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## Traffic Study



# Traffic Impact Study for Sam Jasper Winery



Prepared for the County of Napa

Submitted by  
**W-Trans**

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**TRAFFIC ENGINEERING  
TRANSPORTATION PLANNING**  
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# Table of Contents

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Executive Summary .....	1
Introduction.....	2
Transportation Setting.....	4
Capacity Analysis .....	5
Access and Circulation.....	14
Conclusions and Recommendations.....	15
Study Participants and References.....	16

## Figures

1. Study Area and Traffic Volumes .....	3
2. Site Plan .....	10

## Tables

1. Collision Rates for the Study Segments.....	4
2. Automobile Level of Service Criteria .....	5
3. Existing Traffic Volumes .....	6
4. Existing Peak Hour Roadway Segment Levels of Service .....	6
5. Cumulative Traffic Volumes.....	7
6. Cumulative Peak Hour Roadway Segment Levels of Service .....	8
7. Future Traffic Volumes .....	8
8. Future Peak Hour Roadway Segment Levels of Service .....	8
9. Trip Generation.....	9
10. Existing and Existing plus Project Traffic Volumes near 4059 Silverado Trail .....	11
11. Existing and Existing plus Project Peak Hour Intersection Levels of Service on Silverado Trail .....	11
12. Cumulative and Cumulative plus Project Traffic Volumes near 4059 Silverado Trail.....	12
13. Cumulative and Cumulative plus Project Peak Hour Intersection LOS on Silverado Trail.....	12
14. Future and Future plus Project Traffic Volumes near 4059 Silverado Trail.....	12
15. Future and Future plus Project Peak Hour Intersection Levels of Service on Silverado Trail .....	13

## Appendices

- A Collision Rate Calculations
- B Intersection Level of Service Calculations
- C Trip Generation Worksheet
- D Left-Turn Lane Warrant



# Executive Summary

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The Sam Jasper Winery project is a winery with a proposed 20,000 gallons of wine per year, ten employees, 25 daily visitors, and special events with up to 50 people. The project's anticipated trip generation includes 50 daily trips on average during a weekday, with 19 trips during the p.m. peak hour and 27 during the weekend midday peak hour.

The study area was established by the County and includes Silverado Trail abutting the project site, as well as between Oak Knoll Avenue to the north and Petra Drive and Soda Canyon Road to the south. Analysis indicates that the study segment is operating acceptably for some peak periods and unacceptably for other peak periods under Existing, Cumulative, and Future conditions both with and without project traffic added. The project volumes have a less-than-significant impact under all scenarios, including those where the study segment is operating unacceptably without project-added volumes, as the project-added volumes represent less than one percent of projected future volumes.

Vehicles will access the project via an existing driveway on Silverado Trail. A left-turn lane is warranted under Existing plus Project conditions, and is recommended as part of the project. Sight distance at the project driveway location for both entering and exiting drivers as well as for approaching drivers is adequate.

Although the project's traffic impacts are expected to be less-than-significant, the applicant should consider implementing a transportation demand management program to limit employee trips during peak traffic periods through shift changes, carpooling, and similar measures.

# Introduction

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## Introduction

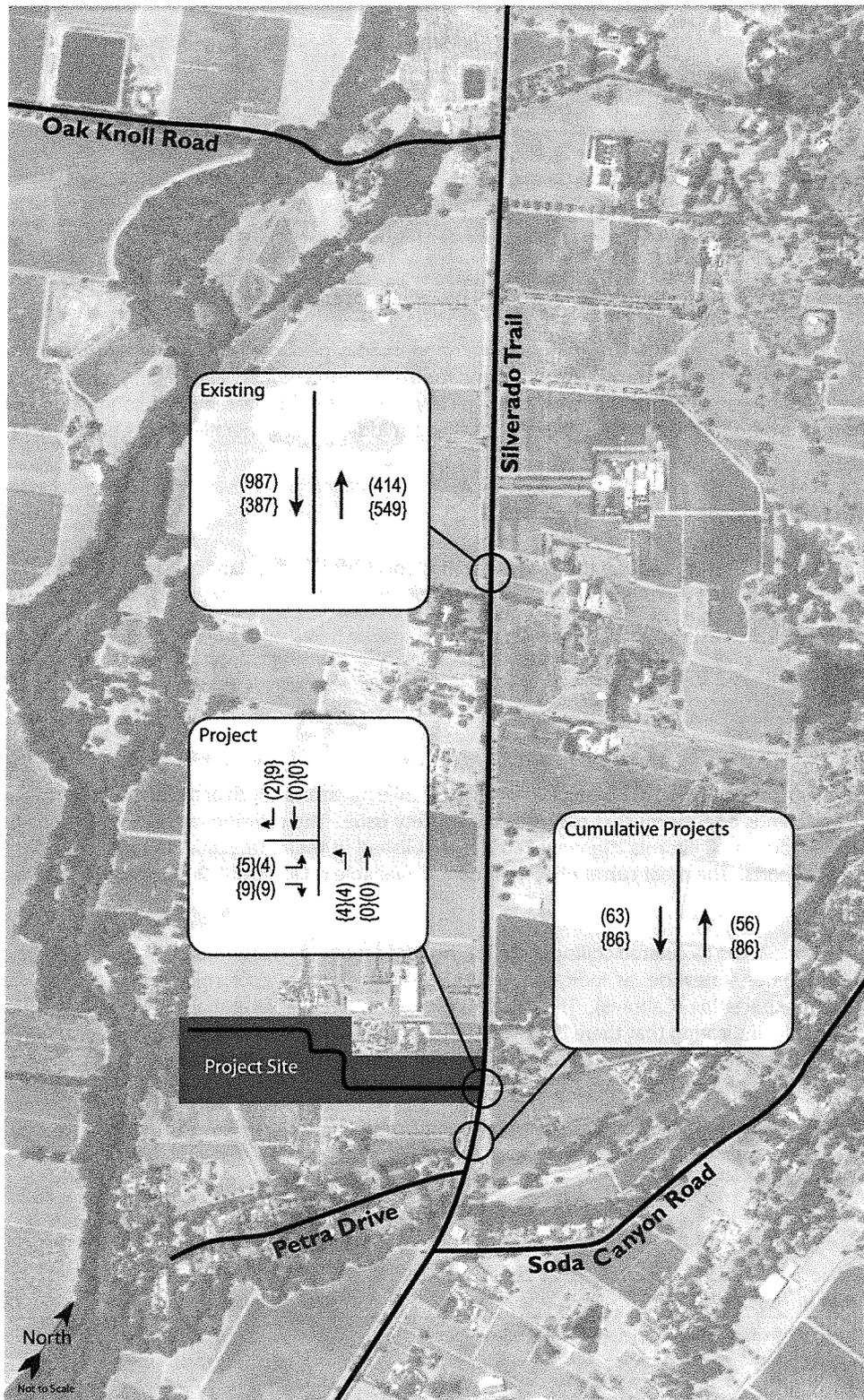
This report presents an analysis of the potential traffic impacts that would be associated with development of a proposed winery to be located at 4059 Silverado Trail in the County of Napa. The traffic study was completed in accordance with the criteria established by the County of Napa, and is consistent with standard traffic engineering techniques.

## Prelude

The purpose of a traffic impact study is to provide County staff and policy makers with data that they can use to make an informed decision regarding the potential traffic impacts of a proposed project, and any associated improvements that would be required in order to mitigate these impacts to a level of insignificance as defined by the County's General Plan or other policies. Vehicular traffic impacts are typically evaluated by determining the number of new trips that the proposed use would be expected to generate, distributing these trips to the surrounding street system based on existing travel patterns or anticipated travel patterns specific to the proposed project, then analyzing the impact the new traffic would be expected to have on roadway segments.

## Project Profile

The proposed project would include production of up to 20,000 gallons of wine annually and operation of a tasting room. Visitation to the tasting room would be by appointment-only and 23 special events per year are proposed. The project site is located at 4059 Silverado Trail, as shown in Figure 1.



LEGEND	
(xx)	P.M. Peak Hour Volume
{xx}	Weekend Midday Peak Hour Volume

**Traffic Impact Study for Sam Jasper Winery**  
**Figure 1 – Study Area and Traffic Volumes**



# Transportation Setting

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## Operational Analysis

### Study Area and Periods

The study area consists of the section of Silverado Trail providing access to the project site.

Operating conditions during the weekday p.m. and weekend midday peak periods were evaluated as these time periods reflect the highest traffic volumes areawide and for the proposed winery and tasting room. The evening peak hour occurs between 4:00 and 6:00 p.m. and typically reflects the highest level of congestion of the day during the homeward bound commute, while the weekend midday peak generally occurs between 11:00 a.m. and 1:00 p.m.

### Study Roadway

*Silverado Trail* runs north-south through the study area, has one 12-foot travel lane in each direction, bike lanes, and a posted speed limit of 55 miles per hour (mph). Based on the *Napa County General Plan, 2013*, Silverado Trail is classified as a rural throughway.

The study area is shown in Figure 1.

## Collision History

The collision history for the section of Silverado Trail one-half mile north and south of the project site was reviewed to determine any trends or patterns that may indicate a safety issue. The collision rate was calculated based on records available from the California Highway Patrol as published in their Statewide Integrated Traffic Records System (SWITRS) reports. The most current five-year period available is October 1, 2008, through September 30, 2013.

As presented in Table 1, the calculated collision rate for the study segment was compared to the average collision rate for similar facilities statewide, as indicated in *2012 Collision Data on California State Highways*, California Department of Transportation (Caltrans). The calculated collision rate is substantially lower than the statewide average collision rate, indicating that there is unlikely to be a specific safety concern. It was noted that 10 of the 16 crashes involved southbound drivers, and of these, seven occurred during the evening peak period when the predominant commute pattern is southbound. The collision rate spreadsheet is provided in Appendix A.

<b>Study Roadway Segments</b>	<b>Number of Collisions (2008-2013)</b>	<b>Calculated Collision Rate (c/mvm)</b>	<b>Statewide Average Collision Rate (c/mvm)</b>
Silverado Trail near 4059	16	0.63	1.29

Note: c/mvm = collisions per million vehicles miles

# Capacity Analysis

## Roadway Segment Level of Service Methodology

The roadway segment Level of Service methodology found in Chapter 15, "Two-Lane Highways," of the *Highway Capacity Manual* is the basis of the automobile LOS analysis. The methodology considers traffic volumes, terrain, roadway cross-section, the proportion of heavy vehicles, and the availability of passing zones. The LOS criteria for two-lane highways differs depending on whether the highway is considered "Class I", "Class II", or "Class III". Class I highways are typically long-distance routes connecting major traffic generators or national highway networks where motorists expect to travel at high speeds. Motorists do not necessarily expect to travel at high speeds on Class II highways, which often function as scenic or recreational routes and typically serve shorter trips. Class III highways may be portions of Class I or Class II highways that pass through towns and communities and have a mix of local traffic and through traffic.

The measure of effectiveness by which Level of Service is determined on Class I and II highways is average travel speed (ATS) and percent time spent following (PTSF), or the proportion of time that drivers on the highway are limited in their speed by a driver in front of them. Class III highways are measured by percent of free-flow speed (PFFS), which represents the ability of vehicles to travel at or near the posted speed limit. Silverado Trail was defined as a Class II roadway for the purposes of this analysis. A summary of the ATS, PTSF, and PFFS breakpoints is shown in Table 2.

LOS	Class I Highways		Class II Highways	Class III Highways
	ATS (mi/h)	PTSF (%)	PTSF (%)	PFFS (%)
A	>55	≤35	≤40	>91.7
B	>50-55	>35-50	>40-55	>83.3-91.7
C	>45-50	>50-65	>55-70	>75.0-83.3
D	>40-45	>65-80	>70-85	>66.7-75.0
E	≤40	>80	≤85	≤66.7

Notes: LOS = Level of Service; ATS = Average Travel Speed  
 PTSF = Percent Time Spent Following  
 PFFS = Percent of Free-Flow Speed

Reference: *Highway Capacity Manual*, Transportation Research Board, 2010

A point just south of the project driveway was chosen to represent segment volumes on Silverado Trail in the vicinity of the project site. Choosing a point just south of the project site captures more of the trips projected to be added to the roadway network due to Cumulative conditions and plus Project conditions because of the assumed trip distribution, resulting in a more conservative analysis.

## Traffic Operation Standards

Policy CIR-13 in the *Napa County General Plan* states, "The County seeks to provide a roadway system that maintains current roadway capacities in most locations and is both safe and efficient in terms of providing local access."

Policy CIR-16 of the *Napa County General Plan* provides guidance for roadways, indicating that, “The County shall seek to maintain an arterial Level of Service D or better on all county roadways, except where maintaining this desired level of service would require the installation of more travel lanes than shown on the Circulation Map. Silverado Trail is shown as a 2-lane Rural Collector on the Circulation Map (Figure CIR-1). A one-percent criteria for the threshold of significance is used for this analysis because it is well within the range of daily variation in traffic as well as the range of accuracy of travel demand forecast models and therefore not likely to be noticeable to drivers. While the traffic study relies on the County’s LOS D standard, if operation falls below this threshold, an increase which is less than one-percent of cumulative volumes is considered less-than-significant.

## Existing Conditions

The Existing Conditions scenario provides an evaluation of current operation based on existing traffic volumes during the weekday p.m. and weekend midday peak periods. This condition does not include project-generated traffic volumes. Volume data was collected April 8 to April 12, 2015.

The existing traffic volumes on Silverado Trail are summarized in Table 3.

Table 3 – Existing Traffic Volumes				
Study Segment	Weekday		Saturday	
	NB	SB	NB	SB
Silverado Trail near 4059	414	987	549	387

Notes: NB = Northbound; SB = Southbound

## Roadway Segment Levels of Service

Under existing conditions, the roadway operates acceptably at LOS C or D or better northbound during both peak periods and southbound during the weekend midday peak period, but unacceptably at LOS E during the weekday p.m. peak hour in the southbound direction. A summary of the roadway segment level of service calculations is shown in Table 4, and copies of the Level of Service calculations are provided in Appendix B.

Table 4 – Existing Peak Hour Roadway Segment Levels of Service									
Study Segment	Weekday PM Peak				Weekend Midday Peak				
	NB		SB		NB		SB		
	PTSF	LOS	PTSF	LOS	PTSF	LOS	PTSF	LOS	
Silverado Trail near 4059	59.9%	C	<b>89.7%</b>	<b>E</b>	78.4%	D	63.1%	C	

Notes: NB = Northbound; SB = Southbound; PTSF = Percent Time Spent Following  
 LOS = Level of Service; **Bold** text = deficient operation

## Cumulative Conditions

Cumulative operating conditions were determined with trips generated by other approved and pending projects that would add traffic to the segment of Silverado Trail within four miles of Sam Jasper Winery added to existing volumes. As directed by County staff, the following projects were included to evaluate Cumulative Conditions.

- **Krupp Winery** – 3150 Silverado Trail, approximately 0.6 miles south of the project site; new winery with an annual production of 50,000 gallons; six full-time employees and four part-time employees; maximum of 124 visitors per day; maximum of 125 guests at special events
- **Melka Winery** – 2900 Silverado Trail, approximately 1.2 miles south of the project site; new winery with an annual production of 10,000 gallons; one full-time employee and one part-time employee; maximum of 7 visitors per day; maximum of 100 guests at special events
- **Reynolds Winery** – 3720 Silverado Trail, approximately 0.4 miles south of the project site; use permit update to produce 20,000 additional gallons annually; 10 additional employees; maximum of 30 additional visitors per day; maximum of 125 guests at special events
- **Davis Estates Winery** – 4060 Silverado Trail, approximately 0.1 miles north of the project site; use permit update to produce an additional 80,000 gallons annually; 10 additional employees; maximum of 190 additional visitors per day; maximum of 200 guests at special events
- **Mountain Peak Winery** – 3265 Soda Canyon Road, approximately 6.5 miles from the project site; new winery with an annual production of 100,000 gallons; 29 full-time employees and eight part-time employees; maximum of 80 visitors per day; maximum of 125 guests at special events (although this winery is more than four miles from the project site, it will add trips to the study segment via the lone connection to Soda Canyon Road)
- **Stag’s Leap Winery** – 5766 Silverado Trail, approximately 3.6 miles north of the project site; use permit update to have an additional 25 employees; maximum of 250 guests at special events
- **Corona Winery** – 3165 Silverado Trail, approximately 0.4 miles south of the project site; new winery with an annual production of 100,000 gallons; 25 employees; maximum of 48 visitors per day; maximum of 125 guests at special events
- **Beau Vigne Winery** – 4057 Silverado Trail, 300 feet south of the project site; use permit update to increase production to 14,000 gallons; three full-time employees and one part-time employee; maximum of 15 visitors per day; maximum of 30 guests at special events

The traffic volumes on the study segment under cumulative conditions are summarized in Table 5. Some visitors to Sam Jasper Winery would be expected to visit multiple wineries during their time in Napa Valley, including those wineries included in the list of approved projects.

<b>Study Segment</b>	<b>Weekday</b>		<b>Saturday</b>	
	<b>NB</b>	<b>SB</b>	<b>NB</b>	<b>SB</b>
Silverado Trail near 4059	470	1,050	635	473

Notes: NB = Northbound; SB = Southbound

Under cumulative conditions, Silverado Trail is expected to operate acceptably at LOS C or D northbound during both peak periods and southbound during the weekend midday peak period, but would continue to operate unacceptably at LOS E during the weekday p.m. peak hour in the southbound direction. These results are summarized in Table 6, and cumulative volumes are shown in Figure 1.

**Table 6 – Cumulative Peak Hour Roadway Segment Levels of Service**

Study Segment	Weekday PM Peak				Weekend Midday Peak			
	NB		SB		NB		SB	
	PTSF	LOS	PTSF	LOS	PTSF	LOS	PTSF	LOS
Silverado Trail near 4059	64.7%	C	<b>90.7%</b>	<b>E</b>	81.3%	D	68.7%	C

Notes: NB = Northbound; SB = Southbound; PTSF = Percent Time Spent Following  
 LOS = Level of Service; **Bold** text = deficient operation

In keeping with Policies CIR-13 and CIR-16 of the *Napa County General Plan*, because it would require adding lanes to achieve a higher service level, the County would instead be expected to focus on improving access, especially at intersections of public roads, so no measures are recommended to achieve LOS D operation on Silverado Trail.

### Future Conditions

Segment volumes for the horizon year of 2030 were obtained from the County's gravity demand model. Because the projected 2030 volumes for Silverado Trail near the project driveway are approximately equal to existing volumes from April 2015 and less than cumulative volumes, a growth factor of 2.15 was calculated based on 2010 and 2030 model volumes and applied to existing volumes to arrive at future volumes.

Table 7 shows the projected future traffic volumes on Silverado Trail near the project site achieved through application of the growth factor.

**Table 7 – Future Traffic Volumes**

Study Segment	Weekday		Saturday	
	NB	SB	NB	SB
Silverado Trail near 4059	890	2,122	1,180	832

Notes: NB = Northbound; SB = Southbound

Under projected future volumes, the roadway study segment is expected to operate unacceptably at LOS E during the weekday p.m. peak in the southbound direction and during the weekend midday peak in the northbound direction and acceptably at LOS D during the weekday p.m. peak in the northbound direction and during the weekend midday peak in the southbound direction. As noted for Cumulative Conditions, capacity enhancements to achieve LOS D operation are not suggested. These results are summarized in Table 8.

**Table 8 – Future Peak Hour Roadway Segment Levels of Service**

Study Segment	Weekday PM Peak				Weekend Midday Peak			
	NB		SB		NB		SB	
	PTSF	LOS	PTSF	LOS	PTSF	LOS	PTSF	LOS
Silverado Trail near 4059	83.9%	D	<b>100%</b>	<b>E</b>	<b>91.1%</b>	<b>E</b>	82.2%	D

Notes: NB = Northbound; SB = Southbound; PTSF = Percent Time Spent Following  
 LOS = Level of Service; **Bold** text = deficient operation

## Project Description

The project consists of a 20,000 gallon winery with fewer than 10 employees, a maximum of 25 visitors per day, and 10 special events per year with 15 guests, 10 special events with 25 guests, and three special events with 50 guests. The proposed project site plan is shown in Figure 2.

