

“Q”

Applicant Response to Public Comments
After April 20, 2016

Part 4

HCM Unsignalized Intersection Capacity Analysis
 2: Conn Creek Road & Silverado Trail

PM WKDY N-T (NP) Conditions
 12/3/2014



MOVEMENT	EB	WB	NB	SB	EB	WB	NB	SB
Lane Configurations	↑	↑	↑	↑	↑	↑	↑	↑
Sign Control	Stop		Stop		Free		Free	
Grade	0%		0%		0%		0%	
Volume (veh/h)	52	0	80	10	3	5	50	367
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	57	0	87	11	3	5	54	399
Pedestrians								
Lane Width (ft)								
Walking Speed (ft/s)								
Percent Blockage								
Right turn flare (veh)			1					
Median type	None		None					
Median storage (veh)								
Upstream signal (ft)								
pX, platoon unblocked								
vC, conflicting volume	1540	1533	1023	1554	1555	399	1046	399
vC1, stage 1 conf vol								
vC2, stage 2 conf vol								
vCu, unblocked vol	1540	1533	1023	1554	1555	399	1046	399
tC, single (s)	7.1	6.5	6.2	7.1	6.5	6.2	4.1	4.1
tC, 2 stage (s)								
tF (s)	3.5	4.0	3.3	3.5	4.0	3.3	2.2	2.2
p0 queue free %	34	100	70	82	97	99	92	100
cM capacity (veh/h)	85	107	286	60	104	651	665	1160
Direction Lane #	EB 1	WB 1	NB 1	NB 2	SB 1	SB 2		
Volume Total	143	20	54	399	1	1046		
Volume Left	57	11	54	0	1	0		
Volume Right	87	5	0	0	0	45		
cSH	171	89	665	1700	1160	1700		
Volume to Capacity	0.84	0.22	0.08	0.23	0.00	0.62		
Queue Length 95th (ft)	145	20	7	0	0	0		
Control Delay (s)	85.8	56.9	10.9	0.0	8.1	0.0		
Lane LOS	F	F	B		A			
Approach Delay (s)	85.8	56.9	1.3		0.0			
Approach LOS	F	F						
Intersection Summary								
Average Delay			8.4					
Intersection Capacity Utilization			69.2%		ICU Level of Service		C	
Analysis Period (min)			15					

HCM Unsignalized Intersection Capacity Analysis
 2: Conn Creek Road & Silverado Trail

MD WKND N-T (NP) Conditions
 12/3/2014



Movement	EBL	EBT	EBN	WBL	WBT	WBN	NBL	NBT	NBN	SBL	SBT	SBN
Lane Configurations		↔	↔		↔	↔	↔	↔	↔	↔	↔	↔
Sign Control		Stop			Stop			Free			Free	
Grade		0%			0%			0%			0%	
Volume (veh/h)	83	2	78	16	1	8	55	478	7	18	616	50
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	90	2	79	17	1	9	60	520	8	20	670	54
Pedestrians												
Lane Width (ft)												
Walking Speed (ft/s)												
Percent Blockage												
Right turn flare (veh)	1											
Median type	None			None								
Median storage (veh)												
Upstream signal (ft)												
pX, platoon unblocked												
vC, conflicting volume	1384	1383	697	1392	1406	523	724			527		
vC1, stage 1 conf vol												
vC2, stage 2 conf vol												
vCu, unblocked vol	1384	1383	697	1392	1406	523	724			527		
tC, single (s)	7.1	6.5	6.2	7.1	6.5	6.2	4.1			4.1		
tC, 2 stage (s)												
tF (s)	3.5	4.0	3.3	3.5	4.0	3.3	2.2			2.2		
p0 queue free %	18	98	82	81	99	98	93			98		
cM capacity (veh/h)	110	131	441	90	127	554	879			1040		
Direction, Lane #	EB 1	WB 1	NB 1	NB 2	SB 1	SB 2						
Volume Total	172	27	60	527	20	724						
Volume Left	90	17	60	0	20	0						
Volume Right	79	9	0	8	0	54						
cSH	179	125	879	1700	1040	1700						
Volume to Capacity	0.96	0.22	0.07	0.31	0.02	0.43						
Queue Length 95th (ft)	190	20	5	0	1	0						
Control Delay (s)	110.2	41.5	9.4	0.0	8.5	0.0						
Lane LOS	F	E	A		A							
Approach Delay (s)	110.2	41.5	1.0		0.2							
Approach LOS	F	E										
Intersection Summary												
Average Delay			13.6									
Intersection Capacity Utilization			56.9%			ICU Level of Service			B			
Analysis Period (min)			15									



