0 0 0 1 0 0

Volume Module: >> Count Date: 30 Jun 2015 << 16:30-17:30

Base Vol: 2 551 60 63 738 3 5 2 11 35 1 41

Growth Adj: 1.72 1.72 1.72 1.72 1.72 1.72 1.72 1.72 1.72 1.72 1.72 1.72

Initial Bse: 3 948 103 108 1269 5 9 3 19 60 2 71

Added Vol: 1 0 0 0 0 1 4 0 5 0 0 0

PasserByVol: 0 0 0 0 0 0 0 0 0 0 0 0

Initial Fut: 4 948 103 108 1269 6 13 3 24 60 2 71

User Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00

PHF Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00

PHF Volume: 4 948 103 108 1269 6 13 3 24 60 2 71

Reduct Vol: 0 0 0 0 0 0 0 0 0 0 0 0

FinalVolume: 4 948 103 108 1269 6 13 3 24 60 2 71

Critical Gap Module:

Critical Gap: 4.1 xxx xxxxxx 4.1 xxx xxxxxx 7.1 6.5 6.2 7.1 6.5 6.2

FollowUpTm: 2.2 xxx xxxxxx 2.2 xxx xxxxxx 3.5 4.0 3.3 3.5 4.0 3.3

Capacity Module:

Conflict Vol: 1276 xxx xxxxxx 1051 xxx xxxxxx 2533 2549 1272 2511 2500 999

Potent Cap.: 551 xxx xxxxxx 670 xxx xxxxxx 19 27 207 19 29 298

Move Cap.: 551 xxx xxxxxx 670 xxx xxxxxx 12 23 207 13 24 298

Volume/Cap: 0.01 xxx xxxxxx 0.16 xxx xxxxxx 1.07 0.15 0.12 4.54 0.07 0.24

Level of Service Module:

2Way95thQ: 0.0 xxx xxxxxx 0.6 xxx xxxxxx xxx xxx xxxxxx xxx xxx xxxxxx

Control Del: 11.6 xxx xxxxxx 11.4 xxx xxxxxx xxx xxx xxxxxx xxx xxx xxxxxx

LOS by Move: B B B B B B B B B B B B

Movement: LT - LTR - RT LT - LTR - RT LT - LTR - RT LT - LTR - RT

Shared Cap.: xxx xxx xxxxxx xxx xxx xxxxxx xxx xxx xxxxxx 30 xxxxxx 27 xxxxxx

SharedQueue: xxx xxx xxxxxx xxx xxx xxxxxx xxx xxx xxxxxx 4.6 xxxxxx 16.2 xxxxxx

Shrd ConDel: xxx xxx xxxxxx xxx xxx xxxxxx xxx xxx xxxxxx 481 xxxxxx 2020 xxxxxx

Shared LOS: F F F F F F F F F F F F

ApproachDel: xxxxxx xxxxxx 480.9 2020.4

ApproachLOS: F F F F F F F F F F F F

Note: Queue reported is the number of cars per lane.

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Saturday Midday Peak Hour - Future plus Project Conditions
Flora Springs Winery Use Permit Modification
County of Napa

Level of Service Computation Report
2000 HCM Unsignalized Method (Future Volume Alternative)

Intersection #1 SR 29/Zinfandel

Average Delay (sec/veh): 1126.2 Worst Case Level of Service: F(3844.0)