## Trip Generation

The anticipated trip generation for a proposed project is typically estimated using standard rates published by the Institute of Transportation Engineers (ITE) in *Trip Generation Manual*, 9<sup>th</sup> Edition, 2012. However, the publication contains no such information for a winery. Therefore, the County of Napa's Winery Traffic Information/Trip Generation Sheet was used to determine the anticipated traffic generated with the current staff as well as that would be generated by both production that is already permitted at the site and with the proposed tasting room. Copies of the worksheets are enclosed for reference.

Since the County of Napa's Winery Traffic Information/Trip Generation Sheet does not include guidance on inbound versus outbound trips, it was assumed that two-thirds of trip ends at the winery would be outbound during the weekday p.m. peak hour since most of the trips would be associated with employees and customers leaving at closure of the winery. For the Saturday midday peak hour it was assumed that inbound and outbound trip ends would be evenly split. The trip generation estimates for the proposed project are shown in Table 9 and the Trip Generation Sheet is included in Appendix C.

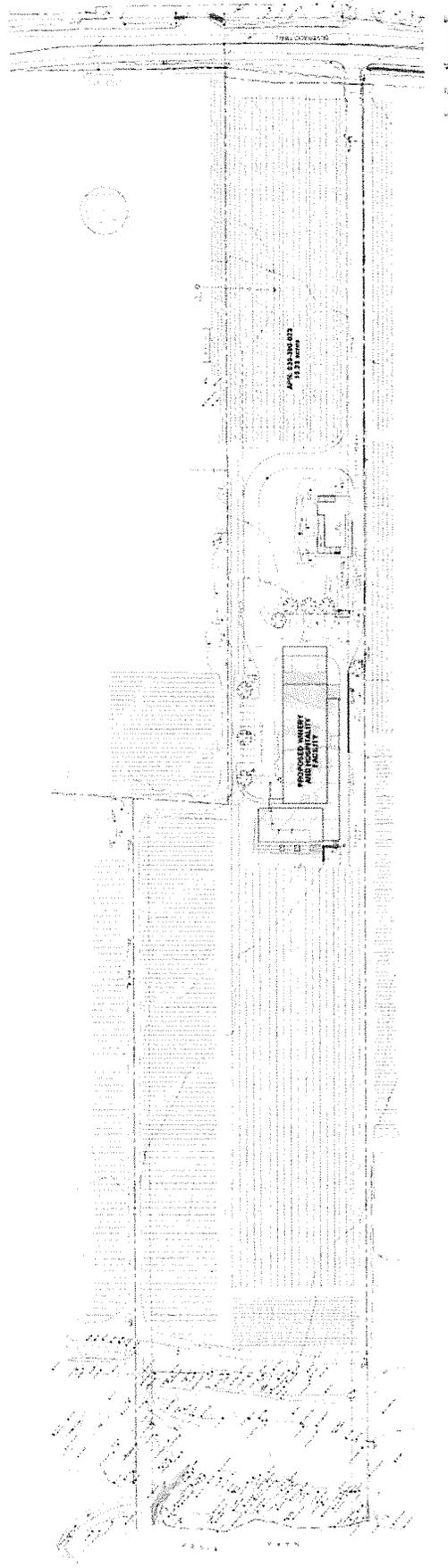
Table 9 – Trip Generation							
Trip Type	Daily	Weekday PM Peak		Saturday Midday Peak			
	Trips	Trips	In	Out	Trips	In	Out
Winery plus Tasting Room	50	19	6	13	27	13	14

## Trip Distribution

The pattern used to allocate new project trips to the street network was based on the directional split of the traffic volume during the p.m. peak period and weekend midday peak period. It was assumed 30 percent of projects trips would be to and from the north, while 70 percent of the project trips would be to and from the south.

## Special Events

Three different sized wine marketing and agricultural promotion events are proposed as part of the project, including ten 15-person events, ten 25-person events, and three 50-person events. It was assumed that a maximum-sized 50-person event would require a staff of four in addition to any winery staff that would also assist with the event. Using an occupancy of 2.8 persons per vehicle for guests and solo occupancy for staff, a maximum-sized 50-person event would be expected to generate 44 trip ends at the driveway, including 22 inbound trips and 22 outbound trips. These events are proposed to take place between 11:00 a.m. to 10:00 p.m., with staff arriving an hour or more prior to the start of the event and leaving an hour or more after its conclusion. Guests would generate 18 trips for arrival and 18 for departure, with these trips generally occurring over an hour's time. Distributed evenly to the north and south, the maximum-sized special event would be expected to generate nine trips an hour on each of the segments of Silverado Trail to the north and south of the driveway. Given that these events are infrequent, are not part of typical daily operation, and often occur outside the peak period for traffic, together with the minimal increase in traffic associated with them, special event traffic was not included in the daily trip generation and resulting intersection operation analysis.



Source: Hall & Bartley Architecture and Planning 3/15

091max.ai 9/15

Traffic Impact Study for Sam Jasper Winery  
**Figure 2 – Site Plan**



## Intersection Operation

### Existing plus Project Conditions

The existing and existing plus project traffic volumes on this segment are summarized in Table 10.

Existing Conditions				Existing plus Project			
PM Peak		Weekend Midday Peak		PM Peak		Weekend Midday Peak	
NB	SB	NB	SB	NB	SB	NB	SB
414	987	549	387	418	996	558	397

Notes: NB = Northbound; SB = Southbound

Upon the addition of project-related traffic to the Existing volumes, the study segment is expected to continue to operate acceptably at LOS C or D northbound during both peak periods and unacceptably at LOS E during the weekday p.m. peak hour in the southbound direction during the weekday p.m. peak period. These results are summarized in Table 11. Project traffic volumes are shown in Figure 1.

Existing Conditions								Existing plus Project							
PM Peak				Weekend Midday Peak				PM Peak				Weekend Midday Peak			
NB		SB		NB		SB		NB		SB		NB		SB	
PTSF	LOS	PTSF	LOS	PTSF	LOS	PTSF	LOS	PTSF	LOS	PTSF	LOS	PTSF	LOS	PTSF	LOS
59.9%	C	<b>89.7%</b>	<b>E</b>	78.4%	D	63.1%	C	60.2%	C	<b>89.5%</b>	<b>E</b>	77.9%	D	63.4%	C

Notes: NB = Northbound; SB = Southbound; PTSF = Percent Time Spent Following  
LOS = Level of Service; **Bold** text = deficient operation

It is noted that the percent time spent following actually decreases in the southbound direction during the weekday p.m. peak hour and the northbound direction during the weekend midday peak period with project trips added. This result is counter-intuitive, but occurs occasionally when the flow is such that the added vehicles will theoretically have delays below the average, thereby reducing it. In practical terms, this result essentially indicates that the change would be imperceptible and drivers would not notice the minor increase in traffic.

**Finding** – The study segment is expected to continue operating at the same levels of service upon the addition of project-generated traffic to existing volumes. Because the project-added trips result in a reduction in the percent time spent following, or improved operation, the impact is considered less-than significant.

### Cumulative plus Project Conditions

The cumulative and cumulative plus project traffic volumes on this segment are summarized in Table 12.

**Table 12 – Cumulative and Cumulative plus Project Traffic Volumes near 4059 Silverado Trail**

Cumulative Conditions				Cumulative plus Project			
PM Peak		Weekend Midday Peak		PM Peak		Weekend Midday Peak	
NB	SB	NB	SB	NB	SB	NB	SB
470	1,050	635	473	475	1,059	644	483

Notes: NB = Northbound; SB = Southbound

Upon the addition of project-related traffic to the Cumulative volumes, the study segment is expected to continue to operate unacceptably at LOS E in the southbound direction during the weekday p.m. peak hour. These results are summarized in Table 13. Cumulative traffic volumes are shown in Figure 1.

**Table 13 – Cumulative and Cumulative plus Project Peak Hour Intersection LOS on Silverado Trail**

Cumulative Conditions								Cumulative plus Project							
PM Peak				Weekend Midday Peak				PM Peak				Weekend Midday Peak			
NB		SB		NB		SB		NB		SB		NB		SB	
PTSF	LOS	PTSF	LOS	PTSF	LOS	PTSF	LOS	PTSF	LOS	PTSF	LOS	PTSF	LOS	PTSF	LOS
64.7%	C	<b>90.7%</b>	<b>E</b>	81.3%	D	68.7%	C	65.0%	C	<b>90.3%</b>	<b>E</b>	81.9%	D	70.0%	C

Notes: LOS = Level of Service; PTSF = Percent Time Spent Following; **Bold** text = deficient operation

**Finding** – The study segment is expected to continue operating at the same levels of service upon the addition of project-generated traffic. Because the project-added trips translate to less than 1 percent of the total p.m. peak hour trips on Silverado Trail, the project’s impact is considered less-than-significant.

### Future plus Project Conditions

The future and future plus project traffic volumes on this segment are summarized in Table 14.

**Table 14 – Future and Future plus Project Traffic Volumes near 4059 Silverado Trail**

Future Conditions				Future plus Project			
PM Peak		Weekend Midday Peak		PM Peak		Weekend Midday Peak	
NB	SB	NB	SB	NB	SB	NB	SB
890	2,122	1,180	832	894	2,131	1,189	842

Notes: NB = Northbound; SB = Southbound

Upon the addition of project-related traffic to the Future volumes, the study segment is expected to continue operating unacceptably at LOS E during the weekday p.m. peak in the southbound direction and during the weekend midday peak in the northbound direction. These results are summarized in Table 15.

**Table 15 – Future and Future plus Project Peak Hour Intersection Levels of Service on Silverado Trail**

Future Conditions								Future plus Project							
PM Peak				Weekend Midday Peak				PM Peak				Weekend Midday Peak			
NB		SB		NB		SB		NB		SB		NB		SB	
PTSF	LOS	PTSF	LOS	PTSF	LOS	PTSF	LOS	PTSF	LOS	PTSF	LOS	PTSF	LOS	PTSF	LOS
83.9%	D	<b>100%</b>	<b>E</b>	<b>91.1%</b>	<b>E</b>	82.2%	D	84.0%	D	<b>100%</b>	<b>E</b>	<b>91.1%</b>	<b>E</b>	82.4%	D

Notes: LOS = Level of Service; PTSF = Percent Time Spent Following; **Bold** text = deficient operation

**Finding** – The study segment is expected to continue operating at the same levels of service upon the addition of project-generated traffic, with no change to the percent time spent following. Because the project-added trips do not change the measure of effectiveness and contribute less than 1 percent of the total projected future trips on Silverado Trail, the impact is considered less-than-significant.

### Transportation Demand Management Program

Although the project is expected to have less-than-significant traffic impacts, to reduce the number of peak hour trips added to the roadway network and minimize impacts, the proposed project should consider promoting the use of carpooling as options for employees arriving at and leaving the project site.

# Access and Circulation

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## Site Access

The project would be accessed via an existing driveway on Silverado Trail. The nearest driveway to the south is 280 feet away and the nearest driveway to the north is 250 feet away.

## Sight Distance

At driveways a substantially clear line of sight should be maintained between the driver of a vehicle waiting on the driveway and the driver of an approaching vehicle. Adequate time must be provided for the waiting vehicle to either cross, turn left, or turn right, without requiring the through traffic to radically alter their speed. Sight distance should be measured from a 3.5-foot height at the location of the driver on the driveway to a 4.25-foot object height in the center of the approaching lane of the major road. Set-back for the driver on the driveway shall be a minimum of 15 feet, measured from the edge of the traveled way.

Sight distance along Silverado Trail at the project driveway was evaluated based on stopping sight distance criteria contained in the *Highway Design Manual* published by Caltrans. The approach travel speed is used as the basis for determining the recommended sight distance. Additionally, the stopping sight distance needed for a following driver to stop if there is a vehicle waiting to turn into a side street or driveway is evaluated based on stopping sight distance criterion and the approach speed on the major street.

Sight distance at the project driveway was field measured. Although sight distance requirements are not technically applicable to urban driveways, the stopping sight distance criterion for private street intersections was applied for evaluation purposes. Based on a design speed of 55 mph, the minimum stopping sight distance needed is 500 feet. Sight distance to the north of the project driveway is 500 feet and sight distance to the south of the project driveway is to Soda Canyon Road, which is more than 900 feet away. Stopping sight distance exceeds 500 feet for a southbound driver following a vehicle that might slow or stop to turn left into the driveway.

**Finding** – Stopping sight distance at the project driveway is adequate to meet the applied criteria from the *Highway Design Manual* for both entering and exiting movements.

## Access Analysis

### Left-Turn Lane Warrants

The need for a left-turn lane on Silverado Trail at the project driveway was evaluated based on criteria contained in the *Napa County Road and Street Standards*, 2011. Based on the volumes obtained on Silverado Trail near the project driveway in April 2015, Silverado Trail has an average daily traffic (ADT) volume of 13,100 vehicles near the project driveway.

Using the County's criteria, for an average daily traffic volume of 13,120 vehicles on Silverado Trail, a left-turn lane is warranted if a project driveway has an ADT of 11 or more vehicles. The ADT at the project driveway is expected to be 50 trips daily. Based on these traffic levels, a left-turn lane is warranted at the project driveway. The left-turn lane warrant graph is provided in Appendix D.

**Finding** – A left turn lane is warranted at the project driveway and should be provided.

# Conclusions and Recommendations

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## Conclusions

- The proposed project would be expected to generate an average of 50 daily trips, including 19 trips during the weekday p.m. peak hour and 27 trips during the weekend midday peak hour.
- A total of 23 special events are proposed, with the largest special event having 50 attendees and four employees; three such events are proposed annually. The remaining 20 events would have 15 to 25 attendees.
- Silverado Trail currently operates acceptably at LOS D or better northbound during both peak periods and southbound during the weekend midday peak period and unacceptably at LOS E in the southbound direction during the weekday p.m. peak hour.
- Acceptably LOS D or better operation on Silverado Trail is projected under Cumulative Conditions for the northbound direction during both peak periods and southbound during the weekend midday peak period. Unacceptable LOS E operation is projected in the southbound direction during the weekday p.m. peak hour.
- Under Future conditions, Silverado Trail is expected to operate unacceptably at LOS E during the weekday p.m. peak in the southbound direction and in the northbound direction during the weekend midday peak and acceptably at LOS D during the weekday p.m. peak in the northbound direction and in the southbound direction during the weekend midday peak.
- Because the project-added trips result in no change to the measures of effectiveness under most scenarios and contribute less than 1 percent of the total trips on Silverado Trail, the impact is considered less-than-significant under Existing, Cumulative, and Future plus Project conditions.
- Sight distance at the project driveway is adequate to meet *Highway Design Manual* requirements in both directions as well as for following drivers.
- A left-turn lane is warranted on Silverado Trail at the project driveway.
- A Transportation Demand Management plan for the project is not currently proposed.

## Recommendations

- A left-turn lane should be installed on Silverado Trail at the project driveway as part of the proposed project.
- The project applicant should consider developing a Transportation Demand Management plan for the project, including promoting carpooling for employees.

# Study Participants and References

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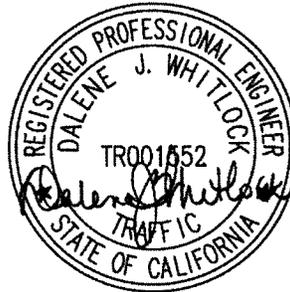
## Study Participants

Principal in Charge	Dalene J. Whitlock, PE, PTOE
Assistant Engineer	Lauren Davini, EIT
Technician/Graphics	Deborah J. Mizell
Editing/Formatting	Angela McCoy

## References

*2012 Collision Data on California State Highways*, California Department of Transportation, 2012  
*Highway Capacity Manual*, Transportation Research Board, 2010  
*Highway Design Manual*, 6<sup>th</sup> Edition, California Department of Transportation, 2012  
*Napa County General Plan*, County of Napa, 2013  
*Napa County Road and Street Standards*, County of Napa, 2011  
*Statewide Integrated Traffic Records System (SWITRS)*, California Highway Patrol, 2008-2013  
*Trip Generation Manual*, 9<sup>th</sup> Edition, Institute of Transportation Engineers, 2012

NAX091



# Appendix A

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## Collision Rate Calculations



**SEGMENT COLLISION RATE CALCULATIONS**

**Sam Jasper Winery**

**Location:** 4059 Silverado Trail

**Date of Count:** Friday, April 10, 2015  
**ADT:** 14,000

**Number of Collisions:** 16  
**Number of Injuries:** 5  
**Number of Fatalities:** 0  
**Start Date:** October 1, 2008  
**End Date:** September 30, 2013  
**Number of Years:** 5

**Highway Type:** Conventional 2 lanes or less  
**Area:** Urban  
**Design Speed:** >45

**Segment Length:** 1.0 miles  
**Direction:** North/South

$$\frac{\text{Number of Collisions} \times 1 \text{ Million}}{\text{ADT} \times 365 \text{ Days per Year} \times \text{Segment Length} \times \text{Number of Years}}$$

$$\frac{16 \times 1,000,000}{14,000 \times 365 \times 1 \times 5}$$

	<u>Collision Rate</u>	<u>Fatality Rate</u>	<u>Injury Rate</u>
<b>Study Segment</b>	0.63 c/mvm	0.0%	31.3%
<b>Statewide Average*</b>	1.29 c/mvm	1.0%	41.2%

ADT = average daily traffic volume  
 c/mvm = collisions per million vehicle miles  
 \* 2012 Collision Data on California State Highways, Caltrans

# Appendix B

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## Intersection Level of Service Calculations

Phone:  
E-Mail:

Directional Two-Lane Highway Segment Analysis  
 Analysis Time Period: 9/8/15  
 Highway: Silverado Trail  
 From/To: 0.5mi N/o & S/o 4059 SilverTrail  
 County of Napa  
 Analysis Year: 2015  
 Description: NB Existing

Analyst: Lauren Davini  
 Agency/Co.: Napa County  
 Date Performed: 9/8/15  
 Analysis Time Period: Weekday PM Peak  
 Highway: Silverado Trail  
 From/To: 0.5mi N/o & S/o 4059 SilverTrail  
 County of Napa  
 Analysis Year: 2015  
 Description: NB Existing

Direction  
 PCE for trucks, ET: 1.0  
 PCE for RVs, ER: 1.0  
 Heavy-vehicle adjustment factor, fHV: 1.000  
 Grade adjustment factor, (note-1) fg: 1.00  
 Directional flow rate, (note-2) vi: 436 pc/h  
 Base percent time-spent-following, (note-4) BFTSfd: 53.5 %  
 Adjustment for no-passing zones, fnp: 21.7 %  
 Percent time-spent-following, FTsfd: 59.9 %

Input Data

Highway Class	Class 2	Peak hour factor, PHF	0.95
Shoulder width	7.0 ft	% Trucks and buses	5 %
Lane width	12.0 ft	Truck crawling	0.0 %
Segment length	1.0 mi	Truck crawl speed	0.0 mi/hr
Terrain type	Level	Recreational vehicles	2 %
Grade:	Length	No-passing zones	100 %
Up/down	-	Access point density	15 /mi

Analysis direction volume, Vd: 414 veh/h  
 Opposing direction volume, Vo: 987 veh/h

Direction	Analysis(d)	Opposing (o)
PCE for trucks, ET	2.0*	2.0*
PCE for RVs, ER	1.0	1.0
Heavy-vehicle adj. factor, (note-5) fHV	0.952	0.952
Grade adj. factor, (note-1) fg	1.00	1.00
Directional flow rate, (note-2) vi	458 pc/h	1091 pc/h

Free-Flow Speed from Field Measurement:  
 Field measured speed, (note-3) S FM: - mi/h  
 Observed total demand, (note-3) V: - veh/h  
 Estimated Free-Flow Speed: 55.0 mi/h  
 Base free-flow speed, (note-3) BFFS: 0.0 mi/h  
 Adj. for lane and shoulder width, (note-3) fLS: 3.8 mi/h  
 Adj. for access point density, (note-3) fA: 51.3 mi/h

Free-flow speed, FTsD: 1.1 mi/h  
 Adjustment for no-passing zones, fnp: 38.2 mi/h  
 Average travel speed, ATSD: 74.5 %  
 Percent Free Flow Speed, PFFS

Percent Time-Spent-Following

Direction	Analysis(d)	Opposing (o)
PCE for trucks, ET	1.0	1.0
PCE for RVs, ER	1.0	1.0
Heavy-vehicle adjustment factor, fHV	1.000	1.000
Grade adjustment factor, (note-1) fg	1.00	1.00
Directional flow rate, (note-2) vi	436 pc/h	1039 pc/h
Base percent time-spent-following, (note-4) BFTSfd	53.5 %	
Adjustment for no-passing zones, fnp	21.7 %	
Percent time-spent-following, FTsfd	59.9 %	

Level of Service and Other Performance Measures

Level of service, LOS	C
Volume to capacity ratio, v/c	0.26
Peak 15-min vehicle-miles of travel, VMT15	109 veh-mi
Peak 15-min vehicle-miles of travel, VMT60	414 veh-mi
Peak 15-min total travel time, TT15	2.9 veh-h
Capacity from ATIS, CdATS	1700 veh/h
Capacity from PTSF, CdPTSF	1700 veh/h
Directional Capacity	1700 veh/h

Passing Lane Analysis

Total length of analysis segment, Lt	1.0 mi
Length of two-lane highway upstream of the passing lane, Lu	- mi
Length of passing lane including tapers, Lpl	- mi
Average travel speed, ATSD (from above)	38.2 mi/h
Percent time-spent-following, FTsfd (from above)	59.9 %
Level of service, LOSd (from above)	C

Average Travel Speed with Passing Lane

Downstream length of two-lane highway within effective length of passing lane for average travel speed, Lde	- mi
Length of two-lane highway downstream of effective length of the passing lane for average travel speed, Ld	- mi
Adj. factor for the effect of passing lane on average speed, fpl	-
Average travel speed including passing lane, ATSp1	-
Percent free flow speed including passing lane, PFFSpl	0.0 %

Percent Time-Spent-Following with Passing Lane

Downstream length of two-lane highway within effective length of passing lane for percent time-spent-following, Lde	- mi
Length of two-lane highway downstream of effective length of the passing lane for percent time-spent-following, Ld	- mi
Adj. factor for the effect of passing lane on percent time-spent-following, fpl	-
Percent time-spent-following including passing lane, PFTSfpl	- %

Level of Service and Other Performance Measures with Passing Lane

Level of service including passing lane, LOSpl	A
Peak 15-min total travel time, TT15	- veh-h

Bicycle Level of Service

Posted speed limit, Sp 55  
 Percent of segment with occupied on-highway parking 0  
 Pavement rating, P 3  
 Flow rate in outside lane, vol 435.8  
 Effective width of outside lane, We 26.00  
 Effective speed factor, St 4.79  
 Bicycle LOS Score, BLOS 2.75  
 Bicycle LOS C

Notes:

1. Note that the adjustment factor for level terrain is 1.00, as level terrain is one of the base conditions. For the purpose of grade adjustment, specific downgrade segments are treated as level terrain.
2. If vi (vd or vo) >= 1,700 pc/h, terminate analysis-the LOS is F.
3. For the analysis direction only and for v>200 veh/h.
4. For the analysis direction only.
5. Use alternative Exhibit 15-14 if some trucks operate at crawl speeds on a specific downgrade.