	EBT	EBR	WB1	WB2	NB1	NB2
Lane Configurations	T		T		T	
Sign Control	Free		Free		Stop	
Grade	0%		0%		0%	
Volume (veh/h)	101	204	66	90	51	29
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	110	222	72	98	55	32
Pedestrians						
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)						
Median type	None					
Median storage (veh)						
Upstream signal (ft)						
pX, platoon unblocked						
vC, conflicting volume			332		462	221
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol			332		462	221
tC, single (s)			4.1		6.4	6.2
tC, 2 stage (s)						
tF (s)			2.2		3.5	3.3
p0 queue free %			94		89	96
cM capacity (veh/h)			1228		525	819
Direction - Lane #						
	EB 1	WB 1	NB 1			
Volume Total	332	170	87			
Volume Left	0	72	55			
Volume Right	222	0	32			
cSH	1700	1228	604			
Volume to Capacity	0.20	0.06	0.14			
Queue Length 95th (ft)	0	5	13			
Control Delay (s)	0.0	3.7	12.0			
Lane LOS			A			B
Approach Delay (s)	0.0	3.7	12.0			
Approach LOS			B			
Intersection Summary						
Average Delay			2.8			
Intersection Capacity Utilization			40.8%	ICU Level of Service	A	
Analysis Period (min)			15			



Direction	EBT	WB	NB	
Lane Configurations	↑		↑	
Sign Control	Free		Free	
Grade	0%		0%	
Volume (veh/h)	146	39	43	
Peak Hour Factor	0.92	0.92	0.92	
Hourly flow rate (vph)	159	42	47	
Pedestrians				
Lane Width (ft)				
Walking Speed (ft/s)				
Percent Blockage				
Right turn flare (veh)				
Median type			None	
Median storage (veh)				
Upstream signal (ft)				
pX, platoon unblocked				
vC, conflicting volume		201	431	
vC1, stage 1 conf vol			180	
vC2, stage 2 conf vol				
vCu, unblocked vol		201	431	
tC, single (s)		4.1	6.4	
tC, 2 stage (s)			6.2	
tF (s)		2.2	3.5	
p0 queue free %		97	96	
cM capacity (veh/h)		1371	562	
		863		
Direction	Lane #	EBT	WB	NB
Volume Total		201	204	65
Volume Left		0	47	22
Volume Right		42	0	43
cSH		1700	1371	732
Volume to Capacity		0.12	0.03	0.09
Queue Length 95th (ft)		0	3	7
Control Delay (s)		0.0	2.0	10.4
Lane LOS			A	B
Approach Delay (s)		0.0	2.0	10.4
Approach LOS				B
Intersection Summary				
Average Delay			2.3	
Intersection Capacity Utilization		33.6%		ICU Level of Service
Analysis Period (min)		15		A

HCM Unsignalized Intersection Capacity Analysis
 1: Frog's Leap & Conn Creek Rd.

PM WKDY E+Prj. Conditions
 12/11/2013



Direction	EB	WB	NB	SB	WB	EB
Lane Configurations	↖	↗		↖	↗	
Sign Control	Stop			Free	Free	
Grade	0%			0%	0%	
Volume (veh/h)	8	16	3	42	35	3
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	9	17	3	46	38	3
Pedestrians						
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)		1				
Median type	None					
Median storage veh						
Upstream signal (ft)						
pX, platoon unblocked						
vC, conflicting volume	92	40	41			
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	92	40	41			
tC, single (s)	6.4	6.2	4.1			
tC, 2 stage (s)						
tF (s)	3.5	3.3	2.2			
p0 queue free %	99	98	100			
cM capacity (veh/h)	906	1032	1568			

Direction	EB	WB	SB
Volume Total	26	49	41
Volume Left	9	3	0
Volume Right	17	0	3
cSH	1548	1568	1700
Volume to Capacity	0.02	0.00	0.02
Queue Length 95th (ft)	1	0	0
Control Delay (s)	8.7	0.5	0.0
Lane LOS	A	A	
Approach Delay (s)	8.7	0.5	0.0
Approach LOS	A		

Intersection Summary			
Average Delay		2.2	
Intersection Capacity Utilization	14.7%		ICU Level of Service A
Analysis Period (min)		15	