Street Name: SR 29 Zinfandel

Approach: North Bound South Bound East Bound West Bound

Movement: L - T - R L - T - R L - T - R L - T - R

Control: Uncontrolled Uncontrolled Uncontrolled Uncontrolled

Rights: Include Include Include Include

Lanes: 1 0 0 1 0 1 0 0 1 0 0 0 1 0 0 0 1 0 0

Volume Module: >> Count Date: 27 Jun 2015 << 14:45-15:45

Base Vol: 7 793 166 74 821 3 10 1 4 47 0 80

Growth Adj: 1.72 1.72 1.72 1.72 1.72 1.72 1.72 1.72 1.72 1.72 1.72 1.72

Initial Bse: 12 1364 286 127 1412 5 17 2 7 81 0 138

Added Vol: 3 0 0 0 0 2 0 0 3 0 0 0

PasserByVol: 0 0 0 0 0 0 0 0 0 0 0 0

Initial Fut: 15 1364 286 127 1412 7 19 2 10 81 0 138

User Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00

PHF Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00

PHF Volume: 15 1364 286 127 1412 7 19 2 10 81 0 138

Reduct Vol: 0 0 0 0 0 0 0 0 0 0 0 0

FinalVolume: 15 1364 286 127 1412 7 19 2 10 81 0 138

Critical Gap Module:

Critical Gap: 4.1 xxx xxxxxx 4.1 xxx xxxxxx 7.1 6.5 6.2 7.1 6.5 6.2

FollowUpTm: 2.2 xxx xxxxxx 2.2 xxx xxxxxx 3.5 4.0 3.3 3.5 4.0 3.3

Capacity Module:

Conflict Vol: 1419 xxx xxxxxx 1649 xxx xxxxxx 3276 3350 1416 3213 3211 1507

Potent Cap.: 486 xxx xxxxxx 397 xxx xxxxxx 5 8 170 6 10 150

Move Cap.: 486 xxx xxxxxx 397 xxx xxxxxx 0 5 170 3 7 150

Volume/Cap: 0.03 xxx xxxxxx 0.32 xxx xxxxxx 58.16 0.32 0.06 25.70 0.00 0.92

Level of Service Module:

2Way95thQ: 0.1 xxx xxxxxx 1.4 xxx xxxxxx xxx xxx xxxxxx xxx xxx xxxxxx

Control Del: 12.6 xxx xxxxxx 18.3 xxx xxxxxx xxx xxx xxxxxx xxx xxx xxxxxx

LOS by Move: B B B B B B B B B B B B

Movement: LT - LTR - RT LT - LTR - RT LT - LTR - RT LT - LTR - RT

Shared Cap.: xxx xxx xxxxxx xxx xxx xxxxxx xxx xxx xxxxxx 1 xxxxxx 8 xxxxxx

SharedQueue: xxx xxx xxxxxx xxx xxx xxxxxx xxx xxx xxxxxx 5.8 xxxxxx 29.1 xxxxxx

Shrd ConDel: xxx xxx xxxxxx xxx xxx xxxxxx xxx xxx xxxxxx xxx xxx xxxxxx

Shared LOS: F F F F F F F F F F F F

ApproachDel: xxxxxx xxxxxx

ApproachLOS: F F F F F F F F F F F F

Note: Queue reported is the number of cars per lane.

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Appendix C

Winery Trip Generation Worksheet

Winery Traffic Information / Trip Generation Sheet

Project Name: Flora Spring Winery

Project Scenario: Existing Conditions

Traffic during a Typical Weekday

Number of FT employees:	12	x 3.05 one-way trips per employee	=	37	daily trips.
Number of PT employees:	4	x 1.90 one-way trips per employee	=	8	daily trips.
Average number of weekday visitors:	68	/ 2.6 visitors per vehicle x 2 one-way trips	=	52	daily trips.
Gallons of production:	120000	/ 1,000 x .009 truck trips daily ³ x 2 one-way trips	=	2	daily trips.
Total				=	99 daily trips.
Number of total weekday trips x .38				=	38 PM peak trips.

Traffic during a Typical Saturday

Number of FT employees (on Saturdays):	3	x 3.05 one-way trips per employee	=	9	daily trips.
Number of PT employees (on Saturdays):	0	x 1.90 one-way trips per employee	=	0	daily trips.
Average number of weekend visitors:	73	/ 2.8 visitors per vehicle x 2 one-way trips	=	52	daily trips.
Total				=	61 daily trips.
Number of total Saturday trips x .57				=	35 PM peak trips.