\* These items have been entered or edited to override calculated value

Phone:  
 E-Mail:  
 Fax:

Directional Two-Lane Highway Segment Analysis

Analyst Lauren Davini  
 Agency/Co. Napa County  
 Date Performed 9/8/15  
 Analysis Time Period Weekday PM Peak  
 Highway Silverado Trail  
 From/To 0.5mi N/O & S/O 4059 SilvTrail  
 Jurisdiction County of Napa  
 Analysis Year 2015  
 Description SB Existing

Input Data

Highway class Class 2  
 Shoulder width 7.0 ft  
 Lane width 12.0 ft  
 Segment length 1.0 mi  
 Terrain type Level  
 Grade: Length - mi  
 Up/down - %  
 Peak hour factor, PHF 0.95  
 % Trucks and buses 5 %  
 % Trucks crawling 0.0 %  
 Truck crawl speed 0.0 mi/hr  
 % Recreational vehicles 2 %  
 % No-passing zones 100 %  
 Access point density 12 /mi

Analysis direction volume, Vd 987 veh/h  
 Opposing direction volume, Vo 414 veh/h

Average Travel Speed

Direction Analysis(d) Opposing (o)  
 PCE for trucks, ET 2.0\*  
 PCE for RVs, ER 1.0  
 Heavy-vehicle adj. factor, (note-5) fHV 0.952  
 Grade adj. factor, (note-1) fg 1.00  
 Directional flow rate, (note-2) vi 1091 pc/h  
 Directional flow rate, (note-3) fa 458 pc/h

Free-Flow Speed from Field Measurement:

Field measured speed, (note-3) S FM - mi/h  
 Observed total demand, (note-3) V - veh/h  
 Estimated Free-Flow Speed: 55.0 mi/h  
 Base free-flow speed, (note-3) BFFS 0.0 mi/h  
 Adj. for lane and shoulder width, (note-3) fLS 3.0 mi/h  
 Adj. for access point density, (note-3) fA 52.0 mi/h  
 Free-flow speed, FFFS 2.5 mi/h  
 Adjustment for no-passing zones, fnp 37.5 mi/h  
 Average travel speed, AWTsd 72.1 %  
 Percent Free Flow Speed, PFFS



Direction  
 PCE for trucks, ET 1.0  
 PCE for RVs, ER 1.0  
 Heavy-vehicle adjustment factor, fHV 1.000  
 Grade adjustment factor, (note-1) fg 1.00  
 Directional flow rate, (note-2) vi 610 pc/h  
 Base percent time-spent-following, (note-4) BPTSfd 57.2 %  
 Adjustment for no-passing zones, fnp 36.2 %  
 Percent time-spent-following, PTSfd 78.4 %

Analysis(d)  
 1.0  
 1.000  
 1.00  
 610 pc/h  
 BPTSfd 57.2 %  
 36.2 %  
 78.4 %

Opposing (o)  
 1.0  
 1.0  
 1.000  
 430 pc/h

Level of Service and Other Performance Measures

Level of service, LOS D  
 Volume to capacity ratio, v/c 0.36  
 Peak 15-min vehicle-miles of travel, VMT15 153 veh-mi  
 Peak-hour vehicle-miles of travel, VMT60 549 veh-mi  
 Peak 15-min total travel time, TT15 3.8 veh-h  
 Capacity from ATS, CdATS 0 veh/h  
 Capacity from PTSF, CdPTSF 1700 veh/h  
 Directional Capacity 1700 veh/h

Passing Lane Analysis

Total length of analysis segment, Lt 1.0 mi  
 Length of two-lane highway upstream of the passing lane, Lu - mi  
 Length of passing lane including tapers, Lpl - mi  
 Average travel speed, ATSD (from above) 40.3 mi/h  
 Percent time-spent-following, PTSfd (from above) 78.4 %  
 Level of service, LOSd (from above) D

Average Travel Speed with Passing Lane

Downstream length of two-lane highway within effective length of passing lane for average travel speed, Lde - mi  
 Length of two-lane highway downstream of effective length of the passing lane for average travel speed, Ld - mi  
 Adj. factor for the effect of passing lane on average speed, fpl -  
 Average travel speed including passing lane, ATSpl -  
 Percent free flow speed including passing lane, PFFSpl 0.0 %

Percent Time-Spent-Following with Passing Lane

Downstream length of two-lane highway within effective length of passing lane for percent time-spent-following, Lde - mi  
 Length of two-lane highway downstream of effective length of the passing lane for percent time-spent-following, Ld - mi  
 Adj. factor for the effect of passing lane on percent time-spent-following, fpl -  
 Percent time-spent-following including passing lane, PTSFpl - %

Level of Service and Other Performance Measures with Passing Lane

Level of service including passing lane, LOSpl A  
 Peak 15-min total travel time, TT15 - veh-h

Bicycle Level of Service

Phone:  
 E-Mail:  
 Fax:

Directional Two-Lane Highway Segment Analysis

Analyst Lauren Davini  
 Agency/Co. Napa County  
 Date Performed 9/8/15  
 Analysis Time Period Weekend Midday Peak  
 Highway Silverado Trail  
 From/To 0.5mi N/o & S/o 4059 Silvertrail  
 Jurisdiction County of Napa  
 Analysis Year 2015  
 Description NB Existing

Input Data

Highway class Class 2  
 Shoulder width 7.0 ft  
 Lane width 12.0 ft  
 Segment length 1.0 mi  
 Terrain type Level  
 Grade: Length -  
 Up/down -  
 Peak hour factor, PHF 0.90  
 % Trucks and buses 5 %  
 % Trucks crawling 0.0 %  
 Truck crawl speed 0.0 mi/hr  
 % Recreational vehicles 2 %  
 % No-passing zones 100 %  
 Access point density 15 /mi

Analysis direction volume, Vd 549 veh/h  
 Opposing direction volume, Vo 387 veh/h

Average Travel Speed

Direction Analysis(d) Opposing (o)  
 PCE for trucks, ET 2.0  
 PCE for RVs, ER 1.0  
 Heavy-vehicle adj. factor, (note-5) fHV 0.952  
 Grade adj. factor, (note-1) fg 1.00  
 Directional flow rate, (note-2) vi 641 pc/h  
 452 pc/h

Free-Flow Speed from Field Measurement:

Field measured speed, (note-3) S FM - mi/h  
 Observed total demand, (note-3) V - veh/h  
 Estimated Free-Flow Speed: 55.0 mi/h  
 Base free-flow speed, (note-3) BFFS 0.0 mi/h  
 Adj. for lane and shoulder width, (note-3) fLS 3.8 mi/h  
 Adj. for access point density, (note-3) fA 51.3 mi/h

Free-flow speed, FFSd 2.5 mi/h  
 Adjustment for no-passing zones, fnp 40.3 mi/h  
 Average travel speed, ATSD 78.6 %  
 Percent Free Flow Speed, PFFS

Posted speed limit, Sp 55  
 Percent of segment with occupied on-highway parking 0  
 Pavement rating, P 3  
 Flow rate in outside lane, VOL 610.0  
 Effective width of outside lane, We 26.00  
 Effective speed factor, St 4.79  
 Bicycle LOS Score, BLOS 2.92  
 Bicycle LOS C

Notes:

1. Note that the adjustment factor for level terrain is 1.00, as level terrain is one of the base conditions. For the purpose of grade adjustment, specific downgrade segments are treated as level terrain.
2. If vi (vd or vo) >= 1,700 pc/h, terminate analysis-the LOS is F.
3. For the analysis direction only and for v>200 veh/h.
4. For the analysis direction only.
5. Use alternative Exhibit 15-14 if some trucks operate at crawl speeds on a specific downgrade.

\* These items have been entered or edited to override calculated value

Phone: E-Mail:  
 Fax:

Directional Two-Lane Highway Segment Analysis

Analyst Lauren Davini  
 Agency/Co. Napa County  
 Date Performed 9/8/15  
 Analysis Time Period Weekend Midday Peak  
 Highway Silverado Trail  
 From/To 0.5mi N/O & S/O 4059 SilvTrail  
 Jurisdiction County of Napa  
 Analysis Year 2015  
 Description SB Existing

Input Data

Highway class Class 2  
 Shoulder width 7.0 ft  
 Lane width 12.0 ft  
 Segment length 1.0 mi  
 Terrain type Level  
 Grade: Length -  
 Up/down -

Peak hour factor, PHF 0.90  
 % Trucks and buses 5 %  
 % Trucks crawling 0.0 %  
 Truck crawl speed 0.0 mi/hr  
 % Recreational vehicles 2 %  
 % No-passing zones 100 %  
 Access point density 12 /mi

Analysis direction volume, Vd 387 veh/h  
 Opposing direction volume, Vo 549 veh/h

Average Travel Speed

Direction	Analysis(d)	Opposing (o)
PCE for trucks, ET	2.0*	2.0*
PCE for RVs, ER	1.0	1.0
Heavy-vehicle adj. factor, (note-5) fHV	0.952	0.952
Grade adj. factor, (note-1) fg	1.00	1.00
Directional flow rate, (note-2) vi	452 pc/h	641 pc/h

Free-Flow Speed from Field Measurement:

Field measured speed, (note-3) S FM - mi/h  
 Observed total demand, (note-3) V - veh/h  
 Estimated Free-Flow Speed: -  
 Base free-flow speed, (note-3) BFFS 55.0 mi/h  
 Adj. for lane and shoulder width, (note-3) fLS 0.0 mi/h  
 Adj. for access point density, (note-3) fA 3.0 mi/h  
 Free-flow speed, FFSd 52.0 mi/h

Adjustment for no-passing zones, fnp 1.8 mi/h  
 Average travel speed, ATSc 41.7 mi/h  
 Percent Free Flow Speed, PFFS 80.3 %

Posted speed limit, Sp 55  
 Percent of segment with occupied on-highway parking 0

Pavement rating, P 3  
 Flow rate in outside lane, VOL 430.0  
 Effective width of outside lane, We 26.00  
 Effective speed factor, St 4.79  
 Bicycle LOS Score, BLOS 2.75  
 Bicycle LOS C

- Notes:
1. Note that the adjustment factor for level terrain is 1.00, as level terrain is one of the base conditions. For the purpose of grade adjustment, specific downgrade segments are treated as level terrain.
  2. If  $v_i$  ( $v_d$  or  $v_o$ )  $>=$  1,700 pc/h, terminate analysis-the LOS is F.
  3. For the analysis direction only and for  $v > 200$  veh/h.
  4. For the analysis direction only.
  5. Use alternative Exhibit 15-14 if some trucks operate at crawl speeds on a specific downgrade.

\* These items have been entered or edited to override calculated value

Percent Time-Spent-Following	
Direction	Analysis(d)
PCE for trucks, ET	1.0
PCE for RVs, ER	1.0
Heavy-vehicle adjustment factor, FHV	1.000
Grade adjustment factor, (note-1) fg	1.00
Directional flow rate, (note-2) vl	430 pc/h
Base percent time-spent-following, (note-4) BPTSFd	48.1 %
Adjustment for no-passing zones, fnp	36.2
Percent time-spent-following, PTFSD	63.1 %

Level of Service and Other Performance Measures	
Level of service, LOS	C
Volume to capacity ratio, v/c	0.25
Peak 15-min vehicle-miles of travel, VMT15	108 veh-mi
Peak-hour vehicle-miles of travel, VMT60	387 veh-mi
Peak 15-min total travel time, TT15	2.6 veh-h
Capacity from ATS, CdATS	1692 veh/h
Capacity from PTF, CdPTF	1700 veh/h
Directional Capacity	1700 veh/h

Passing Lane Analysis	
Total length of analysis segment, Lt	1.0 mi
Length of two-lane highway upstream of the passing lane, Lu	- mi
Length of passing lane including tapers, Lpl	- mi
Average travel speed, ATSD (from above)	41.7 mi/h
Percent time-spent-following, PTFSD (from above)	63.1 %
Level of service, LOSd (from above)	C

Average Travel Speed with Passing Lane	
Downstream length of two-lane highway within effective length of passing lane for average travel speed, Lde	- mi
Length of two-lane highway downstream of effective length of the passing lane for average travel speed, Ld	- mi
Adj. factor for the effect of passing lane on average speed, fpl	-
Average travel speed including passing lane, ATSpI	-
Percent free flow speed including passing lane, PFFSpI	0.0 %

Percent Time-Spent-Following with Passing Lane	
Downstream length of two-lane highway within effective length of passing lane for percent time-spent-following, Lde	- mi
Length of two-lane highway downstream of effective length of the passing lane for percent time-spent-following, Ld	- mi
Adj. factor for the effect of passing lane on percent time-spent-following, fpl	-
Percent time-spent-following including passing lane, PTFSpI	- %

Level of Service and Other Performance Measures with Passing Lane	
Level of service including passing lane, LOSpl	A
Peak 15-min total travel time, TT15	- veh-h

Bicycle Level of Service

Direction PCE for trucks, ET Analysis(d) Opposing (o)  
 PCE for RVs, ER 1.0 1.0  
 Heavy-vehicle adjustment factor, fHV 1.000 1.000  
 Grade adjustment factor, (note-1) fg 1.00 1.00  
 Directional flow rate, (note-2) vi 40 pc/h 1048 pc/h  
 Base percent time-spent-following, (note-4) BPTSfd 53.8 %  
 Adjustment for no-passing zones, fnp 21.5 %  
 Percent time-spent-following, PTFsfd 60.2 %

Level of Service and Other Performance Measures

Level of service, LOS C  
 Volume to capacity ratio, v/c 0.26  
 Peak 15-min vehicle-miles of travel, VMT15 110 veh-mi  
 Peak 15-min total travel time, TTI15 418 veh-h  
 Capacity from ATS, CdATS 2.9 veh-h  
 Capacity from PTSF, CdPTSF 1700 veh/h  
 Directional Capacity 1700 veh/h

Passing Lane Analysis

Total length of analysis segment, Lt 1.0 mi  
 Length of two-lane highway upstream of the passing lane, Lu - mi  
 Length of passing lane including tapers, Lpl - mi  
 Average travel speed, ATSD (from above) 38.1 mi/h  
 Percent time-spent-following, PTFsd (from above) 60.2 %  
 Level of service, LOSd (from above) C

Average Travel Speed with Passing Lane

Downstream length of two-lane highway within effective length of passing lane for average travel speed, Lde - mi  
 Length of two-lane highway downstream of effective length of the passing lane for average travel speed, Ld - mi  
 Adj. factor for the effect of passing lane on average speed, fpl -  
 Average travel speed including passing lane, ATSpI -  
 Percent free flow speed including passing lane, PFRSpI 0.0 %

Percent Time-Spent-Following with Passing Lane

Downstream length of two-lane highway within effective length of passing lane for percent time-spent-following, Lde - mi  
 Length of two-lane highway downstream of effective length of the passing lane for percent time-spent-following, Ld - mi  
 Adj. factor for the effect of passing lane on percent time-spent-following, fpl -  
 Percent time-spent-following including passing lane, PTFSpI - %

Level of Service and Other Performance Measures with Passing Lane

Level of service including passing lane, LOSpl A  
 Peak 15-min total travel time, TTI15 - veh-h  
 Bicycle Level of Service

Fax:

Phone:  
 E-Mail:

Directional Two-Lane Highway Segment Analysis

Analyst Lauren Davini  
 Agency/Co. Napa County  
 Date Performed 9/8/15  
 Analysis Time Period Weekday PM Peak  
 Highway Silverado Trail  
 From/To 0.5mi N/O & S/O 4059 SilvTrail  
 Jurisdiction County of Napa  
 Analysis Year 2015  
 Description NB Existing + Project

Input Data

Highway Class Class 2 Peak hour factor, PHF 0.95  
 Shoulder width 7.0 ft % Trucks and buses 5 %  
 Lane width 12.0 ft % Trucks crawling 0.0 %  
 Segment length 1.0 mi Truck crawl speed 0.0 mi/hr  
 Terrain type Level % Recreational vehicles 2 %  
 Grade: Length mi % No-passing zones 100 %  
 Up/down % Access point density 15 /mi

Analysis direction volume, Vd 418 veh/h  
 Opposing direction volume, Vo 996 veh/h

Average Travel Speed

Direction PCE for trucks, ET Analysis(d) Opposing (o)  
 PCE for RVs, ER 2.0\* 1.0  
 Heavy-vehicle adj. factor, (note-5) fHV 0.952 0.952  
 Grade adj. factor, (note-1) fg 1.00 1.00  
 Directional flow rate, (note-2) vi 462 pc/h 1101 pc/h

Free-Flow Speed from Field Measurement:

Field measured speed, (note-3) S FM - mi/h  
 Observed total demand, (note-3) V - veh/h  
 Estimated Free-Flow Speed: 55.0 mi/h  
 Base free-flow speed, (note-3) BFFS 0.0 mi/h  
 Adj. for lane and shoulder width, (note-3) fLS 3.8 mi/h  
 Adj. for access point density, (note-3) fA 51.3 mi/h

Free-flow speed, FFSd

Adjustment for no-passing zones, fnp 1.0 mi/h  
 Average travel speed, ATSD 38.1 mi/h  
 Percent Free Flow Speed, PFFS 74.3 %

Posted speed limit, Sp 55  
 Percent of segment with occupied on-highway parking 0  
 Pavement rating, P 3  
 Flow rate in outside lane, vOL 440.0  
 Effective width of outside lane, We 26.00  
 Effective speed factor, St 4.79  
 Bicycle LOS Score, BLOS 2.76  
 Bicycle LOS C

Phone:  
 E-Mail:  
 Fax:

Directional Two-Lane Highway Segment Analysis

Analyst Lauren Davini  
 Agency/Co. Napa County  
 Date Performed 9/8/15  
 Analysis Time Period Weekday PM Peak  
 Highway Silverado Trail  
 From/To 0.5mi N/o & S/o 4059 SilvTrail  
 Jurisdiction County of Napa  
 Analysis Year 2015  
 Description SB Existing + Project

- Notes:
- Note that the adjustment factor for level terrain is 1.00, as level terrain is one of the base conditions. For the purpose of grade adjustment, specific downgrade segments are treated as level terrain.
  - If vi (vd or vo) >= 1,700 pc/h, terminate analysis-the LOS is F.
  - For the analysis direction only and for v>200 veh/h.
  - For the analysis direction only.
  - Use alternative exhibit 15-14 if some trucks operate at crawl speeds on a specific downgrade.