Movement	EB	EB	NB	NB	SB	SB
Lane Configurations	↘	↗		↕	↕	
Sign Control	Stop			Free	Free	
Grade	0%			0%	0%	
Volume (veh/h)	19	27	19	105	95	21
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	21	29	21	114	103	23
Pedestrians						
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)		1				
Median type	None					
Median storage (veh)						
Upstream signal (ft)						
pX, platoon unblocked						
vC, conflicting volume	270	115	126			
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	270	115	126			
tC, single (s)	6.4	6.2	4.1			
tC, 2 stage (s)						
tF (s)	3.5	3.3	2.2			
p0 queue free %	97	97	99			
cM capacity (veh/h)	709	938	1460			

Director, Lane #	EB 1	NB 1	SB 1
Volume Total	50	135	126
Volume Left	21	21	0
Volume Right	29	0	23
cSH	1598	1460	1700
Volume to Capacity	0.03	0.01	0.07
Queue Length 95th (ft)	2	1	0
Control Delay (s)	9.5	1.2	0.0
Lane LOS	A	A	
Approach Delay (s)	9.5	1.2	0.0
Approach LOS	A		

Intersection Summary			
Average Delay		2.1	
Intersection Capacity Utilization	23.2%	ICU Level of Service	A
Analysis Period (min)		15	

HCM Unsignalized Intersection Capacity Analysis
 2: Conn Creek Road & Silverado Trail

PM Wkday E+Prj. Conditions
 5/6/2014



Movement	EBL	EB	EBR	WBL	WB	WBR	NBL	NB	NBR	SBL	SB	SBR
Lane Configurations		↕	↗		↕		↖	↖		↖	↖	
Sign Control		Stop			Stop			Free			Free	
Grade		0%			0%			0%			0%	
Volume (veh/h)	35	0	56	10	3	5	42	356	0	1	893	35
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	38	0	61	11	3	5	46	387	0	1	971	38
Pedestrians												
Lane Width (ft)												
Walking Speed (ft/s)												
Percent Blockage												
Right turn flare (veh)			1									
Median type		None			None							
Median storage (veh)												
Upstream signal (ft)												
pX, platoon unblocked												
vC, conflicting volume	1477	1470	990	1482	1489	387	1009			387		
vC1, stage 1 conf vol												
vC2, stage 2 conf vol												
vCu, unblocked vol	1477	1470	990	1482	1489	387	1009			387		
tC, single (s)	7.1	6.5	6.2	7.1	6.5	6.2	4.1			4.1		
tC, 2 stage (s)												
tF (s)	3.5	4.0	3.3	3.5	4.0	3.3	2.2			2.2		
p0 queue free %	60	100	80	86	97	99	93			100		
cM capacity (veh/h)	96	119	299	78	116	661	687			1172		

Direction, Lane #	EB 1	WB 1	NB 1	NB 2	SB 1	SB 2
Volume Total	99	20	46	387	1	1009
Volume Left	38	11	46	0	1	0
Volume Right	61	5	0	0	0	38
cSH	207	111	687	1700	1172	1700
Volume to Capacity	0.48	0.18	0.07	0.23	0.00	0.59
Queue Length 95th (ft)	58	15	5	0	0	0
Control Delay (s)	37.3	44.1	10.6	0.0	8.1	0.0
Lane LOS	E	E	B		A	
Approach Delay (s)	37.3	44.1	1.1		0.0	
Approach LOS	E	E				

Intersection Summary		
Average Delay		3.2
Intersection Capacity Utilization	65.9%	ICU Level of Service C
Analysis Period (min)		15

HCM Unsignalized Intersection Capacity Analysis
 2: Conn Creek Road & Silverado Trail

MD WKND E+Prj. Conditions
 5/6/2014



Movement	EB	WB	NB	WB	NB	SB	WB	NB	SB	WB	NB
Lane Configurations		↖	↗		↖	↗		↖	↗		↖
Sign Control		Stop		Stop		Free		Free		Free	
Grade		0%		0%		0%		0%		0%	
Volume (veh/h)	65	2	57	16	1	8	42	464	7	18	597
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	71	2	62	17	1	9	46	504	8	20	649