Traffic during a Crush Saturday

Number of FT employees (during crush):	12	x 3.05 one-way trips per employee	=	37	daily trips.
Number of PT employees (during crush):	4	x 1.90 one-way trips per employee	=	8	daily trips.
Average number of weekend visitors:	73	/ 2.8 visitors per vehicle x 2 one-way trips	=	52	daily trips.
Gallons of production:	120000	/ 1,000 x .009 truck trips daily x 2 one-way trips	=	2	daily trips.
Avg. annual tons of grape on-haul:	1400	x .11 truck trips daily ⁴ x 2 one-way trips	=	19	daily trips.
Total				=	118 daily trips.
Number of total Saturday trips x .57				=	67 PM peak trips.

Largest Marketing Event- Additional Traffic

Number of event staff (largest event):	6	x 2 one-way trips per staff person	=	12	trips.
Number of visitors (largest event):	70	/ 2.8 visitors per vehicle x 2 one-way trips	=	50	trips.
Number of special event truck trips (largest event):	2	x 2 one-way trips	=	4	trips.

³ Assumes 1.47 materials & supplies trips + 0.8 case goods trips per 1,000 gallons of production / 250 days per year (see *Traffic Information Sheet Addendum* for reference).

⁴ Assumes 4 tons per trip / 36 crush days per year (see *Traffic Information Sheet Addendum* for reference).

Winery Traffic Information / Trip Generation Sheet

Project Name: Flora Spring Winery

Project Scenario: Proposed Conditions

Traffic during a Typical Weekday

Number of FT employees:	16	x 3.05 one-way trips per employee	=	49	daily trips.
Number of PT employees:	1	x 1.90 one-way trips per employee	=	2	daily trips.
Average number of weekday visitors:	100	/ 2.6 visitors per vehicle x 2 one-way trips	=	77	daily trips.
Gallons of production:	120000	/ 1,000 x .009 truck trips daily ³ x 2 one-way trips	=	2	daily trips.
Total				130	daily trips.
Number of total weekday trips x .38				49	PM peak trips.

Traffic during a Typical Saturday

Number of FT employees (on Saturdays):	3	x 3.05 one-way trips per employee	=	9	daily trips.
Number of PT employees (on Saturdays):	0	x 1.90 one-way trips per employee	=	0	daily trips.
Average number of weekend visitors:	100	/ 2.8 visitors per vehicle x 2 one-way trips	=	71	daily trips.
Total				80	daily trips.
Number of total Saturday trips x .57				46	PM peak trips.

Traffic during a Crush Saturday

Number of FT employees (during crush):	16	x 3.05 one-way trips per employee	=	49	daily trips.
Number of PT employees (during crush):	8	x 1.90 one-way trips per employee	=	15	daily trips.
Average number of weekend visitors:	100	/ 2.8 visitors per vehicle x 2 one-way trips	=	71	daily trips.
Gallons of production:	120000	/ 1,000 x .009 truck trips daily x 2 one-way trips	=	2	daily trips.
Avg. annual tons of grape on-haul:	1400	x .11 truck trips daily ⁴ x 2 one-way trips	=	19	daily trips.
Total				157	daily trips.
Number of total Saturday trips x .57				89	PM peak trips.

Largest Marketing Event- Additional Traffic

Number of event staff (largest event):	18	x 2 one-way trips per staff person	=	36	trips.
Number of visitors (largest event):	350	/ 2.8 visitors per vehicle x 2 one-way trips	=	250	trips.
Number of special event truck trips (largest event):	2	x 2 one-way trips	=	4	trips.

³ Assumes 1.47 materials & supplies trips + 0.8 case goods trips per 1,000 gallons of production / 250 days per year (see *Traffic Information Sheet Addendum* for reference).

⁴ Assumes 4 tons per trip / 36 crush days per year (see *Traffic Information Sheet Addendum* for reference).