\* These items have been entered or edited to override calculated value

		Input Data	
Highway class	Class 2	Peak hour factor, PHF	0.95
Shoulder width	7.0 ft	% Trucks and buses	5 %
Lane width	12.0 ft	% Trucks crawling	0.0 %
Segment length	1.0 mi	Truck crawl speed	0.0 mi/hr
Terrain type	Level	% Recreational vehicles	2 %
Grade:	Up/down	% No-passing zones	100 %
		Access point density	12 /mi

Analysis direction volume, Vd 996 veh/h  
 Opposing direction volume, Vo 418 veh/h

		Average Travel Speed	
Direction	Analysis(d)	Analysis(d)	Opposing (o)
PCE for trucks, ET	2.0*	2.0*	2.0*
PCE for RVs, ER	1.0	1.0	1.0
Heavy-vehicle adj. factor, (note-5) fHV	0.952	0.952	0.952
Grade adj factor, (note-1) fg	1.00	1.00	1.00
Directional flow rate, (note-2) vi	1101 pc/h	1101 pc/h	462 pc/h

Free-Flow Speed from Field Measurement:  
 Field measured speed, (note-3) S FM - mi/h  
 Observed total demand, (note-3) V - veh/h  
 Estimated Free-Flow Speed:  
 Base free-flow speed, (note-3) BFFS 55.0 mi/h  
 Adj. for lane and shoulder width, (note-3) fLS 0.0 mi/h  
 Adj. for access point density, (note-3) fA 3.0 mi/h  
 Free-flow speed, FFSd 52.0 mi/h  
 Adjustment for no-passing zones, fnp 2.5 mi/h  
 Average travel speed, ATSD 37.4 mi/h  
 Percent Free Flow Speed, PFFS 71.9 %

Percent Time-Spent-Following

Direction	Analysis(d)	Opposing (o)
PCE for trucks, ET	1.0	1.0
PCE for RVs, ER	1.0	1.0
Heavy-vehicle adjustment factor, fhv	1.000	1.000
Grade adjustment factor, (note-1) fg	1.00	1.00
Directional flow rate, (note-2) vi	1048 pc/h	440 pc/h
Base percent time-spent-following, (note-4) BPTSfd	74.4 %	
Adjustment for no-passing zones, fnp	21.5 %	
Percent time-spent-following, PTSfd	89.5 %	

Level of Service and Other Performance Measures

Level of service, LOS	E
Volume to capacity ratio, v/c	0.62
Peak 15-min vehicle-miles of travel, VMT15	262 veh-mi
Peak-hour vehicle-miles of travel, VMT60	996 veh-mi
Peak 15-min total travel time, TTT15	7.0 veh-h
Capacity from AFS, CdATS	0 veh/h
Capacity from PTSF, CdPTSF	1700 veh/h
Directional Capacity	1700 veh/h

Passing Lane Analysis

Total length of analysis segment, Lt	1.0 mi
Length of two-lane highway upstream of the passing lane, Lu	- mi
Length of passing lane including tapers, Lpl	- mi
Average travel speed, ATSD (from above)	37.4 mi/h
Percent time-spent-following, PTSFd (from above)	89.5 %
Level of service, LOSd (from above)	E

Average Travel Speed with Passing Lane

Downstream length of two-lane highway within effective length of passing lane for average travel speed, Lde	- mi
Length of two-lane highway downstream of effective length of the passing lane for average travel speed, Ld	- mi
Adj. factor for the effect of passing lane on average speed, fpl	-
Average travel speed including passing lane, ATSpI	-
Percent free flow speed including passing lane, PFFSpI	0.0 %

Percent Time-Spent-Following with Passing Lane

Downstream length of two-lane highway within effective length of passing lane for percent time-spent-following, Lde	- mi
Length of two-lane highway downstream of effective length of the passing lane for percent time-spent-following, Ld	- mi
Adj. factor for the effect of passing lane on percent time-spent-following, fpl	-
Percent time-spent-following including passing lane, PTSFpl	- %

Level of Service and Other Performance Measures with Passing Lane

Level of service including passing lane, LOSpl	A
Peak 15-min total travel time, TTT15	- veh-h

Bicycle Level of Service

Posted speed limit, Sp 55  
 Percent of segment with occupied on-highway parking 0  
 Pavement rating, P 3  
 Flow rate in outside lane, VOL 1048.4  
 Effective width of outside lane, We 26.00  
 Effective speed factor, St 4.79  
 Bicycle LOS Score, BLOS 3.20  
 Bicycle LOS C

Notes:  
 1. Note that the adjustment factor for level terrain is 1.00, as level terrain is one of the base conditions. For the purpose of grade adjustment, specific downgrade segments are treated as level terrain.  
 2. If  $v_i (vd \text{ or } vo) >= 1,700 \text{ pc/h}$ , terminate analysis-the LOS is F.  
 3. For the analysis direction only, and for  $v > 200 \text{ veh/h}$ .  
 4. For the analysis direction only.  
 5. Use alternative Exhibit 15-14 if some trucks operate at crawl speeds on a specific downgrade.

\* These items have been entered or edited to override calculated value

Phone:  
E-Mail:

Fax:

Directional Two-Lane Highway Segment Analysis

Analyst Lauren Davini  
Agency/Co. Napa County  
Date Performed 9/8/15  
Analysis Time Period Weekend Midday Peak  
Highway Silverado Trail  
From/To 0.5mi N/O & S/O 4059 silvTrail  
Jurisdiction County of Napa  
Analysis Year 2015  
Description NB Existing + Project

Input Data

Highway class	Class 2	Peak hour factor, PHF	0.90
Shoulder width	7.0 ft	% Trucks and buses	5 %
Lane width	12.0 ft	% Trucks crawling	0.0 %
Segment length	1.0 mi	Truck crawl speed	0.0 mi/hr
Terrain type	Level	% Recreational vehicles	2 %
Grade:		% No-passing zones	100 %
Up/down	-	Access point density	15 /mi

Analysis direction volume, Vd 558 veh/h  
Opposing direction volume, Vo 397 veh/h

Average Travel Speed

Direction	Analysis (d)	Opposing (o)
PCE for trucks, ET	2.0*	2.0*
PCE for RVs, ER	1.0	1.0
Heavy-vehicle adj. factor, (note-5) fHV	0.952	0.952
Grade adj. factor, (note-1) fg	1.00	1.00
Directional flow rate, (note-2) vi	651 pc/h	463 pc/h

Free-Flow Speed from Field Measurement:

Field measured speed, (note-3) S <sub>FM</sub>	-	mi/h
Observed total demand, (note-3) V	-	veh/h
Estimated Free-Flow Speed:		
Base free-flow speed, (note-3) BFFS	55.0	mi/h
Adj. for lane and shoulder width, (note-3) fLS	0.0	mi/h
Adj. for access point density, (note-3) fA	3.8	mi/h
Free-flow speed, FFSd	51.3	mi/h

Adjustment for no-passing zones, fnp 2.5 mi/h  
Average travel speed, ATSD 40.1 mi/h  
Percent Free Flow Speed, PFFS 78.3 %

Direction	Analysis (d)	Opposing (o)
PCE for trucks, ET	1.0	1.0
PCE for RVs, ER	1.0	1.0
Heavy-vehicle adjustment factor, fHV	1.000	1.000
Grade adjustment factor, (note-1) fg	1.00	1.00
Directional flow rate, (note-2) vi	620 pc/h	441 pc/h
Base percent time-spent-following, (note-4) BPTSFD	57.0 %	57.0 %
Adjustment for no-passing zones, fnp	53.8	53.8
Percent time-spent-following, PTFSD	77.9 %	77.9 %

Level of Service and Other Performance Measures

Level of service, LOS	D
Volume to capacity ratio, v/c	0.36
Peak 15-min vehicle-miles of travel, VMT15	155 veh-mi
Peak 15-min vehicle-miles of travel, VMT60	558 veh-mi
Peak 15-min total travel time, TTT15	3.9 veh-h
Capacity from ATS, CdATS	0 veh/h
Capacity from PTFSD, CdPTSF	1700 veh/h
Directional Capacity	1700 veh/h

Passing Lane Analysis

Total length of analysis segment, Lt	1.0 mi
Length of two-lane highway upstream of the passing lane, Lu	- mi
Length of passing lane including tapers, Lpl	- mi
Average travel speed, ATSD (from above)	40.1 mi/h
Percent time-spent-following, PTFSD (from above)	77.9 %
Level of service, LOSd (from above)	D

Average Travel Speed with Passing Lane

Downstream length of two-lane highway within effective length of passing lane for average travel speed, Lde	- mi
Length of two-lane highway downstream of effective length of the passing lane for average travel speed, Ld	- mi
Adj. factor for the effect of passing lane on average speed, fpl	-
Average travel speed including passing lane, ATSpI	-
Percent free flow speed including passing lane, PFFSpI	0.0 %

Percent Time-Spent-Following with Passing Lane

Downstream length of two-lane highway within effective length of passing lane for percent time-spent-following, Lde	- mi
Length of two-lane highway downstream of effective length of the passing lane for percent time-spent-following, Ld	- mi
Adj. factor for the effect of passing lane on percent time-spent-following, fpl	-
Percent time-spent-following including passing lane, PTFSpI	- %

Level of Service and Other Performance Measures with Passing Lane

Level of service including passing lane, LOSpl	A
Peak 15-min total travel time, TTT15	- veh-h

Bicycle Level of Service

Posted speed limit, Sp 55  
 Percent of segment with occupied on-highway parking 0  
 Pavement rating, P 3  
 Flow rate in outside lane, VOL 620.0  
 Effective width of outside lane, We 26.00  
 Effective speed factor, St 4.79  
 Bicycle LOS Score, BLOS 2.93  
 Bicycle LOS C

Notes:

1. Note that the adjustment factor for level terrain is 1.00, as level terrain is one of the base conditions. For the purpose of grade adjustment, specific downgrade segments are treated as level terrain.
2. If  $v_i$  (vd or vo)  $\geq$  1,700 pc/h, terminate analysis-the LOS is F.
3. For the analysis direction only and for  $v > 200$  veh/h.
4. For the analysis direction only.
5. Use alternative Exhibit 15-14 if some trucks operate at crawl speeds on a specific downgrade.

\* These items have been entered or edited to override calculated value

Phone:   
 E-Mail:   
 Fax:

Directional Two-Lane Highway Segment Analysis

Analyst Lauren Davini  
 Agency/Co. Napa County  
 Date Performed 9/8/15  
 Analysis Time Period Weekend Midday Peak  
 Highway Silverado Trail  
 From/To 0.5mi N/o & S/o 4059 SilvTrail  
 Jurisdiction County of Napa  
 Analysis Year 2015  
 Description SB Existing + Project

Input Data

Highway class Class 2  
 Shoulder width 7.0 ft  
 Lane width 12.0 ft  
 Segment length 1.0 mi  
 Terrain type Level  
 Grade: Length - mi  
 Up/down - %  
 Peak hour factor, PHF 0.90  
 % Trucks and buses 5 %  
 % Trucks crawling 0.0 %  
 Truck crawl speed 0.0 mi/hr  
 % Recreational vehicles 2 %  
 % No-passing zones 100 %  
 Access point density 12 /mi

Analysis direction volume, Vd 397 veh/h  
 Opposing direction volume, Vo 558 veh/h

Average Travel Speed

Direction	Analysis(d)	Opposing (o)
PCE for trucks, ET	2.0*	2.0*
PCE for RVs, ER	1.0	1.0
Heavy-vehicle adj. factor, (note-5) fhv	0.952	0.952
Grade adj. factor, (note-1) fg	1.00	1.00
Directional flow rate, (note-2) vi	463 pc/h	651 pc/h

Free-Flow Speed from Field Measurement:

Field measured speed, (note-3) S FM - mi/h  
 Observed total demand, (note-3) V - veh/h  
 Estimated free-flow speed:  
 Base free-flow speed, (note-3) BFFS 55.0 mi/h  
 Adj. for lane and shoulder width, (note-3) fLS 0.0 mi/h  
 Adj. for access point density, (note-3) fA 3.0 mi/h  
 Free-flow speed, FFSd 52.0 mi/h  
 Adjustment for no-passing zones, fnp 1.8 mi/h  
 Average travel speed, ATSD 41.6 mi/h  
 Percent Free Flow Speed, PFFS 80.0 %

Posted speed limit, Sp 55  
 Percent of segment with occupied on-highway parking 0  
 Pavement rating, P 3  
 Flow rate in outside lane, VOL 441.1  
 Effective width of outside lane, We 26.00  
 Effective speed factor, St 4.79  
 Bicycle LOS Score, BLOS 2.76  
 Bicycle LOS C

Notes:  
 1. Note that the adjustment factor for level terrain is 1.00, as level terrain is one of the base conditions. For the purpose of grade adjustment, specific downgrade segments are treated as level terrain.  
 2. If  $v_i$  (vd or vo)  $\geq$  1,700 pc/h, terminate analysis-the LOS is F.  
 3. For the analysis direction only and for  $v > 200$  veh/h.  
 4. For the analysis direction only.  
 5. Use alternative Exhibit 15-14 if some trucks operate at crawl speeds on a specific downgrade.  
 \* These items have been entered or edited to override calculated value

Percent Time-Spent-Following		Opposing (o)	
Direction	Analysis(d)		
PCE for trucks, ET	1.0	1.0	
PCE for RVs, ER	1.0	1.0	
Heavy-vehicle adjustment factor, fHV	1.000	1.000	
Grade adjustment factor, (note-1) fg	1.00	1.00	
Directional flow rate, (note-2) vi	441 pc/h	620	pc/h
Base percent time-spent-following, (note-4) BFRStd	48.5 %		
Adjustment for no-passing zones, fnp	35.8		
Percent time-spent-following, PFSFd	63.4 %		

Level of Service and Other Performance Measures

Level of service, LOS	C
Volume to capacity ratio, v/c	0.26
Peak 15-min vehicle-miles of travel, VMT15	110 veh-mi
Peak-hour vehicle-miles of travel, VMT60	397 veh-mi
Peak 15-min total travel time, TT15	2.6 veh-h
Capacity from AFS, CdAFS	1692 veh/h
Capacity from PFSF, CdPFSF	1700 veh/h
Directional Capacity	1700 veh/h

Passing Lane Analysis

Total length of analysis segment, Lt	1.0 mi
Length of two-lane highway upstream of the passing lane, Lu	- mi
Length of passing lane including tapers, Lpl	- mi
Average travel speed, ATSD (from above)	41.6 mi/h
Percent time-spent-following, PFSFd (from above)	63.4 %
Level of service, LOSd (from above)	C

Average Travel Speed with Passing Lane

Downstream length of two-lane highway within effective length of passing lane for average travel speed, Lde	- mi
Length of two-lane highway downstream of effective length of the passing lane for average travel speed, Ld	- mi
Adj. factor for the effect of passing lane on average speed, fpl	-
Average travel speed including passing lane, ATSP1	-
Percent free flow speed including passing lane, PFFSP1	0.0 %

Percent Time-Spent-Following with Passing Lane

Downstream length of two-lane highway within effective length of passing lane for percent time-spent-following, Lde	- mi
Length of two-lane highway downstream of effective length of the passing lane for percent time-spent-following, Ld	- mi
Adj. factor for the effect of passing lane on percent time-spent-following, fpl	-
Percent time-spent-following including passing lane, PFSFpl	- %

Level of Service and Other Performance Measures with Passing Lane

Level of service including passing lane, LOSpl	A
Peak 15-min total travel time, TT15	- veh-h

Bicycle Level of Service

HCS 2010: Two-Lane Highways Release 6.70

Percent Time-Spent-Following

Direction  
 PCE for trucks, ET  
 PCE for RVs, ER  
 Heavy-vehicle adjustment factor, fHV  
 Grade adjustment factor, (note-1) fg  
 Directional flow rate, (note-2) vi  
 Base percent time-spent-following, (note-4) BFTSfd  
 Adjustment for no-passing zones, fnp  
 Percent time-spent-following, PFSfd

Analysis(d)  
 1.0  
 1.0  
 1.000  
 1.00  
 495  
 BFTSfd 58.5 %  
 20.1  
 64.7 %

Opposing (o)  
 1.0  
 1.0  
 1.000  
 1.00  
 1105  
 %  
 %

Phone:  
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Directional Two-Lane Highway Segment Analysis

Analyst Lauren Davini  
 Agency/Co. Napa County  
 Date Performed 9/8/15  
 Analysis Time Period Weekday PM Peak  
 Highway Silverado Trail  
 From/To 0.5mi N/o & S/o 4059 SilvTrail  
 Jurisdiction County of Napa  
 Analysis Year 2015  
 Description NB Cumulative

Input Data

Highway class Class 2  
 Shoulder width 7.0 ft  
 Lane width 12.0 ft  
 Segment length 1.0 mi  
 Terrain type Level  
 Grade: Up/down -  
 Peak hour factor, PHF 0.95  
 % Trucks and buses 5 %  
 Truck crawl speed 0.0 mi/hr  
 % Recreational vehicles 2 %  
 % No-passing zones 100 %  
 Access point density 15 /mi

Analysis direction volume, Vd 470 veh/h  
 Opposing direction volume, Vo 1050 veh/h

Average Travel Speed

Direction Analysis(d) Opposing (o)  
 PCE for trucks, ET 2.0\* 2.0\*  
 PCE for RVs, ER 1.0 1.0  
 Heavy-vehicle adj. factor, (note-3) fHV 0.952 0.952  
 Grade adj. factor, (note-1) fg 1.00 1.00  
 Directional flow rate, (note-2) vi 520 pc/h 1161 pc/h

Free-Flow Speed from Field Measurement:

Field measured speed, (note-3) S FM - mi/h  
 Observed total demand, (note-3) V - veh/h  
 Estimated Free-Flow Speed: 55.0 mi/h  
 Base free-flow speed, (note-3) BFFS 0.0 mi/h  
 Adj. for lane and shoulder width, (note-3) fLS 3.8 mi/h  
 Adj. for access point density, (note-3) fA 51.3 mi/h

Free-flow speed, PFSd 1.0 mi/h  
 Adjustment for no-passing zones, fnp 37.2 mi/h  
 Average travel speed, ATSD 72.6 %  
 Percent Free Flow Speed, PFFS

Level of Service and Other Performance Measures

Level of service, LOS C  
 Volume to capacity ratio, v/c 0.29  
 Peak 15-min vehicle-miles of travel, VMT15 124 veh-mi  
 Peak-hour vehicle-miles of travel, VMT60 470 veh-mi  
 Peak 15-min total travel time, TTI15 3.3 veh-h  
 Capacity from ATs, CdATS 1700 veh/h  
 Capacity from PFSF, CdPFSF 1700 veh/h  
 Directional Capacity 1700 veh/h

Passing Lane Analysis

Total length of analysis segment, Lt 1.0 mi  
 Length of two-lane highway upstream of the passing lane, Lu - mi  
 Length of passing lane including tapers, Lpl - mi  
 Average travel speed, ATSD (from above) 37.2 mi/h  
 Percent time-spent-following, PFSfd (from above) 64.7 %  
 Level of service, LOSd (from above) C

Average Travel Speed with Passing Lane

Downstream length of two-lane highway within effective length of passing lane for average travel speed, Lde - mi  
 Length of two-lane highway downstream of effective length of the passing lane for average travel speed, Ld - mi  
 Adj. factor for the effect of passing lane on average speed, fpl -  
 Average travel speed including passing lane, ATSppl -  
 Percent free flow speed including passing lane, PFFSppl 0.0 %

Percent Time-Spent-Following with Passing Lane

Downstream length of two-lane highway within effective length of passing lane for percent time-spent-following, Lde - mi  
 Length of two-lane highway downstream of effective length of the passing lane for percent time-spent-following, Ld - mi  
 Adj. factor for the effect of passing lane on percent time-spent-following, fpl -  
 Percent time-spent-following including passing lane, PFSfpl - %

Level of Service and Other Performance Measures with Passing Lane

Level of service including passing lane, LOSpl A  
 Peak 15-min total travel time, TTI15 - veh-h

Bicycle Level of Service

Posted speed limit, sp 55  
 Percent of segment with occupied on-highway parking 0  
 Pavement rating, p 3  
 Flow rate in outside lane, vol 494.7  
 Effective width of outside lane, we 26.00  
 Effective speed factor, St 4.79  
 Bicycle LOS Score, BLOS 2.82  
 Bicycle LOS C

- Notes:
- Note that the adjustment factor for level terrain is 1.00, as level terrain is one of the base conditions. For the purpose of grade adjustment, specific downgrade segments are treated as level terrain.
  - If  $v_i$  (vd or vo)  $\geq$  1,700 pc/h, terminate analysis-the LOS is F.
  - For the analysis direction only and for  $v > 200$  veh/h.
  - For the analysis direction only.
  - Use alternative Exhibit 15-14 if some trucks operate at crawl speeds on a specific downgrade.