Pedestrians

Lane Width (ft)	
Walking Speed (ft/s)	
Percent Blockage	
Right turn flare (veh)	1
Median type	None
Median storage (veh)	
Upstream signal (ft)	
pX, platoon unblocked	
vC, conflicting volume	1313 1311 669 1320 1328 508 689 512
vC1, stage 1 conf vol	
vC2, stage 2 conf vol	
vCu, unblocked vol	1313 1311 669 1320 1328 508 689 512
tC, single (s)	7.1 6.5 6.2 7.1 6.5 6.2 4.1 4.1
tC, 2 stage (s)	
tF (s)	3.5 4.0 3.3 3.5 4.0 3.3 2.2 2.2
p0 queue free %	44 99 86 84 99 98 95 98
cM capacity (veh/h)	126 148 458 109 145 565 905 1053

Direction, Lane #	EB-1	WB-1	NB-1	NB-2	SB-1	SB-2
Volume Total	135	27	46	512	20	689
Volume Left	71	17	46	0	20	0
Volume Right	62	9	0	8	0	40
cSH	211	148	905	1700	1053	1700
Volume to Capacity	0.64	0.18	0.05	0.30	0.02	0.41
Queue Length 95th (ft)	95	16	4	0	1	0
Control Delay (s)	47.9	34.6	9.2	0.0	8.5	0.0
Lane LOS	E	D	A		A	
Approach Delay (s)	47.9	34.6	0.8		0.2	
Approach LOS	E	D				

Intersection Summary

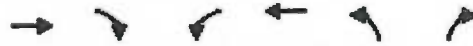
Average Delay	5.6
Intersection Capacity Utilization	50.5%
ICU Level of Service	A
Analysis Period (min)	15



	EB	WB	NB	WT	WT	WT
Lane Configurations	T		T		T	
Sign Control	Free		Free		Stop	
Grade	0%		0%		0%	
Volume (veh/h)	82	184	40	51	32	11
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	89	200	43	55	35	12
Pedestrians						
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)						
Median type	None					
Median storage veh						
Upstream signal (ft)						
pX, platoon unblocked						
vC, conflicting volume			289		332	189
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol			289		332	189
tC, single (s)			4.1		6.4	6.2
tC, 2 stage (s)						
tF (s)			2.2		3.5	3.3
p0 queue free %			97		95	99
cM capacity (veh/h)			1273		641	853

Direction, Lane #	EB T	WB T	NB T
Volume Total	289	99	47
Volume Left	0	43	35
Volume Right	200	0	12
cSH	1700	1273	684
Volume to Capacity	0.17	0.03	0.07
Queue Length 95th (ft)	0	3	5
Control Delay (s)	0.0	3.6	10.6
Lane LOS		A	B
Approach Delay (s)	0.0	3.6	10.6
Approach LOS			B

Intersection Summary			
Average Delay	2.0		
Intersection Capacity Utilization	33.9%	ICU Level of Service	A
Analysis Period (min)	15		



Volume	EB	WB	NB	WB	NB	EB
Lane Configurations	↑		↑		↑	
Sign Control	Free		Free		Stop	
Grade	0%		0%		0%	
Volume (veh/h)	95	35	18	97	18	13
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	103	38	20	105	20	14

Pedestrians

Lane Width (ft)	
Walking Speed (ft/s)	
Percent Blockage	
Right turn flare (veh)	
Median type	None
Median storage veh	
Upstream signal (ft)	
pX, platoon unblocked	
vC, conflicting volume	141 267 122
vC1, stage 1 conf vol	
vC2, stage 2 conf vol	
vCu, unblocked vol	141 267 122
tC, single (s)	4.1 6.4 6.2
tC, 2 stage (s)	
tF (s)	2.2 3.5 3.3
p0 queue free %	99 97 98
cM capacity (veh/h)	1442 713 929

Direction Lane #	EB	WB	NB
Volume Total	141	125	34
Volume Left	0	20	20
Volume Right	38	0	14
cSH	1700	1442	790
Volume to Capacity	0.08	0.01	0.04
Queue Length 95th (ft)	0	1	3
Control Delay (s)	0.0	1.3	9.8
Lane LOS		A	A
Approach Delay (s)	0.0	1.3	9.8
Approach LOS			A

Intersection Summary			
Average Delay	1.6		
Intersection Capacity Utilization	26.6%	ICU Level of Service	A
Analysis Period (min)	15		

HCM Unsignalized Intersection Capacity Analysis
 1: Frog's Leap & Conn Creek Rd.

PM WKDY N-T+Prj. Conditions
 12/3/2014



Movement	EB	NB	SB
Lane Configurations	↑	↑	↑
Sign Control	Stop		Free
Grade	0%		0%
Volume (veh/h)	8	16	3
Peak Hour Factor	0.92	0.92	0.92
Hourly flow rate (vph)	9	17	3
Pedestrians			
Lane Width (ft)			
Walking Speed (ft/s)			
Percent Blockage			
Right turn flare (veh)		1	
Median type	None		
Median storage veh			
Upstream signal (ft)			
pX, platoon unblocked			
vC, conflicting volume	165	71	73
vC1, stage 1 conf vol			
vC2, stage 2 conf vol			
vCu, unblocked vol	165	71	73
tC, single (s)	6.4	6.2	4.1
tC, 2 stage (s)			
tF (s)	3.5	3.3	2.2
p0 queue free %	99	98	100
cm capacity (veh/h)	824	991	1527
Direction, Lane #	EB 1	NB 1	SB 1
Volume Total	26	90	73
Volume Left	9	3	0
Volume Right	17	0	3
cSH	1487	1527	1700
Volume to Capacity	0.02	0.00	0.04
Queue Length 95th (ft)	1	0	0
Control Delay (s)	8.9	0.3	0.0
Lane LOS	A	A	
Approach Delay (s)	8.9	0.3	0.0
Approach LOS	A		
Intersection Summary			
Average Delay		1.4	
Intersection Capacity Utilization		16.6%	ICU Level of Service
Analysis Period (min)		15	A



	EB	WB	SB	NB	WB	SB
Lane Configurations						
Sign Control	Stop			Free	Free	
Grade	0%			0%	0%	
Volume (veh/h)	19	27	19	149	137	21
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	21	29	21	162	149	23
Pedestrians						
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)		1				
Median type None						
Median storage veh						
Upstream signal (ft)						
pX, platoon unblocked						
vC, conflicting volume	364	160	172			
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	364	160	172			
tC, single (s)	6.4	6.2	4.1			
tC, 2 stage (s)						
tF (s)	3.5	3.3	2.2			
p0 queue free %	97	97	99			
cM capacity (veh/h)	626	885	1405			
Direction Lane #						
	EB 1	WB 1	SB 1			
Volume Total	50	183	172			
Volume Left	21	21	0			
Volume Right	29	0	23			
cSH	1507	1405	1700			
Volume to Capacity	0.03	0.01	0.10			
Queue Length 95th (ft)	3	1	0			
Control Delay (s)	9.9	1.0	0.0			
Lane LOS	A	A				
Approach Delay (s)	9.9	1.0	0.0			
Approach LOS	A					
Intersection Summary						
Average Delay			1.7			
Intersection Capacity Utilization		30.7%		ICU Level of Service		A
Analysis Period (min)			15			

HCM Unsignalized Intersection Capacity Analysis
 2: Conn Creek Road & Silverado Trail

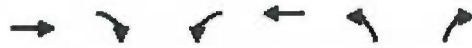
PM WKDY N-T+Prj. Conditions
 12/3/2014



Lane Configurations	EB		WB		NB		SB					
Sign Control	Stop		Stop		Free		Free					
Grade	0%		0%		0%		0%					
Volume (veh/h)	52	0	81	10	3	5	50	367	0	1	921	41
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	57	0	88	11	3	5	54	399	0	1	1001	45
Pedestrians												
Lane Width (ft)												
Walking Speed (ft/s)												
Percent Blockage												
Right turn flare (veh)	1											
Median type	None				None							
Median storage (veh)												
Upstream signal (ft)												
pX, platoon unblocked												
vC, conflicting volume	1540	1533	1023	1555	1555	399	1046			399		
vC1, stage 1 conf vol												
vC2, stage 2 conf vol												
vCu, unblocked vol	1540	1533	1023	1555	1555	399	1046			399		
tC, single (s)	7.1	6.5	6.2	7.1	6.5	6.2	4.1			4.1		
tC, 2 stage (s)												
tF (s)	3.5	4.0	3.3	3.5	4.0	3.3	2.2			2.2		
p0 queue free %	34	100	69	82	97	99	92			100		
cM capacity (veh/h)	85	107	286	60	104	651	665			1160		
Direction, Lane #	EB 1	WB 1	NB 1	NB 2	SB 1	SB 2						
Volume Total	145	20	54	399	1	1046						
Volume Left	57	11	54	0	1	0						
Volume Right	88	5	0	0	0	45						
cSH	172	88	665	1700	1160	1700						
Volume to Capacity	0.84	0.22	0.08	0.23	0.00	0.62						
Queue Length 95th (ft)	146	20	7	0	0	0						
Control Delay (s)	85.8	57.2	10.9	0.0	8.1	0.0						
Lane LOS	F	F	B		A							
Approach Delay (s)	85.8	57.2	1.3		0.0							
Approach LOS	F	F										
Intersection Summary												
Average Delay	8.5											
Intersection Capacity Utilization	69.3%				ICU Level of Service				C			
Analysis Period (min)	15											