\* These items have been entered or edited to override calculated value

Phone:  
 E-Mail:

Fax:

Directional Two-Lane Highway Segment Analysis

Analyst Lauren Davini  
 Agency/Co. Napa County  
 Date Performed 9/8/15  
 Analysis Time Period Weekday PM Peak  
 Highway Silverado Trail  
 From/To 0.5mi N/o & S/o 4059 SilvTrail  
 Jurisdiction County of Napa  
 Analysis Year 2015  
 Description SB Cumulative

Input Data

Highway class	Class 2	7.0	ft	Peak hour factor, PHF	0.95	
Shoulder width		12.0	ft	% Trucks and buses	5	%
Lane width		1.0	mi	Trucks crawling	0.0	%
Segment length			Level	Truck crawl speed	0.0	mi/hr
Terrain type				% Recreational vehicles	2	%
Grade:	Length			% No-passing zones	100	%
	Up/down			Access point density	12	/mi

Analysis direction volume, Vd 1050 veh/h  
 Opposing direction volume, Vo 470 veh/h

Average Travel Speed

Direction	Analysis(d)	Opposing (o)
PCE for trucks, ET	2.0*	2.0*
PCE for RVs, ER	1.0	1.0
Heavy-vehicle adj. factor, (note-5) fHV	0.952	0.952
Grade adj. factor, (note-1) fg	1.00	1.00
Directional flow rate, (note-2) vi	1161	520
	pc/h	pc/h

Free-Flow Speed from Field Measurement:

Field measured speed, (note-3) S FM	-	mi/h
Observed total demand, (note-3) V	-	veh/h
Estimated Free-Flow Speed:		
Base free-flow speed, (note-3) BFFS	55.0	mi/h
Adj. for lane and shoulder width, (note-3) fLS	0.0	mi/h
Adj. for access point density, (note-3) fA	3.0	mi/h
Free-flow speed, FFSD	52.0	mi/h
Adjustment for no-passing zones, fnp	2.2	mi/h
Average travel speed, ATSD	36.7	mi/h
Percent Free Flow Speed, PFFS	70.6	%

Direction  
 PCE for trucks, ET  
 PCE for RVs, ER  
 Heavy-vehicle adjustment factor, fHV  
 Grade adjustment factor, (note-1) fg  
 Directional flow rate, (note-2) vi  
 Base percent time-spent-following, (note-4) BPTSfd  
 Adjustment for no-passing zones, fnp  
 Percent time-spent-following, PTSfd

Analysis(d)  
 1.0  
 1.0  
 1.000  
 1.00  
 1105 pc/h  
 76.8 %  
 90.7 %

Opposing (o)  
 1.0  
 1.0  
 1.000  
 1.00  
 495 pc/h  
 %  
 %

Level of Service and Other Performance Measures  
 Level of service, LOS  
 Volume to capacity ratio, v/c  
 Peak 15-min vehicle-miles of travel, VMT15  
 Peak-hour vehicle-miles of travel, VMT60  
 Peak 15-min total travel time, TT15  
 Capacity from ATS, CdATS  
 Capacity from PTSF, CdPTSF  
 Directional Capacity

E  
 0.65  
 276 veh-mi  
 1050 veh-mi  
 7.5 veh/h  
 0 veh/h  
 1700 veh/h  
 1700 veh/h

Passing Lane Analysis

Total length of analysis segment, Lt  
 Length of two-lane highway upstream of the passing lane, Lu  
 Length of passing lane including tapers, Lpl  
 Average travel speed, ATSD (from above)  
 Percent time-spent-following, PTSfd (from above)  
 Level of service, LOSd (from above)

1.0 mi  
 - mi  
 - mi  
 36.7 mi/h  
 90.7  
 E

Average Travel Speed with Passing Lane

Downstream length of two-lane highway within effective length of passing lane for average travel speed, Lde  
 Length of two-lane highway downstream of effective length of the passing lane for average travel speed, Ld  
 Adj. factor for the effect of passing lane on average speed, fpl  
 Average travel speed including passing lane, ATSpl  
 Percent free flow speed including passing lane, PFFSpl

- mi  
 - mi  
 -  
 -  
 0.0 %

Percent Time-Spent-Following with Passing Lane

Downstream length of two-lane highway within effective length of passing lane for percent time-spent-following, Lde  
 Length of two-lane highway downstream of effective length of the passing lane for percent time-spent-following, Ld  
 Adj. factor for the effect of passing lane on percent time-spent-following, fpl  
 Percent time-spent-following including passing lane, PTSFpl

- mi  
 - mi  
 -  
 - %

Level of Service and Other Performance Measures with Passing Lane

Level of service including passing lane, LOSpl  
 Peak 15-min total travel time, TT15

A  
 - veh-h

Bicycle Level of Service

Posted speed limit, Sp  
 Percent of segment with occupied on-highway parking  
 Pavement rating, P  
 Flow rate in outside lane, VOL  
 Effective width of outside lane, We  
 Effective speed factor, St  
 Bicycle LOS Score, BLOS  
 Bicycle LOS

55  
 0  
 3  
 1105.3  
 26.00  
 4.79  
 3.23  
 C

Notes:  
 1. Note that the adjustment factor for level terrain is 1.00, as level terrain is one of the base conditions. For the purpose of grade adjustment, specific downgrade segments are treated as level terrain.  
 2. If vi (vd or vo) >= 1,700 pc/h, terminate analysis-the LOS is F.  
 3. For the analysis direction only and for v>200 veh/h.  
 4. For the analysis direction only.  
 5. Use alternative Exhibit 15-14 if some trucks operate at crawl speeds on a specific downgrade.

\* These items have been entered or edited to override calculated value

Phone:  
E-Mail:

Fax:

Directional Two-Lane Highway Segment Analysis

Analyst Lauren Davini  
 Agency/Co. Napa County  
 Date Performed 9/8/15  
 Analysis Time Period Weekend Midday Peak  
 Highway Silverado Trail  
 From/To 0.5mi N/O & S/O 4059 SilvTrail  
 Jurisdiction County of Napa  
 Analysis Year 2015  
 Description NB Cumulative

Input Data

Highway class Class 2  
 Shoulder width 7.0 ft  
 Lane width 12.0 ft  
 Segment length 1.0 mi  
 Terrain type Level  
 Grade: Up/down  
 Peak hour factor, PHF 0.90  
 % Trucks and buses 5 %  
 % Trucks crawling 0.0 %  
 Truck crawl speed 0.0 mi/hr  
 % Recreational vehicles 2 %  
 % No-passing zones 100 %  
 Access point density 15 /mi

Analysis direction volume, Vd 635 veh/h  
 Opposing direction volume, Vo 473 veh/h

Average Travel Speed

Direction	Analysis (d)	Opposing (o)
PCE for trucks, ET	2.0*	2.0*
PCE for RVs, ER	1.0	1.0
Heavy-vehicle adj. factor, (note-5) fHV	0.952	0.952
Grade adj. factor, (note-1) fg	1.00	1.00
Directional flow rate, (note-2) vi	741 pc/h	552 pc/h

Free-Flow Speed from Field Measurement:

Field measured speed, (note-3) S FM - mi/h  
 Observed total demand, (note-3) V - veh/h  
 Estimated Free-Flow Speed: 55.0 mi/h  
 Base free-flow speed, (note-3) BFFS 0.0 mi/h  
 Adj. for lane and shoulder width, (note-3) fLS 3.8 mi/h  
 Adj. for access point density, (note-3) fA 51.3 mi/h

Free-flow speed, FFSd 2.1 mi/h  
 Adjustment for no-passing zones, fnp 39.1 mi/h  
 Average travel speed, ATSD 76.3 %

Percent Free Flow Speed, PFFS

Direction Analysis (d) Opposing (o)  
 PCE for trucks, ET 1.0 1.0  
 PCE for RVs, ER 1.0 1.0  
 Heavy-vehicle adjustment factor, fHV 1.000 1.000  
 Grade adjustment factor, (note-1) fg 1.00 1.00  
 Directional flow rate, (note-2) vi 706 pc/h 526 pc/h  
 Base percent time-spent-following, (note-4) BFFSfd 63.0 %  
 Adjustment for no-passing zones, fnp 31.9 %  
 Percent time-spent-following, PFSfd 81.3 %

Level of Service and Other Performance Measures

Level of service, LOS D  
 Volume to capacity ratio, v/c 0.42  
 Peak 15-min vehicle-miles of travel, VMT15 176 veh-mi  
 Peak-hour vehicle-miles of travel, VMT60 633 veh-mi  
 Peak 15-min total travel time, TT15 4.5 veh-h  
 Capacity from ATS, CdATS 0 veh/h  
 Capacity from PTSF, CdPTSF 1700 veh/h  
 Directional Capacity 1700 veh/h

Passing Lane Analysis

Total length of analysis segment, Lt 1.0 mi  
 Length of two-lane highway upstream of the passing lane, Lu - mi  
 Length of passing lane including tapers, Lpl - mi  
 Average travel speed, AVSD (from above) 39.1 mi/h  
 Percent time-spent-following, PFSfd (from above) 81.3 %  
 Level of service, LOSd (from above) D

Average Travel Speed with Passing Lane

Downstream length of two-lane highway within effective length of passing lane for average travel speed, Lde - mi  
 Length of two-lane highway downstream of effective length of the passing lane for average travel speed, Ld - mi  
 Adj. factor for the effect of passing lane on average speed, fpl -  
 Average travel speed including passing lane, ATSpI -  
 Percent free flow speed including passing lane, PFFSpI 0.0 %

Percent Time-Spent-Following with Passing Lane

Downstream length of two-lane highway within effective length of passing lane for percent time-spent-following, Lde - mi  
 Length of two-lane highway downstream of effective length of the passing lane for percent time-spent-following, Ld - mi  
 Adj. factor for the effect of passing lane on percent time-spent-following, fpl -  
 Percent time-spent-following including passing lane, PFSfpl - %

Level of Service and Other Performance Measures with Passing Lane

Level of service including passing lane, LOSpl A  
 Peak 15-min total travel time, TT15 - veh-h

Bicycle Level of Service

Posted speed limit, Sp 55  
 Percent of segment with occupied on-highway parking 0  
 Pavement rating, P 3  
 Flow rate in outside lane, VOL 705.6  
 Effective width of outside lane, We 26.00  
 Effective speed factor, St 4.79  
 Bicycle LOS Score, BLOS 3.00  
 Bicycle LOS C

Notes:

1. Note that the adjustment factor for level terrain is 1.00, as level terrain is one of the base conditions. For the purpose of grade adjustment, specific downgrade segments are treated as level terrain.
2. If  $v_i$  (vd or vo)  $\geq 1,700$  pc/h, terminate analysis-the LOS is F.
3. For the analysis direction only and for  $v > 200$  veh/h.
4. For the analysis direction only.
5. Use alternative Exhibit 15-14 if some trucks operate at crawl speeds on a specific downgrade.

\* These items have been entered or edited to override calculated value

Phone: E-Mail: Fax:

Directional Two-Lane Highway Segment Analysis

Analyst Lauren Davini  
 Agency/Co. Napa County  
 Date Performed 9/8/15  
 Analysis Time Period Weekend Midday Peak  
 Highway Silverado Trail  
 From/To 0.5mi N/o & S/o 4059 SilvTrail  
 Jurisdiction County of Napa  
 Analysis Year 2015  
 Description SB Cumulative

Input Data

Highway class Class 2  
 Shoulder width 7.0 ft  
 Lane width 12.0 ft  
 Segment length 1.0 mi  
 Terrain type Level  
 Grade: Length mi  
 Up/down %  
 Peak hour factor, PHF 0.90  
 % Trucks and buses 5 %  
 % Trucks crawling 0.0 %  
 Truck crawl speed 0.0 mi/hr  
 % Recreational vehicles 2 %  
 % No-passing zones 100 %  
 Access point density 12 /mi

Analysis direction volume, Vd 473 veh/h  
 Opposing direction volume, Vo 635 veh/h

Average Travel Speed

Direction	Analysis(d)	Opposing (o)
PCE for trucks, ET	2.0*	2.0*
PCE for RVs, ER	1.0	1.0
Heavy-vehicle adj. factor, (note-5) fhv	0.952	0.952
Grade adj. factor, (note-1) fg	1.00	1.00
Directional flow rate, (note-2) vi	552 pc/h	741 pc/h

Free-Flow Speed from Field Measurement:

Field measured speed, (note-3) S FM - mi/h  
 Observed total demand, (note-3) V - veh/h  
 Estimated Free-Flow Speed:  
 Base free-flow speed, (note-3) BFFS 55.0 mi/h  
 Adj. for lane and shoulder width, (note-3) fLS 0.0 mi/h  
 Adj. for access point density, (note-3) fA 3.0 mi/h  
 Free-flow speed, FFSD 52.0 mi/h  
 Adjustment for no-passing zones, fnp 1.5 mi/h  
 Average travel speed, ATSc 40.5 mi/h  
 Percent Free Flow Speed, PFFS 77.8 %



Directional Two-Lane Highway Segment Analysis

Analyst Lauren Davini  
 Agency/Co. Napa County  
 Date Performed 9/8/15  
 Analysis Time Period Weekday PM Peak  
 Highway Silverado Trail  
 From/To 0.5mi N/o & S/o 4059 SilverTrail  
 Jurisdiction County of Napa  
 Analysis Year 2015  
 Description NB Cumulative + Project

Phone:  
 E-Mail:  
 Fax:

Direction Percent Time-Spent-Following Analysis(d) Opposing (o)  
 PCE for trucks, ET 1.0 1.0  
 PCE for RVs, ER 1.0 1.0  
 Heavy-vehicle adjustment factor, fHV 1.000 1.000  
 Grade adjustment factor, (note-1) fg 1.00 1.00  
 Directional flow rate, (note-2) vi 500 pc/h 1115 pc/h  
 Base percent time-spent-following, (note-4) BPTSfd 58.8 %  
 Adjustment for no-passing zones, fnp 19.9 %  
 Percent time-spent-following, PTFSD 65.0 %

Level of Service and Other Performance Measures  
 Level of service, LOS C  
 Volume to capacity ratio, v/c 0.29  
 Peak 15-min vehicle-miles of travel, VMT15 125 veh-mi  
 Peak-hour vehicle-miles of travel, VMT60 475 veh-mi  
 Peak 15-min total travel time, TTT15 3.4 veh-h  
 Capacity from AVT, CdAVT 1700 veh/h  
 Capacity from PTF, CdPTF 1700 veh/h  
 Directional Capacity 1700 veh/h

Passing Lane Analysis  
 Total length of analysis segment, Lt 1.0 mi  
 Length of two-lane highway upstream of the passing lane, Lu - mi  
 Length of passing lane including tapers, Lpl - mi  
 Average travel speed, ATSD (from above) 37.1 mi/h  
 Percent time-spent-following, PTFSD (from above) 65.0 %  
 Level of service, LOSd (from above) C

Average Travel Speed with Passing Lane  
 Downstream length of two-lane highway within effective length of passing lane for average travel speed, Lde - mi  
 Length of two-lane highway downstream of effective length of the passing lane for average travel speed, Ld - mi  
 Adj. factor for the effect of passing lane on average speed, fpl -  
 Average travel speed including passing lane, ATSpI -  
 Percent free flow speed including passing lane, PFFSpI 0.0 %

Percent Time-Spent-Following with Passing Lane  
 Downstream length of two-lane highway within effective length of passing lane for percent time-spent-following, Lde - mi  
 Length of two-lane highway downstream of effective length of the passing lane for percent time-spent-following, Ld - mi  
 Adj. factor for the effect of passing lane on percent time-spent-following, fpl -  
 Percent time-spent-following including passing lane, PTFSpI - %

Level of Service and Other Performance Measures with Passing Lane  
 Level of service including passing lane, LOSpl A  
 Peak 15-min total travel time, TTT15 - veh-h  
 Bicycle Level of Service

Input Data

Highway class Class 2 Peak hour factor, PHF 0.95  
 Shoulder width 7.0 ft % Trucks and buses 5 %  
 Lane width 12.0 ft % Trucks crawling 0.0 %  
 Segment length 1.0 mi Truck crawl speed 0.0 mi/hr  
 Terrain type Level % Recreational vehicles 2 %  
 Grade: Up/down - % No-passing zones 100 %  
 Access point density 15 /mi  
 Analysis direction volume, Vd 475 veh/h  
 Opposing direction volume, Vo 1059 veh/h

Average Travel Speed  
 Direction Analysis(d) Opposing (o)  
 PCE for trucks, ET 2.0\* 2.0\*  
 PCE for RVs, ER 1.0 1.0  
 Heavy-vehicle adj. factor, (note-5) fHV 0.952 0.952  
 Grade adj. factor, (note-1) fg 1.00 1.00  
 Directional flow rate, (note-2) vi 525 pc/h 1171 pc/h

Free-Flow Speed from Field Measurement:  
 Field measured speed, (note-3) S.FM - mi/h  
 Observed total demand, (note-3) V - veh/h  
 Estimated Free-Flow Speed:  
 Base free-flow speed, (note-3) BFFS 55.0 mi/h  
 Adj. for lane and shoulder width, (note-3) fLS 0.0 mi/h  
 Adj. for access point density, (note-3) fA 3.8 mi/h

Free-flow speed, PFFSD 51.3 mi/h  
 Adjustment for no-passing zones, fnp 1.0 mi/h  
 Average travel speed, ATSD 37.1 mi/h  
 Percent Free Flow Speed, PFFS 72.3 %

Posted speed limit, Sp 55  
 Percent of segment with occupied on-highway parking 0  
 Pavement rating, P 3  
 Flow rate in outside lane, VOL 500.0  
 Effective width of outside lane, We 26.00  
 Bicycle LOS Score, BLOS 4.79  
 Bicycle LOS 2.82  
 C

Notes:

- Note that the adjustment factor for level terrain is 1.00, as level terrain is one of the base conditions. For the purpose of grade adjustment, specific downgrade segments are treated as level terrain.
- If  $v_i (vd \text{ or } vo) \geq 1,700$  pc/h, terminate analysis-the LOS is F.
- For the analysis direction only and for  $v > 200$  veh/h.
- For the analysis direction only.
- Use alternative Exhibit 15-14 if some trucks operate at crawl speeds on a specific downgrade.

\* These items have been entered or edited to override calculated value

Phone:  
 E-Mail:

Fax:

Directional Two-Lane Highway Segment Analysis

Analyst Lauren Davini  
 Agency/Co. Napa County  
 Date Performed 9/8/15  
 Analysis Time Period Weekday PM Peak  
 Highway Silverado Trail  
 From/To 0.5mi N/o & S/o 4059 SilVTrail  
 Jurisdiction County of Napa  
 Analysis Year 2015  
 Description SB Cumulative + Project

Input Data

Highway class	Class 2	Peak hour factor, PHF	0.95
Shoulder width	7.0 ft	% Trucks and buses	5 %
Lane width	12.0 ft	% Trucks crawling	0.0 %
Segment length	1.0 mi	Truck crawl speed	0.0 mi/hr
Terrain type	Level	% Recreational vehicles	2 %
Grade:	Length -	% No-passing zones	100 %
	Up/down -	Access point density	12 /mi

Analysis direction volume, Vd 1059 veh/h  
 Opposing direction volume, Vo 475 veh/h

Average Travel Speed

Direction	Analysis(d)	Opposing (o)
PCE for trucks, ET	2.0*	2.0*
PCE for RVs, ER	1.0	1.0
Heavy-vehicle adj. factor, (note-5) fHV	0.952	0.952
Grade adj. factor, (note-1) fg	1.00	1.00
Directional flow rate, (note-2) vi	1171 pc/h	525 pc/h

Free-Flow Speed from Field Measurement:

Field measured speed, (note-3) S <sub>FM</sub>	-	mi/h
Observed total demand, (note-3) V	-	veh/h
Estimated Free-Flow Speed:		
Base free-flow speed, (note-3) BFFS	55.0	mi/h
Adj. for lane and shoulder width, (note-3) fLS	0.0	mi/h
Adj. for access point density, (note-3) fA	3.0	mi/h

Free-flow speed, FFSd 52.0 mi/h

Adjustment for no-passing zones, fnp 2.2 mi/h  
 Average travel speed, ATSD 36.6 mi/h  
 Percent Free Flow Speed, PFFS 70.4 %

Percent Time-Spent-Following

Direction	Analysis(d)	Opposing (o)
PCE for trucks, ET	1.0	1.0
PCE for RVs, ER	1.0	1.0
Heavy-vehicle adjustment factor, fHV	1.000	1.000
Grade adjustment factor, (note-1) fg	1.00	1.00
Directional flow rate, (note-2) vi	1115 pc/h	500 pc/h
Base percent time-spent-following, (note-4) BPTSfd	76.6 %	
Adjustment for no-passing zones, fnp	19.9	
Percent time-spent-following, PFSfd	90.3 %	

Level of Service and Other Performance Measures

Level of service, LOS	E
Volume to capacity ratio, v/c	0.66
Peak 15-min vehicle-miles of travel, VMT15	279 veh-mi
Peak-hour vehicle-miles of travel, VMT60	1059 veh-mi
Peak 15-min total travel time, TT15	7.6 veh-h
Capacity from AFS, CdATS	0 veh/h
Capacity from PFSF, CdPFSF	1700 veh/h
Directional Capacity	1700 veh/h

Passing Lane Analysis

Total length of analysis segment, Lt	1.0 mi
Length of two-lane highway upstream of the passing lane, Lu	- mi
Length of passing lane including tapers, Lpl	- mi
Average travel speed, ATSD (from above)	36.6 mi/h
Percent time-spent-following, PTFSD (from above)	90.3
Level of service, LOSd (from above)	E

Average Travel Speed with Passing Lane

Downstream length of two-lane highway within effective length of passing lane for average travel speed, Lde	- mi
Length of two-lane highway downstream of effective length of the passing lane for average travel speed, Ld	- mi
Adj. factor for the effect of passing lane on average speed, fpl	-
Average travel speed including passing lane, ATSP1	-
Percent free flow speed including passing lane, PFFSP1	0.0 %

Percent Time-Spent-Following with Passing Lane

Downstream length of two-lane highway within effective length of passing lane for percent time-spent-following, Lde	- mi
Length of two-lane highway downstream of effective length of the passing lane for percent time-spent-following, Ld	- mi
Adj. factor for the effect of passing lane on percent time-spent-following, fpl	-
Percent time-spent-following including passing lane, PTFSP1	- %

Level of Service and Other Performance Measures with Passing Lane

Level of service including passing lane, LOSpl	A
Peak 15-min total travel time, TT15	- veh-h

Bicycle Level of Service

Posted speed limit, SP 55  
 Percent of segment with occupied on-highway parking 0  
 Pavement rating, P 3  
 Flow rate in outside lane, vOL 1114.7  
 Effective width of outside lane, We 26.00  
 Effective speed factor, St 4.79  
 Bicycle LOS Score, BLOS 3.23  
 Bicycle LOS C

Notes:  
 1. Note that the adjustment factor for level terrain is 1.00, as level terrain is one of the base conditions. For the purpose of grade adjustment, specific downgrade segments are treated as level terrain.  
 2. If vi (vd or vo) >= 1,700 pc/h, terminate analysis-the LOS is F.  
 3. For the analysis direction only, and for v>200 veh/h.  
 4. For the analysis direction only.  
 5. Use alternative Exhibit 15-14 if some trucks operate at crawl speeds on a specific downgrade.

\* These items have been entered or edited to override calculated value

Direction Analysis (d) Opposing (o)  
 PCE for trucks, ET 1.0 1.0  
 PCE for RVs, ER 1.0 1.0  
 Heavy-vehicle adjustment factor, fhv 1.000 1.000  
 Grade adjustment factor, (note-1) fg 1.00 1.00  
 Directional flow rate, (note-2) vi 716 pc/h 537 pc/h  
 Base percent time-spent-following, (note-4) BPTSFD 64.0 %  
 Adjustment for no-passing zones, fnp 31.4 %  
 Percent time-spent-following, PTSFD 81.9 %

Level of Service and Other Performance Measures

Level of service, LOS D  
 Volume to capacity ratio, v/c 0.42  
 Peak 15-min vehicle-miles of travel, VMT15 179 veh-mi  
 Peak 15-min total travel time, TTT15 644 veh-h  
 Capacity from ATIS, CdATS 0 veh/h  
 Capacity from PTSF, CdPTSF 1700 veh/h  
 Directional Capacity 1700 veh/h

Phone:  
 E-Mail:

Directional Two-Lane Highway Segment Analysis

Analyst Lauren Davini  
 Agency/Co. Napa County  
 Date Performed 9/8/15  
 Analysis Time Period Weekend Midday Peak  
 Highway Silverado Trail  
 From/To 0.5mi N/O & S/O 4059 SilvTrail  
 Jurisdiction County of Napa  
 Analysis Year 2015  
 Description NB Cumulative + Project

Input Data

Highway class Class 2 Peak hour factor, PHF 0.90  
 Shoulder width 7.0 ft % Trucks and buses 5 %  
 Lane width 12.0 ft % Trucks crawling 0.0 %  
 Segment length 1.0 mi Truck crawl speed 0.0 mi/hr  
 Terrain type Level % Recreational vehicles 2 %  
 Grade: Length mi % No-passing zones 100 %  
 Up/Down % Access point density 15 /mi

Analysis direction volume, Vd 644 veh/h  
 Opposing direction volume, Vo 483 veh/h

Average Travel Speed

Direction Analysis (d) Opposing (o)  
 PCE for trucks, ET 2.0\* 2.0\*  
 PCE for RVs, ER 1.0 1.0  
 Heavy-vehicle adj. factor, (note-5) fhv 0.952 0.952  
 Grade adj. factor, (note-1) fg 1.00 1.00  
 Directional flow rate, (note-2) vi 752 pc/h 564 pc/h

Free-Flow Speed from Field Measurement:

Field measured speed, (note-3) S FM - mi/h  
 Observed total demand, (note-3) V - veh/h  
 Estimated Free-Flow Speed: 39.0 mi/h  
 Base free-flow speed, (note-3) BFFS 55.0 mi/h  
 Adj. for lane and shoulder width, (note-3) fLS 0.0 mi/h  
 Adj. for access point density, (note-3) fA 3.8 mi/h  
 Free-flow speed, FFSd 51.3 mi/h

Adjustment for no-passing zones, fnp 2.0 mi/h  
 Average travel speed, ATSD 39.0 mi/h  
 Percent Free Flow Speed, PFFS 76.1 %

Passing Lane Analysis

Total length of analysis segment, Lt 1.0 mi  
 Length of two-lane highway upstream of the passing lane, Lu - mi  
 Length of passing lane including tapers, Lpl - mi  
 Average travel speed, ATSD (from above) 39.0 mi/h  
 Percent time-spent-following, PTSFD (from above) 81.9 %  
 Level of service, LOSd (from above) D

Average Travel Speed with Passing Lane

Downstream length of two-lane highway within effective length of passing lane for average travel speed, Lde - mi  
 Length of two-lane highway downstream of effective length of the passing lane for average travel speed, Ld - mi  
 Adj. factor for the effect of passing lane on average speed, fpl -  
 Average travel speed including passing lane, ATSPpl -  
 Percent free flow speed including passing lane, PFFSpl 0.0 %

Percent Time-Spent-Following with Passing Lane

Downstream length of two-lane highway within effective length of passing lane for percent time-spent-following, Lde - mi  
 Length of two-lane highway downstream of effective length of the passing lane for percent time-spent-following, Ld - mi  
 Adj. factor for the effect of passing lane on percent time-spent-following, fpl -  
 Percent time-spent-following including passing lane, PTSFpl - %

Level of Service and Other Performance Measures with Passing Lane

Level of service including passing lane, LOSpl A  
 Peak 15-min total travel time, TTT15 - veh-h

Bicycle Level of Service

Posted speed limit, Sp 55  
 Percent of segment with occupied on-highway parking 0  
 Pavement rating, P 3  
 Flow rate in outside lane, VOL 715.6  
 Effective width of outside lane, We 26.00  
 Effective speed factor, St 4.79  
 Bicycle LOS Score, BLOS 3.01  
 Bicycle LOS C

Notes:

1. Note that the adjustment factor for level terrain is 1.00, as level terrain is one of the base conditions. For the purpose of grade adjustment, specific downgrade segments are treated as level terrain.
2. If  $v_i$  (vd or vo)  $\geq$  1,700 pc/h, terminate analysis-the LOS is F.
3. For the analysis direction only and for  $v > 200$  veh/h.
4. For the analysis direction only.
5. Use alternative Exhibit 15-14 if some trucks operate at crawl speeds on a specific downgrade.

\* These items have been entered or edited to override calculated value

Phone:  
 E-Mail:

Fax:

Directional Two-Lane Highway Segment Analysis

Analyst Lauren Davini  
 Agency/Co. Napa County  
 Date Performed 9/8/15  
 Analysis Time Period Weekend Midday Peak  
 Highway Silverado Trail  
 From/To 0.5mi N/o & S/o 4059 SilvTrail  
 Jurisdiction County of Napa  
 Analysis Year 2015  
 Description SB Cumulative + Project

Input Data

Highway class	Class 2	Peak hour factor, PHF	0.90
Shoulder width	7.0 ft	% Trucks and buses	5 %
Lane width	12.0 ft	% Trucks crawling	0.0 %
Segment length	1.0 mi	Truck crawl speed	0.0 mi/hr
Terrain type	Level	% Recreational vehicles	2 %
Grade:	Length -	% No-passing zones	100 %
	Up/down -	Access point density	12 /mi

Analysis direction volume, Vd 483 veh/h  
 Opposing direction volume, Vo 644 veh/h

Average Travel Speed

Direction	Analysis(d)	Opposing (o)
PCE for trucks, ET	2.0*	2.0*
PCE for RVs, ER	1.0	1.0
Heavy-vehicle adj. factor, (note-5) FHV	0.952	0.952
Grade adj. factor, (note-1) fg	1.00	1.00
Directional flow rate, (note-2) vi	564 pc/h	752 pc/h

Free-Flow Speed from Field Measurement:

Field measured speed, (note-3) S FM	-	mi/h
Observed total demand, (note-3) V	-	veh/h
Estimated Free-Flow Speed:		
Base free-flow speed, (note-3) BFFS	55.0	mi/h
Adj. for lane and shoulder width, (note-3) fLS	0.0	mi/h
Adj. for access point density, (note-3) fA	3.0	mi/h

Free-flow speed, FFSD 52.0 mi/h

Adjustment for no-passing zones, fnp 1.5 mi/h  
 Average travel speed, ATSD 40.3 mi/h  
 Percent Free Flow Speed, PFFS 77.5 %

Posted speed limit, SP 55  
 Percent of segment with occupied on-highway parking 0  
 Pavement rating, P 3  
 Flow rate in outside lane, VOL 536.7  
 Effective width of outside lane, We 26.00  
 Effective speed factor, St 4.79  
 Bicycle LOS Score, BLOS 2.86  
 Bicycle LOS C

- Notes:
1. Note that the adjustment factor for level terrain is 1.00, as level terrain is one of the base conditions. For the purpose of grade adjustment, specific downgrade segments are treated as level terrain.
  2. If  $v_i$  (vd or vo)  $\geq 1,700$  pc/h, terminate analysis-the LOS is F.
  3. For the analysis direction only and for  $v > 200$  veh/h.
  4. For the analysis direction only.
  5. Use alternative Exhibit 13-14 if some trucks operate at crawl speeds on a specific downgrade.

\* These items have been entered or edited to override calculated value

Percent Time-Spent-Following

Direction	Analysis(d)	Opposing (o)
PCE for trucks, ET	1.0	1.0
PCE for RVs, ER	1.0	1.0
Heavy-vehicle adjustment factor, fHV	1.000	1.000
Grade adjustment factor, (note-1) f <sub>g</sub>	1.00	1.00
Directional flow rate, (note-2) v <sub>i</sub>	537 pc/h	716 pc/h
Base percent time-spent-following, (note-4) BPTSfd	56.5 %	
Adjustment for no-passing zones, f <sub>np</sub>	31.4	
Percent time-spent-following, PTSfd	70.0 %	

Level of Service and Other Performance Measures

Level of service, LOS	C
Volume to capacity ratio, v/c	0.32
Peak 15-min vehicle-miles of travel, VMt15	134 veh-mi
Peak-hour vehicle-miles of travel, VMt60	483 veh-mi
Peak 15-min total travel time, TT15	3.3 veh-h
Capacity from ATS, CdATS	1692 veh/h
Capacity from PTSF, CdPTSF	1700 veh/h
Directional Capacity	1700 veh/h

Passing Lane Analysis

Total length of analysis segment, Lt	1.0 mi
Length of two-lane highway upstream of the passing lane, Lu	- mi
Length of passing lane including tapers, Lpl	- mi
Average travel speed, ATSD (from above)	40.3 mi/h
Percent time-spent-following, PTSfd (from above)	70.0 %
Level of service, LOSd (from above)	C

Average Travel Speed with Passing Lane

Downstream length of two-lane highway within effective length of passing lane for average travel speed, Lde	- mi
Length of two-lane highway downstream of effective length of the passing lane for average travel speed, Ld	- mi
Adj. factor for the effect of passing lane on average speed, fpl	-
Average travel speed including passing lane, ATSpI	-
Percent free flow speed including passing lane, PFFSpI	0.0 %

Percent Time-Spent-Following with Passing Lane

Downstream length of two-lane highway within effective length of passing lane for percent time-spent-following, Lde	- mi
Length of two-lane highway downstream of effective length of the passing lane for percent time-spent-following, Ld	- mi
Adj. factor for the effect of passing lane on percent time-spent-following, fpl	-
Percent time-spent-following including passing lane, PTSfpI	- %

Level of Service and Other Performance Measures with Passing Lane

Level of service including passing lane, LOSpl	A
Peak 15-min total travel time, TT15	- veh-h

Bicycle Level of Service

Direction Analysis(d) Opposing (o)  
 PCE for trucks, ET 1.0 1.0  
 PCE for RVs, ER 1.0 1.0  
 Heavy-vehicle adjustment factor, FHV 1.000 1.000  
 Grade adjustment factor, (note-1) fg 1.00 1.00  
 Directional flow rate, (note-2) vi 890 pc/h 2122 pc/h  
 Base percent time-spent-following, (note-4) BFTSfd 79.7 %  
 Adjustment for no-passing zones, fnp 14.3 %  
 Percent time-spent-following, PFSfd 83.9 %

Level of Service and Other Performance Measures

Level of service, LOS F  
 Volume to capacity ratio, v/c 0.52  
 Peak 15-min vehicle-miles of travel, VMT15 223 veh-mi  
 Peak 15-min vehicle-miles of travel, VMT60 890 veh-mi  
 Peak 15-min total travel time, TTI15 8.5 veh-h  
 Capacity from ATS, CdATS 1700 veh/h  
 Capacity from PTSF, CdPTSF 1700 veh/h  
 Directional Capacity 1700 veh/h

Passing Lane Analysis

Total length of analysis segment, Lt 1.0 mi  
 Length of two-lane highway upstream of the passing lane, Lu - mi  
 Length of passing lane including tapers, Lpl - mi  
 Average travel speed, ATSD (from above) 26.1 mi/h  
 Percent time-spent-following, PTFSD (from above) 83.9 %  
 Level of service, LOSD (from above) F

Average Travel Speed with Passing Lane

Downstream length of two-lane highway within effective length of passing lane for average travel speed, Lde - mi  
 Length of two-lane highway downstream of effective length of the passing lane for average travel speed, Ld - mi  
 Adj. factor for the effect of passing lane on average speed, fpl -  
 Average travel speed including passing lane, ATSpI -  
 Percent free flow speed including passing lane, PFFSpI 0.0 %

Percent Time-Spent-Following with Passing Lane

Downstream length of two-lane highway within effective length of passing lane for percent time-spent-following, Lde - mi  
 Length of two-lane highway downstream of effective length of the passing lane for percent time-spent-following, Ld - mi  
 Adj. factor for the effect of passing lane on percent time-spent-following, fpl -  
 Percent time-spent-following including passing lane, PTFSpI - %

Level of Service and Other Performance Measures with Passing Lane  
 Level of service including passing lane, LOSpl A  
 Peak 15-min total travel time, TTI15 - veh-h  
 Bicycle Level of Service

Fax:

Directional Two-Lane Highway Segment Analysis

Analyst Lauren Davini  
 Agency/Co. Napa County  
 Date Performed 9/6/15  
 Analysis time Period Weekday PM peak  
 Highway Silverado Trail  
 From/To 0.5mi N/o & S/o 4059 SilvTrail  
 Jurisdiction County of Napa  
 Analysis Year 2015  
 Description NB Future

Input Data

Highway Class Class 2 Peak hour factor, PHF 1.00  
 Shoulder width 7.0 ft % Trucks and buses 5 %  
 Lane width 12.0 ft % Trucks crawling 0.0 %  
 Segment length 1.0 mi Truck crawl speed 0.0 mi/hr  
 Terrain type Level 2 % Recreational vehicles 2 %  
 Grade: Length mi % No-passing zones 100 %  
 Up/down % Access point density 15 /mi

Analysis direction volume, Vd 890 veh/h  
 Opposing direction volume, Vo 2122 veh/h

Average Travel Speed

Direction Analysis(d) Opposing (o)  
 PCE for trucks, ET 2.0\* 2.0\*  
 PCE for RVs, ER 1.0 1.0  
 Heavy-vehicle adj. factor, (note-5) FHV 0.952 0.952  
 Grade adj. factor, (note-1) fg 1.00 1.00  
 Directional flow rate, (note-2) vi 935 pc/h 2229 pc/h

Free-Flow Speed from Field Measurement:

Field measured speed, (note-3) S FM - mi/h  
 Observed total demand, (note-3) V - veh/h  
 Estimated Free-Flow Speed: 55.0 mi/h  
 Base free-flow speed, (note-3) BFFS 0.0 mi/h  
 Adj. for lane and shoulder width, (note-3) fLS 3.8 mi/h  
 Adj. for access point density, (note-3) fA 51.3 mi/h

Free-flow speed, FFSd

Adjustment for no-passing zones, fnp 0.6 mi/h  
 Average travel speed, ATSD 26.1 mi/h  
 Percent Free Flow Speed, PFFS 51.0 %

Posted speed limit, Sp 55  
 Percent of segment with occupied on-highway parking 0  
 Pavement rating, P 3  
 Flow rate in outside lane, VOL 890.0  
 Effective width of outside lane, We 26.00  
 Effective speed factor, St 4.79  
 Bicycle LOS Score, BLOS 3.12  
 Bicycle LOS C

Notes:

- Note that the adjustment factor for level terrain is 1.00, as level terrain is one of the base conditions. For the purpose of grade adjustment, specific downgrade segments are treated as level terrain.
- If  $v_i$  (vd or vo)  $\geq$  1,700 pc/h, terminate analysis-the LOS is F.
- For the analysis direction only and for  $v > 200$  veh/h.
- For the analysis direction only.
- Use alternative Exhibit 15-14 if some trucks operate at crawl speeds on a specific downgrade.