Approach	EB	WB	NB	SB
Lane Configurations	↖ ↗	↖ ↗	↖ ↗	↖ ↗
Sign Control	Stop		Stop	Free
Grade	0%		0%	0%
Volume (veh/h)	86	2	78	16
Peak Hour Factor	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	93	2	85	17
Pedestrians				
Lane Width (ft)				
Walking Speed (ft/s)				
Percent Blockage				
Right turn flare (veh)		1		
Median type	None		None	
Median storage (veh)				
Upstream signal (ft)				
pX, platoon unblocked				
vC, conflicting volume	1397	1396	699	1406
vC1, stage 1 conf vol				
vC2, stage 2 conf vol				
vCu, unblocked vol	1397	1396	699	1406
tC, single (s)	7.1	6.5	6.2	7.1
tC, 2 stage (s)				
fF (s)	3.5	4.0	3.3	3.5
p0 queue free %	13	98	81	80
cM capacity (veh/h)	108	128	440	86
Direction Lane #	EB 1	WB 1	NB 1	SB 1
Volume Total	180	27	65	527
Volume Left	93	17	65	0
Volume Right	85	9	0	8
cSH	176	120	875	1700
Volume to Capacity	1.02	0.23	0.07	0.31
Queue Length 95th (ft)	212	20	6	0
Control Delay (s)	127.6	43.4	9.4	0.0
Lane LOS	F	E	A	A
Approach Delay (s)	127.6	43.4	1.0	0.2
Approach LOS	F	E		
Intersection Summary				
Average Delay			16.1	
Intersection Capacity Utilization		57.1%		ICU Level of Service
Analysis Period (min)			15	B



	EB	WB	NB	SB	WB	EB
Lane Configurations	T		T		T	
Sign Control	Free		Free		Stop	
Grade	0%		0%		0%	
Volume (veh/h)	102	204	67	92	51	29
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	111	222	73	100	55	32
Pedestrians						
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)						
Median type	None					
Median storage (veh)						
Upstream signal (ft)						
pX, platoon unblocked						
vC, conflicting volume	333		467		222	
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	333		467		222	
tC, single (s)	4.1		6.4		6.2	
tC, 2 stage (s)						
tF (s)	2.2		3.5		3.3	
p0 queue free %	94		89		96	
cM capacity (veh/h)	1227		521		818	
Direction, Lane #	EB 1	WB 1	NB 1			
Volume Total	333	173	87			
Volume Left	0	73	55			
Volume Right	222	0	32			
cSH	1700	1227	600			
Volume to Capacity	0.20	0.06	0.14			
Queue Length 95th (ft)	0	5	13			
Control Delay (s)	0.0	3.7	12.0			
Lane LOS	A		B			
Approach Delay (s)	0.0	3.7	12.0			
Approach LOS	A		B			
Intersection Summary						
Average Delay	2.8					
Intersection Capacity Utilization	41.0%		ICU Level of Service		A	
Analysis Period (min)	15					



Movement	EB	WB	WB	NB	WB	
Lane Configurations	T		T		T	
Sign Control	Free		Free		Stop	
Grade	0%		0%		0%	
Volume (veh/h)	153	39	46	154	20	41
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	166	42	50	167	22	45
Pedestrians						
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)						
Median type	None					
Median storage veh						
Upstream signal (ft)						
pX, platoon unblocked						
vC, conflicting volume			209	455	188	
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol			209	455	188	
tC, single (s)			4.1	6.4	6.2	
tC, 2 stage (s)						
tF (s)			2.2	3.5	3.3	
p0 queue free %			96	96	95	
cM capacity (veh/h)			1362	543	855	
Direction, Lane #	EB 1	WB 1	WB 1	WB 1	WB 1	WB 1
Volume Total	209	217	66			
Volume Left	0	50	22			
Volume Right	42	0	45			
cSH	1700	1362	719			
Volume to Capacity	0.12	0.04	0.09			
Queue Length 95th (ft)	0	3	8			
Control Delay (s)	0.0	2.0	10.5			
Lane LOS		A	B			
Approach Delay (s)	0.0	2.0	10.5			
Approach LOS			B			
Intersection Summary						
Average Delay			2.3			
Intersection Capacity Utilization			34.7%	ICU Level of Service	A	
Analysis Period (min)			15			



Direction	EB	WB	NB	SB
Lane Configurations	↑	↑	↑	↑
Sign Control	Stop		Free	Free
Grade	0%		0%	0%
Volume (veh/h)	6	16	3	105
Peak Hour Factor	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	9	17	3	114
Pedestrians				
Lane Width (ft)				
Walking Speed (ft/s)				
Percent Blockage				
Right turn flare (veh)		1		
Median type	None			
Median storage (veh)				
Upstream signal (ft)				
pX, platoon unblocked				
vC, conflicting volume	214	93	95	
vC1, stage 1 conf vol				
vC2, stage 2 conf vol				
vCu, unblocked vol	214	93	95	
tC, single (s)	6.4	6.2	4.1	
tC, 2 stage (s)				
tF (s)	3.5	3.3	2.2	
p0 queue free %	99	98	100	
cM capacity (veh/h)	773	964	1499	
Direction, Lane #	EB 1	NB 1	SB 1	
Volume Total	26	117	95	
Volume Left	9	3	0	
Volume Right	17	0	3	
cSH	1446	1499	1700	
Volume to Capacity	0.02	0.00	0.06	
Queue Length 95th (ft)	1	0	0	
Control Delay (s)	9.1	0.2	0.0	
Lane LOS	A	A		
Approach Delay (s)	9.1	0.2	0.0	
Approach LOS	A			
Intersection Summary				
Average Delay		1.1		
Intersection Capacity Utilization		17.9%	ICU Level of Service	A
Analysis Period (min)		15		



Movement	EBL	EBR	NBL	NBT	SBT	EBR
Lane Configurations	←	←		←	←	
Sign Control	Stop			Free	Free	
Grade	0%			0%	0%	
Volume (veh/h)	19	27	19	210	130	21
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	21	29	21	228	141	23

Pedestrians

Lane Width (ft)

Walking Speed (ft/s)

Percent Blockage

Right turn flare (veh) 1

Median type None

Median storage (veh)

Upstream signal (ft)

pX, platoon unblocked

vC, conflicting volume 422 153 164

vC1, stage 1 conf vol

vC2, stage 2 conf vol

vCu, unblocked vol 422 153 164

tC, single (s) 6.4 6.2 4.1

tC, 2 stage (s)

tF (s) 3.5 3.3 2.2

p0 queue free % 96 97 99

cM capacity (veh/h) 580 893 1414

Direction Lane #	EB 1	NB 1	SB 1
Volume Total	50	249	164
Volume Left	21	21	0
Volume Right	29	0	23
cSH	1403	1414	1700
Volume to Capacity	0.04	0.01	0.10
Queue Length 95th (ft)	3	1	0
Control Delay (s)	10.1	0.7	0.0
Lane LOS	B	A	
Approach Delay (s)	10.1	0.7	0.0
Approach LOS	B		

Intersection Summary

Average Delay	1.5		
Intersection Capacity Utilization	33.6%	ICU Level of Service	A
Analysis Period (min)	15		

HCM Unsignalized Intersection Capacity Analysis
 2: Conn Creek Road & Silverado Trail

PM Wkday Cumulative (NP) Conditions
 12/3/2014



MOVEMENT	EBL	EB	EBR	WBL	WB	WBR	NBL	NB	NBR	SBL	SB	SBR
Lane Configurations		↔	↔		↔	↔		↔	↔		↔	↔
Sign Control		Stop			Stop			Free			Free	
Grade		0%			0%			0%			0%	
Volume (veh/h)	49	0	76	10	3	5	49	585	0	1	1467	40
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	53	0	83	11	3	5	53	636	0	1	1595	43
Pedestrians												
Lane Width (ft)												
Walking Speed (ft/s)												
Percent Blockage												
Right turn flare (veh)	1											
Median type	None			None								
Median storage (veh)												
Upstream signal (ft)												
pX, platoon unblocked												
vC, conflicting volume	2368	2361	1616	2380	2383	636	1638				636	
vC1, stage 1 conf vol												
vC2, stage 2 conf vol												
vCu, unblocked vol	2368	2361	1616	2380	2383	636	1638				636	
tC, single (s)	7.1	6.5	6.2	7.1	6.5	6.2	4.1				4.1	
tC, 2 stage (s)												
tF (s)	3.5	4.0	3.3	3.5	4.0	3.3	2.2				2.2	
p0 queue free %	0	100	35	0	89	99	87				100	
cM capacity (veh/h)	20	31	128	8	30	478	395				948	
Direction, Lane #	EB 1	WB 1	NB 1	NB 2	SB 1	SB 2						
Volume Total	136	20	53	636	1	1638						
Volume Left	53	11	53	0	1	0						
Volume Right	83	5	0	0	0	43						
cSH	41	13	395	1700	948	1700						
Volume to Capacity	3.30	1.56	0.13	0.37	0.00	0.96						
Queue Length 95th (ft)	Err	80	12	0	0	0						
Control Delay (s)	Err	885.9	15.5	0.0	8.8	0.0						
Lane LOS	F	F	C		A							
Approach Delay (s)	Err	885.9	1.2		0.0							
Approach LOS	F	F										
Intersection Summary												
Average Delay	554.3											
Intersection Capacity Utilization	97.7%											
ICU Level of Service	F											
Analysis Period (min)	15											