\* These items have been entered or edited to override calculated value

Phone: Fax:  
 E-Mail:

Directional Two-Lane Highway Segment Analysis

Analyst Lauren Davini  
 Agency/Co. Napa County  
 Date Performed 9/8/15  
 Analysis Time Period Weekday PM Peak  
 Highway Silverado Trail  
 From/To 0.5mi N/o & S/o 4059 SilvrTrail  
 Jurisdiction County of Napa  
 Analysis Year 2015  
 Description SB Future

Input Data

Highway class Class 2  
 Shoulder width 7.0 ft  
 Lane width 12.0 ft  
 Segment length 1.0 mi  
 Terrain type Level  
 Grade: Length - mi  
 Up/down - %  
 Peak hour factor, PHF 1.00  
 % Trucks and buses 5 %  
 % Trucks crawling 0.0 %  
 Truck crawl speed 0.0 mi/hr  
 % Recreational vehicles 2 %  
 % No-passing zones 100 %  
 Access point density 12 /mi

Analysis direction volume, Vd 2122 veh/h  
 Opposing direction volume, Vo 890 veh/h

Average Travel Speed

Direction Analysis(d) Opposing (o)  
 PCE for trucks, ET 2.0\*  
 PCE for RVs, ER 1.0  
 Heavy-vehicle adj. factor, (note-5) fHV 0.952  
 Grade adj. factor, (note-1) fg 1.00  
 Directional flow rate, (note-2) vi 2229 pc/h  
 Free-Flow Speed from Field Measurement: - mi/h  
 Field measured speed, (note-3) S FM - veh/h  
 Observed total demand, (note-3) V -  
 Estimated Free-Flow Speed: 55.0 mi/h  
 Base free-flow speed, (note-3) BFBS 0.0 mi/h  
 Adj. for lane and shoulder width, (note-3) fLS 3.0 mi/h  
 Adj. for access point density, (note-3) fA 935 pc/h

Free-flow speed, FFSd 52.0 mi/h  
 Adjustment for no-passing zones, fnp 1.2 mi/h  
 Average travel speed, ATSD 26.3 mi/h  
 Percent Free Flow Speed, PFFS 50.5 %

Posted speed limit, SP  
 Percent of segment with occupied on-highway parking  
 Pavement rating, P  
 Flow rate in outside lane, vOL  
 Effective width of outside lane, We  
 Effective speed factor, St  
 Bicycle LOS Score, BLOS  
 Bicycle LOS

2122.0  
 0  
 3  
 2122.0  
 26.00  
 4.79  
 3.56  
 D

Notes:

1. Note that the adjustment factor for level terrain is 1.00, as level terrain is one of the base conditions. For the purpose of grade adjustment, specific downgrade segments are treated as level terrain.
2. If  $v_i (v_d \text{ or } v_o) >= 1,700$  pc/h, terminate analysis-the LOS is F.
3. For the analysis direction only and for  $v > 200$  veh/h.
4. For the analysis direction only.
5. Use alternative Exhibit 15-14 if some trucks operate at crawl speeds on a specific downgrade.

\* These items have been entered or edited to override calculated value

Percent Time-Spent-Following	
Direction	Analysis(d)
PCE for trucks, ET	1.0
PCE for RVs, ER	1.0
Heavy-vehicle adjustment factor, fHV	1.000
Grade adjustment factor, (note-1) fg	1.00
Directional flow rate, (note-2) vi	2122 pc/h
Base percent time-spent-following, (note-4) BPTSfd	93.5 %
Adjustment for no-passing zones, fnp	14.3
Percent time-spent-following, PTFsd	100.0 %

Level of Service and Other Performance Measures	
Level of service, LOS	F
Volume to capacity ratio, v/c	1.25
Peak 15-min vehicle-miles of travel, VMT15	531 veh-mi
Peak-hour vehicle-miles of travel, VMT60	2122 veh-mi
Peak 15-min total travel time, TTT15	20.2 veh-h
Capacity from AIS, CdAIS	0 veh/h
Capacity from PTFs, CdPTSF	1700 veh/h
Directional Capacity	1700 veh/h

Passing Lane Analysis	
Total length of analysis segment, Lt	1.0 mi
Length of two-lane highway upstream of the passing lane, Lu	- mi
Length of passing lane including tapers, Lpl	- mi
Average travel speed, ATSD (from above)	26.3 mi/h
Percent time-spent-following, PTFsd (from above)	100.0
Level of service, LOSd (from above)	F

Average Travel Speed with Passing Lane	
Downstream length of two-lane highway within effective length of passing lane for average travel speed, Lde	- mi
Length of two-lane highway downstream of effective length of the passing lane for average travel speed, Ld	- mi
Adj. factor for the effect of passing lane on average speed, fpl	-
Average travel speed including passing lane, ATSP1	-
Percent free flow speed including passing lane, FFFSP1	0.0 %

Percent Time-Spent-Following with Passing Lane	
Downstream length of two-lane highway within effective length of passing lane for percent time-spent-following, Lde	- mi
Length of two-lane highway downstream of effective length of the passing lane for percent time-spent-following, Ld	- mi
Adj. factor for the effect of passing lane on percent time-spent-following, fpl	-
Percent time-spent-following including passing lane, PTFSP1	- %

Level of Service and Other Performance Measures with Passing Lane	
Level of service including passing lane, LOSpl	A
Peak 15-min total travel time, TTT15	- veh-h

Bicycle Level of Service

Direction  
 PCE for trucks, ET 1.0  
 PCE for RVs, ER 1.0  
 Heavy-vehicle adjustment factor, fHV 1.000  
 Grade adjustment factor, (note-1) fg 1.00  
 Directional flow rate, (note-2) vi 1180 pc/h  
 Base percent time-spent-following, (note-4) BPTSFd 80.9 %  
 Adjustment for no-passing zones, fnp 17.4 %  
 Percent time-spent-following, PTFSD 91.1 %

Analysis (d)  
 1.0  
 1.0  
 1.000  
 1.00  
 1180 pc/h  
 BPTSFd 80.9 %  
 17.4 %  
 91.1 %

Opposing (o)  
 1.0  
 1.0  
 1.000  
 1.00  
 832 pc/h  
 %  
 %

Level of Service and Other Performance Measures

Level of service, LOS E  
 Volume to capacity ratio, v/c 0.69  
 Peak 15-min vehicle-miles of travel, VMT15 295 veh-mi  
 Peak-hour vehicle-miles of travel, VMT60 1180 veh-mi  
 Peak 15-min total travel time, TTT15 8.8 veh-h  
 Capacity from ATS, CdATS 0 veh/h  
 Capacity from PTFd, CdPTFd 1700 veh/h  
 Directional Capacity 1700 veh/h

Phone:  
 E-Mail:

Fax:

Directional Two-Lane Highway Segment Analysis

Analyst Lauren Davini  
 Agency/Co. Napa County  
 Date Performed 9/8/15  
 Analysis Time Period Weekend Midday Peak  
 Highway Silverado Trail  
 From/To 0.5mi N/O & S/O 4059 SilvTrail  
 Jurisdiction County of Napa  
 Analysis Year 2015  
 Description NB Future

Input Data

Highway class Class 2  
 Shoulder width 7.0 ft  
 Lane width 12.0 ft  
 Segment length 1.0 mi  
 Terrain type Level  
 Grade: Length -  
 Up/down -  
 Peak hour factor, PHF 1.00  
 % Trucks and buses 5 %  
 % Trucks crawling 0.0 %  
 Truck crawl speed 0.0 mi/hr  
 % Recreational vehicles 2 %  
 % No-passing zones 100 %  
 Access point density 15 /mi

Analysis direction volume, Vd 1180 veh/h  
 Opposing direction volume, Vo 832 veh/h

Average Travel Speed

Direction Analysis (d) Opposing (o)  
 PCE for trucks, ET 2.0\* 2.0\*  
 PCE for RVs, ER 1.0 1.0  
 Heavy-vehicle adj. factor, (note-5) fHV 0.952 0.952  
 Grade adj. factor, (note-1) fg 1.00 1.00  
 Directional flow rate, (note-2) vi 1239 pc/h 874 pc/h

Free-Flow Speed from Field Measurement:

Field measured speed, (note-3) S FM - mi/h  
 Observed total demand, (note-3) V - veh/h  
 Estimated Free-Flow Speed: 55.0 mi/h  
 Base free-flow speed, (note-3) BFFS 0.0 mi/h  
 Adj. for lane and shoulder width, (note-3) fLS 3.8 mi/h  
 Adj. for access point density, (note-3) fA 51.3 mi/h

Free-flow speed, FFSd

Adjustment for no-passing zones, fnp 1.2 mi/h  
 Average travel speed, ATSD 33.6 mi/h  
 Percent Free Flow Speed, PFFS 65.6 %

Passing Lane Analysis

Total length of analysis segment, Lt 1.0 mi  
 Length of two-lane highway upstream of the passing lane, Lu - mi  
 Length of passing lane including tapers, Lpl - mi  
 Average travel speed, ATSD (from above) 33.6 mi/h  
 Percent time-spent-following, PTFSD (from above) 91.1 %  
 Level of service, LOSd (from above) E

Average Travel Speed with Passing Lane

Downstream length of two-lane highway within effective length of passing lane for average travel speed, Lde - mi  
 Length of two-lane highway downstream of effective length of the passing lane for average travel speed, Ld - mi  
 Adj. factor for the effect of passing lane on average speed, fpl -  
 Average travel speed including passing lane, ATSpL -  
 Percent free flow speed including passing lane, PFFSpL 0.0 %

Percent Time-Spent-Following with Passing Lane

Downstream length of two-lane highway within effective length of passing lane for percent time-spent-following, Lde - mi  
 Length of two-lane highway downstream of effective length of the passing lane for percent time-spent-following, Ld - mi  
 Adj. factor for the effect of passing lane on percent time-spent-following, fpl -  
 Percent time-spent-following including passing lane, PTFSpL - %

Level of Service and Other Performance Measures with Passing Lane

Level of service including passing lane, LOSpl A  
 Peak 15-min total travel time, TTT15 - veh-h

Bicycle Level of Service

Posted speed limit, Sp 55  
 Percent of segment with occupied on-highway parking 0  
 Pavement rating, P 3  
 Flow rate in outside lane, VOL 1180.0  
 Effective width of outside lane, We 26.00  
 Effective speed factor, St 4.79  
 Bicycle LOS Score, BLOS 3.26  
 Bicycle LOS C

Phone: E-Mail: Fax:

Directional Two-Lane Highway Segment Analysis

Analyst Lauren Davini  
 Agency/Co. Napa County  
 Date Performed 9/8/15  
 Analysis Time Period Weekend Midday Peak  
 Highway Silverado Trail  
 From/To 0.5mi N/o & S/o 4059 SilvTrail  
 Jurisdiction County of Napa  
 Analysis Year 2015  
 Description SB Future

Input Data

Highway class Class 2  
 Shoulder width 7.0 ft  
 Lane width 12.0 ft  
 Segment length 1.0 mi  
 Terrain type Level  
 Grade: Length -  
 Up/down -

Peak hour factor, PHF 1.00  
 % Trucks and buses 5  
 % Trucks crawling 0.0  
 Truck crawl speed 0.0 mi/hr  
 % Recreational vehicles 2  
 % No-passing zones 100  
 Access point density 12 /mi

Analysis direction volume, Vd 832 veh/h  
 Opposing direction volume, Vo 1180 veh/h

Average Travel Speed

Direction	Analysis(d)	Opposing (o)
PCE for trucks, ET	2.0*	2.0*
PCE for RVs, ER	1.0	1.0
Heavy-vehicle adj. factor, (note-5) fHV	0.952	0.952
Grade adj. factor, (note-1) fg	1.00	1.00
Directional flow rate, (note-2) vi	874 pc/h	1239 pc/h

Free-Flow Speed from Field Measurement:

Field measured speed, (note-3) S FM - mi/h  
 Observed total demand, (note-3) V - veh/h  
 Estimated Free-Flow Speed: -  
 Base free-flow speed, (note-3) BFFS 55.0 mi/h  
 Adj. for lane and shoulder width, (note-3) fLS 0.0 mi/h  
 Adj. for access point density, (note-3) fA 3.0 mi/h  
 Free-flow speed, FFSd 52.0 mi/h  
 Adjustment for no-passing zones, fnp 1.0 mi/h  
 Average travel speed, ATSD 34.6 mi/h  
 Percent Free Flow Speed, PFFS 66.6 %

Notes:

1. Note that the adjustment factor for level terrain is 1.00, as level terrain is one of the base conditions. For the purpose of grade adjustment, specific downgrade segments are treated as level terrain.
2. If vi (vd or vo) >= 1,700 pc/h, terminate analysis-the LOS is F.
3. For the analysis direction only and for v>200 veh/h.
4. For the analysis direction only.
5. Use alternative Exhibit 15-14 if some trucks operate at crawl speeds on a specific downgrade.

\* These items have been entered or edited to override calculated value

Posted speed limit, Sp 55  
 Percent of segment with occupied on-highway parking 0  
 Pavement rating, P 3  
 Flow rate in outside lane, VOL 832.0  
 Effective width of outside lane, We 26.00  
 Effective speed factor, St 4.79  
 Bicycle LOS Score, BLOS 3.08  
 Bicycle LOS C

- Notes:  
 1. Note that the adjustment factor for level terrain is 1.00, as level terrain is one of the base conditions. For the purpose of grade adjustment, specific downgrade segments are treated as level terrain.  
 2. If  $v_i (vd \text{ or } vo) \geq 1,700 \text{ pc/h}$ , terminate analysis-the LOS is F.  
 3. For the analysis direction only and for  $v > 200 \text{ veh/h}$ .  
 4. For the analysis direction only.  
 5. Use alternative Exhibit 15-14 if some trucks operate at crawl speeds on a specific downgrade.

\* These items have been entered or edited to override calculated value

Percent Time-Spent-Following	
Direction	Analysis(d)
PCE for trucks, ET	1.0
PCE for RVs, ER	1.0
Heavy-vehicle adjustment factor, FHV	1.000
Grade adjustment factor, (note-1), fg	1.00
Directional flow rate, (note-2), vi	832 pc/h
Base percent time-spent-following, (note-4), BTRFsd	75.0 %
Adjustment for no-passing zones, fnp	17.4
Percent time-spent-following, PTRFsd	82.2 %

Level of Service and Other Performance Measures	
Level of service, LOS	D
Volume to capacity ratio, v/c	0.49
Peak 15-min vehicle-miles of travel, VMT15	208 veh-mi
Peak-hour vehicle-miles of travel, VMT60	832 veh-mi
Peak 15-min total travel time, TT15	6.0 veh-h
Capacity from ATS, CdATS	1700 veh/h
Capacity from PTSF, CdPTSF	1700 veh/h
Directional Capacity	1700 veh/h

Passing Lane Analysis	
Total length of analysis segment, Lt	1.0 mi
Length of two-lane highway upstream of the passing lane, Lu	- mi
Length of passing lane including tapers, Lpl	- mi
Average travel speed, ATSD (from above)	34.6 mi/h
Percent time-spent-following, PTRFpd (from above)	82.2 %
Level of service, LOSd (from above)	D

Average Travel Speed with Passing Lane	
Downstream length of two-lane highway within effective length of passing lane for average travel speed, Lde	- mi
Length of two-lane highway downstream of effective length of the passing lane for average travel speed, Ld	- mi
Adj. factor for the effect of passing lane on average speed, fpl	-
Average travel speed including passing lane, ATSpI	-
Percent free flow speed including passing lane, PFFSpI	0.0 %

Percent Time-Spent-Following with Passing Lane	
Downstream length of two-lane highway within effective length of passing lane for percent time-spent-following, Lde	- mi
Length of two-lane highway downstream of effective length of the passing lane for percent time-spent-following, Ld	- mi
Adj. factor for the effect of passing lane on percent time-spent-following, fpl	-
Percent time-spent-following including passing lane, PTSFpl	- %

Level of Service and Other Performance Measures with Passing Lane	
Level of service including passing lane, LOSpl	A
Peak 15-min total travel time, TT15	- veh-h

Bicycle Level of Service

Direction	Analysis(d)	Opposing (o)
PCE for trucks, ET	1.0	1.0
PCE for RVs, ER	1.0	1.0
Heavy-vehicle adjustment factor, fHV	1.000	1.000
Grade adjustment factor, (note-1) fg	1.00	1.00
Directional flow rate, (note-2) vi	941	2243
Base percent time-spent-following, (note-4) BPTSfd	81.1	%
Adjustment for no-passing zones, fnp	14.3	%
Percent time-spent-following, PTSfd	85.3	%

Level of Service and Other Performance Measures

Level of service, LOS	F
Volume to capacity ratio, v/c	0.55
Peak 15-min vehicle-miles of travel, VMT15	235
Peak-hour vehicle-miles of travel, VMT60	894
Peak 15-min total travel time, TTT15	9.5
Capacity from AFS, CdAFS	1700
Capacity from PTSF, CdPTSF	1700
Directional Capacity	1700

Phone:  
E-Mail:

Directional Two-Lane Highway Segment Analysis

Analyst: Lauren Davini  
Agency/Co.: Napa County  
Date Performed: 9/8/15  
Analysis Time Period: Weekday PM Peak  
Highway: Silverado Trail  
From/To: 0.5mi N/o & S/o 4059 SilvTrail  
Jurisdiction: County of Napa  
Analysis Year: 2015  
Description: NB Future + Project

Fax:

Percent Time-Spent-Following

Direction	Analysis(d)	Opposing (o)
PCE for trucks, ET	1.0	1.0
PCE for RVs, ER	1.0	1.0
Heavy-vehicle adjustment factor, fHV	1.000	1.000
Grade adjustment factor, (note-1) fg	1.00	1.00
Directional flow rate, (note-2) vi	941	2243
Base percent time-spent-following, (note-4) BPTSfd	81.1	%
Adjustment for no-passing zones, fnp	14.3	%
Percent time-spent-following, PTSfd	85.3	%

Input Data

Highway class	Class 2	Peak hour factor, PHF	0.95
Shoulder width	7.0 ft	% Trucks and buses	5
Lane width	12.0 ft	% Trucks crawling	0.0
Segment length	1.0 mi	Truck crawl speed	mi/hr
Terrain type	Level	% Recreational vehicles	2
Grade:	mi	% No-passing zones	100
Up/down	%	Access point density	15 /mi

Analysis direction volume, Vd 894 veh/h  
Opposing direction volume, Vo 2131 veh/h

Average Travel Speed

Direction	Analysis(d)	Opposing (o)
PCE for trucks, ET	2.0*	2.0*
PCE for RVs, ER	1.0	1.0
Heavy-vehicle adj. factor, (note-5) fHV	0.952	0.952
Grade adj factor, (note-1) fg	1.00	1.00
Directional flow rate, (note-2) vi	989	2356

Free-Flow Speed from Field Measurement:

Field measured speed, (note-3) S FM	-	mi/h
Observed total demand, (note-3) V	-	veh/h
Estimated Free-Flow Speed:		
Base free-flow speed, (note-3) BFFS	55.0	mi/h
Adj. for lane and shoulder width, (note-3) fLS	0.0	mi/h
Adj. for access point density, (note-3) fA	3.8	mi/h
Free-flow speed, FFSd	51.3	mi/h

Adjustment for no-passing zones, fnp 0.6  
Average travel speed, ATSpd 24.7  
Percent Free Flow Speed, PFFS 48.3

Passing Lane Analysis

Total length of analysis segment, Lt	mi
Length of two-lane highway upstream of the passing lane, Lu	1.0
Length of passing lane including tapers, Lpl	mi
Average travel speed, ATSpd (from above)	24.7
Percent time-spent-following, PTSfd (from above)	85.3
Level of service, LOSd (from above)	F

Average Travel Speed with Passing Lane

Downstream length of two-lane highway within effective length of passing lane for average travel speed, Lde	mi
Length of two-lane highway downstream of effective length of the passing lane for average travel speed, Ld	mi
Adj. factor for the effect of passing lane on average speed, fpl	-
Average travel speed including passing lane, ATSpI	-
Percent free flow speed including passing lane, PFFSpI	0.0

Percent Time-Spent-Following with Passing Lane

Downstream length of two-lane highway within effective length of passing lane for percent time-spent-following, Lde	mi
Length of two-lane highway downstream of effective length of the passing lane for percent time-spent-following, Ld	mi
Adj. factor for the effect of passing lane on percent time-spent-following, fpl	-
Percent time-spent-following including passing lane, PTFSpI	-

Level of Service and Other Performance Measures with Passing Lane

Level of service including passing lane, LOSpl A  
Peak 15-min total travel time, TTT15 - veh-h

Bicycle Level of Service

Posted speed limit, Sp 55  
 Percent of segment with occupied on-highway parking 0  
 Pavement rating, P 3  
 Flow rate in outside lane, VOL 941.1  
 Effective width of outside lane, We 26.00  
 Effective speed factor, St 4.79  
 Bicycle LOS Score, BLOS 3.14  
 Bicycle LOS C

Notes:

- Note that the adjustment factor for level terrain is 1.00, as level terrain is one of the base conditions. For the purpose of grade adjustment, specific downgrade segments are treated as level terrain.
- If  $v_1$  (vd or vo)  $\geq$  1,700 pc/h, terminate analysis-the LOS is F.
- For the analysis direction only and for  $v > 200$  veh/h.
- For the analysis direction only.
- Use alternative Exhibit 15-14 if some trucks operate at crawl speeds on a specific downgrade.

\* These items have been entered or edited to override calculated value

Phone:  
 E-Mail:

Fax:

Directional Two-Lane Highway Segment Analysis

Analyst Lauren Davini  
 Agency/Co. Napa County  
 Date Performed 9/8/15  
 Analysis Time Period Weekday PM Peak  
 Highway Silverado Trail  
 From/To 0.5mi N/o & S/o 4059 SilvTrail  
 Jurisdiction County of Napa  
 Analysis Year 2015  
 Description SB Future + Project

Input Data

Highway class	Class 2	Peak hour factor, PHF	0.95
Shoulder width	7.0 ft	% Trucks and buses	5 %
Lane width	12.0 ft	% Trucks crawling	0.0 %
Segment length	1.0 mi	Truck crawl speed	2 mi/hr
Terrain type	Level	% Recreational vehicles	2 %
Grade:	Length -	% No-passing zones	100 %
	Up/down -	Access point density	12 /mi

Analysis direction volume, Vd 2131 veh/h  
 Opposing direction volume, Vo 894 veh/h

Average Travel Speed

Direction	Analysis(d)	Opposing (o)
PCE for trucks, ET	2.0*	2.0*
PCE for RVs, ER	1.0	1.0
Heavy-vehicle adj. factor, (note-5) fHV	0.952	0.952
Grade adj. factor, (note-1) fg	1.00	1.00
Directional flow rate, (note-2) vl	2356 pc/h	989 pc/h

Free-Flow Speed from Field Measurement:

Field measured speed, (note-3) S <sub>FM</sub>	-	mi/h
Observed total demand, (note-3) V	-	veh/h
Estimated Free-Flow Speed:		
Base free-flow speed, (note-3) BFFS	55.0	mi/h
Adj. for lane and shoulder width, (note-3) fLS	0.0	mi/h
Adj. for access point density, (note-3) fA	3.0	mi/h
Free-flow speed, FFS	52.0	mi/h
Adjustment for no-passing zones, fnp	1.1	mi/h
Average travel speed, ATSD	24.9	mi/h
Percent Free Flow Speed, PFFS	47.9	%

Posted speed limit, Sp 55  
 Percent of segment with occupied on-highway parking 0  
 Pavement rating, P 3  
 Flow rate in outside lane, vOL 2243.2  
 Effective width of outside lane, We 26.00  
 Effective speed factor, St 4.79  
 Bicycle LOS Score, BLOS 3.58  
 Bicycle LOS D

Notes:  
 1. Note that the adjustment factor for level terrain is 1.00, as level terrain is one of the base conditions. For the purpose of grade adjustment, specific downgrade segments are treated as level terrain.  
 2. If vi (vd or vo) >= 1,700 pc/h, terminate analysis-the LOS is F.  
 3. For the analysis direction only and for v>200 veh/h.  
 4. For the analysis direction only.  
 5. Use alternative Exhibit 15-14 if some trucks operate at crawl speeds on a specific downgrade.

\* These items have been entered or edited to override calculated value

Percent Time-Spent-Following	
Direction	Analysis(d)
PCE for trucks, ET	1.0
PCE for RVs, ER	1.0
Heavy-vehicle adjustment factor, fHV	1.000
Grade adjustment factor, (note-1) fg	1.00
Directional flow rate, (note-2) vi	2243 pc/h
Base percent time-spent-following, (note-4) BPTSfd	94.5
Adjustment for non-passing zones, fnp	14.3
Percent time-spent-following, PTSfd	100.0 %

Level of Service and Other Performance Measures	
Level of service, LOS	F
Volume to capacity ratio, v/c	1.32
Peak 15-min vehicle-miles of travel, VMT15	561 veh-mi
Peak-hour vehicle-miles of travel, VMT60	2131 veh-mi
Peak 15-min total travel time, TT15	22.5 veh-h
Capacity from ATS, CGATS	0 veh/h
Capacity from PTSF, CdPTSF	1700 veh/h
Directional Capacity	1700 veh/h

Passing Lane Analysis	
Total length of analysis segment, Lt	1.0 mi
Length of two-lane highway upstream of the passing lane, Lu	- mi
Length of passing lane including tapers, Lpl	- mi
Average travel speed, ATSD (from above)	24.9 mi/h
Percent time-spent-following, PTSfd (from above)	100.0
Level of service, LOSd (from above)	F

Average Travel Speed with Passing Lane	
Downstream length of two-lane highway within effective length of passing lane for average travel speed, Lde	- mi
Length of two-lane highway downstream of effective length of the passing lane for average travel speed, Ld	- mi
Adj. factor for the effect of passing lane on average speed, fpl	-
Average travel speed including passing lane, ATSpI	-
Percent free flow speed including passing lane, PFFSpI	0.0 %

Percent Time-Spent-Following with Passing Lane	
Downstream length of two-lane highway within effective length of passing lane for percent time-spent-following, Lde	- mi
Length of two-lane highway downstream of effective length of the passing lane for percent time-spent-following, Ld	- mi
Adj. factor for the effect of passing lane on percent time-spent-following, fpl	-
Percent time-spent-following including passing lane, PTSFpl	- %

Level of Service and Other Performance Measures with Passing Lane	
Level of service including passing lane, LOSpl	A
Peak 15-min total travel time, TT15	- veh-h

Bicycle Level of Service	
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Phone:  
E-Mail:

Fax:

Directional Two-Lane Highway Segment Analysis

Analyst Lauren Davini  
Agency/Co. Napa County  
Date Performed 9/8/15  
Analysis Time Period Weekend Midday Peak  
Highway Silverado Trail  
From/To 0.5mi N/O & S/O 4059 SilvTrail  
Jurisdiction County of Napa  
Analysis Year 2015  
Description NB Future + Project

Input Data

Highway class	Class 2	Peak hour factor, PHF	1.00
Shoulder width	7.0 ft	% Trucks and buses	5
Lane width	12.0 ft	% Trucks crawling	0.0
Segment length	1.0 mi	Truck crawl speed	0.0 mi/hr
Terrain type	Level	% Recreational vehicles	2
Grade:		% No-passing zones	100
Up/down		Access point density	15 /mi

Analysis direction volume, Vd 1189 veh/h  
Opposing direction volume, Vo 842 veh/h

Average Travel Speed

Direction	Analysis(d)	Opposing (o)
PCE for trucks, ET	2.0*	2.0*
PCE for RVs, ER	1.0	1.0
Heavy-vehicle adj. factor, (note-5) fHV	0.952	0.952
Grade adj. factor, (note-1) fg	1.00	1.00
Directional flow rate, (note-2) vi	1249 pc/h	884 pc/h

Free-Flow Speed from Field Measurement:

Field measured speed, (note-3) S <sub>FM</sub>	-	mi/h
Observed total demand, (note-3) V	-	veh/h
Estimated Free-Flow Speed:		
Base free-flow speed, (note-3) BFFS	55.0	mi/h
Adj. for lane and shoulder width, (note-3) fLS	0.0	mi/h
Adj. for access point density, (note-3) fA	3.8	mi/h
Free-flow speed, FFSd	51.3	mi/h

Adjustment for no-passing zones, fnp	1.2	mi/h
Average travel speed, ATSD	33.5	mi/h
Percent Free Flow Speed, PFFS	65.3	%

Percent Time-Spent-Following

Direction	Analysis(d)	Opposing (o)
PCE for trucks, ET	1.0	1.0
PCE for RVs, ER	1.0	1.0
Heavy-vehicle adjustment factor, fHV	1.000	1.000
Grade adjustment factor, (note-1) fg	1.00	1.00
Directional flow rate, (note-2) vi	1189 pc/h	842 pc/h
Base percent time-spent-following, (note-4) BPTSfd	81.1	%
Adjustment for no-passing zones, fnp	17.1	%
Percent time-spent-following, PTFSD	91.1	%

Level of Service and Other Performance Measures

Level of service, LOS	E
Volume to capacity ratio, v/c	0.70
Peak 15-min vehicle-miles of travel, VMT15	297 veh-mi
Peak 15-min vehicle-miles of travel, VMT60	1189 veh-mi
Peak 15-min total travel time, TTT15	8.9 veh-h
Capacity from ATIS, CdATS	0 veh/h
Capacity from PTSF, CdPTSF	1700 veh/h
Directional Capacity	1700 veh/h

Passing Lane Analysis

Total length of analysis segment, Lt	1.0	mi
Length of two-lane highway upstream of the passing lane, Lu	-	mi
Length of passing lane including tapers, Lpl	-	mi
Average travel speed, ATSD (from above)	33.5	mi/h
Percent time-spent-following, PTFSD (from above)	91.1	%
Level of service, LOSd (from above)	E	

Average Travel Speed with Passing Lane

Downstream length of two-lane highway within effective length of passing lane for average travel speed, Lde	-	mi
Length of two-lane highway downstream of effective length of the passing lane for average travel speed, Ld	-	mi
Adj. factor for the effect of passing lane on average speed, fpl	-	
Average travel speed including passing lane, ATSpI	-	%
Percent free flow speed including passing lane, PFFSpI	0.0	%

Percent Time-Spent-Following with Passing Lane

Downstream length of two-lane highway within effective length of passing lane for percent time-spent-following, Lde	-	mi
Length of two-lane highway downstream of effective length of the passing lane for percent time-spent-following, Ld	-	mi
Adj. factor for the effect of passing lane on percent time-spent-following, fpl	-	
Percent time-spent-following including passing lane, PTFSpI	-	%

Level of Service and Other Performance Measures with Passing Lane

Level of service including passing lane, LOSpl	A	
Peak 15-min total travel time, TTT15	-	veh-h

Bicycle Level of Service

Posted speed limit, Sp 55  
 Percent of segment with occupied on-highway parking 0  
 Pavement rating, p 3  
 Flow rate in outside lane, VOL 1189.0  
 Effective width of outside lane, We 26.00  
 Effective speed factor, St 4.79  
 Bicycle LOS Score, BLOS 3.26  
 Bicycle LOS C

Notes:

1. Note that the adjustment factor for level terrain is 1.00, as level terrain is one of the base conditions. For the purpose of grade adjustment, specific downgrade segments are treated as level terrain.
2. If vi (vd or vo) >= 1,700 pc/h, terminate analysis-the LOS is F.
3. For the analysis direction only and for v>200 veh/h.
4. For the analysis direction only.
5. Use alternative Exhibit 15-14 if some trucks operate at crawl speeds on a specific downgrade.

\* These items have been entered or edited to override calculated value

Phone:  
 E-Mail:

Fax:  
 Directional Two-Lane Highway Segment Analysis

Analyst Lauren Davini  
 Agency/Co. Napa County  
 Date Performed 9/8/15  
 Analysis Time Period Weekend Midday Peak  
 Highway Silverado Trail  
 From/To 0.5mi N/o & S/o 4059 SilvTrail  
 Jurisdiction County of Napa  
 Analysis Year 2015  
 Description SB Future + Project

Input Data

Highway class	Class 2	Peak hour factor, PHF	1.00
Shoulder width	7.0 ft	% Trucks and buses	5 %
Lane width	12.0 ft	% Trucks crawling	0.0 %
Segment length	1.0 mi	Truck crawl speed	0.0 mi/hr
Terrain type	Level	% Recreational vehicles	2 %
Grade:	Length -	% No-passing zones	100 %
	Up/down -	Access point density	12 /mi

Analysis direction volume, Vd 842 veh/h  
 Opposing direction volume, Vo 1189 veh/h

Average Travel Speed

Direction	Analysis(d)	Opposing (o)
PCE for trucks, ET	2.0*	2.0*
PCE for RVs, ER	1.0	1.0
Heavy-vehicle adj. factor, (note-5) fHV	0.952	0.952
Grade adj. factor, (note-1) fg	1.00	1.00
Directional flow rate, (note-2) vi	884 pc/h	1249 pc/h

Free-Flow Speed from Field Measurement:

Field measured speed, (note-3) S FM	-	mi/h
Observed total demand, (note-3) V	-	veh/h
Estimated free-flow speed:		
Base free-flow speed, (note-3) BFFS	55.0	mi/h
Adj. for lane and shoulder width, (note-3) fLS	0.0	mi/h
Adj. for access point density, (note-3) fA	3.0	mi/b
Free-flow speed, FFSD	52.0	mi/h
Adjustment for no-passing zones, fnp	1.0	mi/h
Average travel speed, ATSD	34.5	mi/h
Percent Free Flow Speed, PFFS	66.3	%

Posted speed limit, Sp 55  
 Percent of segment with occupied on-highway parking 0  
 Pavement rating, P 3  
 Flow rate in outside lane, VOL 842.0  
 Effective width of outside lane, We 26.00  
 Effective speed factor, St 4.79  
 Bicycle LOS Score, BLOS 3.09  
 Bicycle LOS C

Notes:  
 1. Note that the adjustment factor for level terrain is 1.00, as level terrain is one of the base conditions. For the purpose of grade adjustment, specific downgrade segments are treated as level terrain.  
 2. If  $v_i$  (vd or vo)  $\geq 1,700$  pc/h, terminate analysis-the LOS is F.  
 3. For the analysis direction only and for  $v > 200$  veh/h.  
 4. For the analysis direction only.  
 5. Use alternative Exhibit 15-14 if some trucks operate at crawl speeds on a specific downgrade.

\* These items have been entered or edited to override calculated value

Percent Time-Spent-Following  
 Direction Analysis (d) Opposing (o)  
 PCE for trucks, ET 1.0 1.0  
 PCE for RVs, ER 1.0 1.0  
 Heavy-vehicle adjustment factor, FHV 1.000 1.000  
 Grade adjustment factor, (note-1) fg 1.00 1.00  
 Directional flow rate, (note-2) vi 842 pc/h 1189 pc/h  
 Base percent time-spent-following, (note-4) BPTSfd 75.3 %  
 Adjustment for no-passing zones, fnp 17.1 %  
 Percent time-spent-following, PTSfd 82.4 %

Level of Service and Other Performance Measures

Level of service, LOS D  
 Volume to capacity ratio, v/c 0.50  
 Peak 15-min vehicle-miles of travel, VMT15 211 veh-mi  
 Peak-hour vehicle-miles of travel, VMT60 842 veh-mi  
 Peak 15-min total travel time, TT15 6.1 veh-h  
 Capacity from ATS, CdRFS 1700 veh/h  
 Capacity from PTSf, CdPFSF 1700 veh/h  
 Directional Capacity 1700 veh/h

Passing Lane Analysis

Total length of analysis segment, Lt 1.0 mi  
 Length of two-lane highway upstream of the passing lane, Lu - mi  
 Length of passing lane including tapers, Lpl - mi  
 Average travel speed, ATSD (from above) 34.5 mi/h  
 Percent time-spent-following, PTFSD (from above) 82.4 %  
 Level of service, LOSD (from above) D

Average Travel Speed with Passing Lane

Downstream length of two-lane highway within effective length of passing lane for average travel speed, Lde - mi  
 Length of two-lane highway downstream of effective length of the passing lane for average travel speed, Ld - mi  
 Adj. factor for the effect of passing lane on average speed, fpl -  
 Average travel speed including passing lane, ATSp1 -  
 Percent free flow speed including passing lane, PFFSp1 0.0 %

Percent Time-Spent-Following with Passing Lane

Downstream length of two-lane highway within effective length of passing lane for percent time-spent-following, Lde - mi  
 Length of two-lane highway downstream of effective length of the passing lane for percent time-spent-following, Ld - mi  
 Adj. factor for the effect of passing lane on percent time-spent-following, fpl -  
 Percent time-spent-following including passing lane, PTFSp1 - %

Level of Service and Other Performance Measures with Passing Lane

Level of service including passing lane, LOSpl A  
 Peak 15-min total travel time, TT15 - veh-h

Bicycle Level of Service

# Appendix C

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## Trip Generation Worksheet



## Winery Traffic Information / Trip Generation Sheet

Sam Jasper Winery

Project Scenario:

### Traffic during a Typical Weekday

Number of FT employees: <u>10</u> x 3.05 one-way trips per employee	=	<u>31</u>	daily trips.
Number of PT employees: <u>0</u> x 1.90 one-way trips per employee	=	<u>0</u>	daily trips.
Average number of weekday visitors: <u>25</u> / 2.6 visitors per vehicle x 2 one-way trips	=	<u>19</u>	daily trips.
Gallons of production: <u>20000</u> / 1,000 x .009 truck trips daily <sup>3</sup> x 2 one-way trips	=	<u>0</u>	daily trips.
<b>Total</b>	<b>=</b>	<b><u>50</u></b>	<b>daily trips.</b>
Number of total weekday trips x .38	=	<u>19</u>	<b>PM peak trips.</b>

### Traffic during a Typical Saturday

Number of FT employees (on Saturdays): <u>10</u> x 3.05 one-way trips per employee	=	<u>31</u>	daily trips.
Number of PT employees (on Saturdays): <u>0</u> x 1.90 one-way trips per employee	=	<u>0</u>	daily trips.
Average number of weekend visitors: <u>25</u> / 2.8 visitors per vehicle x 2 one-way trips	=	<u>18</u>	daily trips.
<b>Total</b>	<b>=</b>	<b><u>48</u></b>	<b>daily trips.</b>
Number of total Saturday trips x .57	=	<u>27</u>	<b>PM peak trips.</b>

### Traffic during a Crush Saturday

Number of FT employees (during crush): <u>10</u> x 3.05 one-way trips per employee	=	<u>31</u>	daily trips.
Number of PT employees (during crush): <u>0</u> x 1.90 one-way trips per employee	=	<u>0</u>	daily trips.
Average number of weekend visitors: <u>25</u> / 2.8 visitors per vehicle x 2 one-way trips	=	<u>18</u>	daily trips.
Gallons of production: <u>20000</u> / 1,000 x .009 truck trips daily x 2 one-way trips	=	<u>0</u>	daily trips.
Avg. annual tons of grape on-haul: <u>150</u> x .11 truck trips daily <sup>4</sup> x 2 one-way trips	=	<u>2</u>	daily trips.
<b>Total</b>	<b>=</b>	<b><u>51</u></b>	<b>daily trips.</b>
Number of total Saturday trips x .57	=	<u>29</u>	<b>PM peak trips.</b>

### Largest Marketing Event- Additional Traffic

Number of event staff (largest event): <u>4</u> x 2 one-way trips per staff person	=	<u>8</u>	trips.
Number of visitors (largest event): <u>50</u> / 2.8 visitors per vehicle x 2 one-way trips	=	<u>36</u>	trips.
Number of special event truck trips (largest event): <u>2</u> x 2 one-way trips	=	<u>4</u>	trips.

<sup>3</sup> Assumes 1.47 materials & supplies trips + 0.8 case goods trips per 1,000 gallons of production / 250 days per year (see *Traffic Information Sheet Addendum* for reference).

<sup>4</sup> Assumes 4 tons per trip / 36 crush days per year (see *Traffic Information Sheet Addendum* for reference).

# Appendix D

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## Left-Turn Lane Warrant

# Napa County Left Turn Lane Warrant Graph