HCM Unsignalized Intersection Capacity Analysis
 2: Conn Creek Road & Silverado Trail

MD WKND Cumulative (NP) Conditions
 12/3/2014



Direction	EB	WB	NB	SB	EB	WB	NB	SB
Lane Configurations	↔	↔	↔	↔	↔	↔	↔	↔
Sign Control	Stop		Stop		Free		Free	
Grade	0%		0%		0%		0%	
Volume (veh/h)	78	2	69	16	1	8	54	763
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	85	2	75	17	1	9	59	829
Pedestrians								
Lane Width (ft)								
Walking Speed (ft/s)								
Percent Blockage								
Right turn flare (veh)			1					
Median type	None		None					
Median storage (veh)								
Upstream signal (ft)								
pX, platoon unblocked								
vC, conflicting volume	2088	2086	1093	2095	2109	833	1120	837
vC1, stage 1 conf vol								
vC2, stage 2 conf vol								
vCu, unblocked vol	2088	2086	1093	2095	2109	833	1120	837
tC, single (s)	7.1	6.5	6.2	7.1	6.5	6.2	4.1	4.1
tC, 2 stage (s)								
f (s)	3.5	4.0	3.3	3.5	4.0	3.3	2.2	2.2
p0 queue free %	0	95	71	27	98	98	91	98
cM capacity (veh/h)	34	47	261	24	45	368	624	797
Direction, Lane #	EB 1	WB 1	NB 1	NB 2	SB 1	SB 2		
Volume Total	162	27	59	837	20	1120		
Volume Left	85	17	59	0	20	0		
Volume Right	75	9	0	8	0	53		
cSH	57	35	624	1700	797	1700		
Volume to Capacity	2.84	0.77	0.09	0.49	0.02	0.66		
Queue Length 95th (ft)	418	68	8	0	2	0		
Control Delay (s)	981.8	252.9	11.4	0.0	9.6	0.0		
Lane LOS	F	F	B		A			
Approach Delay (s)	981.8	252.9	0.7		0.2			
Approach LOS	F	F						
Intersection Summary								
Average Delay			75.0					
Intersection Capacity Utilization			72.2%		ICU Level of Service		C	
Analysis Period (min)			15					



Direction	EB1	EB2	WB1	WB2	NB1	NB2
Lane Configurations	↑		↑		↑	
Sign Control	Free		Free		Stop	
Grade	0%		0%		0%	
Volume (veh/h)	554	190	63	364	49	28
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	602	207	68	396	53	30

Pedestrians

Lane Width (ft)

Walking Speed (ft/s)

Percent Blockage

Right turn flare (veh)

Median type: None

Median storage (veh)

Upstream signal (ft)

pX, platoon unblocked

vC, conflicting volume	809	1238	705
vC1, stage 1 conf vol			
vC2, stage 2 conf vol			
vCu, unblocked vol	809	1238	705
tC, single (s)	4.1	6.4	6.2
tC, 2 stage (s)			
fF (s)	2.2	3.5	3.3
p0 queue free %	92	70	93
cM capacity (veh/h)	817	178	436

Direction Lane #	EB 1	WB 1	NB 1
Volume Total	809	464	84
Volume Left	0	68	53
Volume Right	207	0	30
cSH	1700	817	227
Volume to Capacity	0.48	0.08	0.37
Queue Length 95th (ft)	0	7	40
Control Delay (s)	0.0	2.4	29.9
Lane LOS		A	D
Approach Delay (s)	0.0	2.4	29.9
Approach LOS			D

Intersection Summary

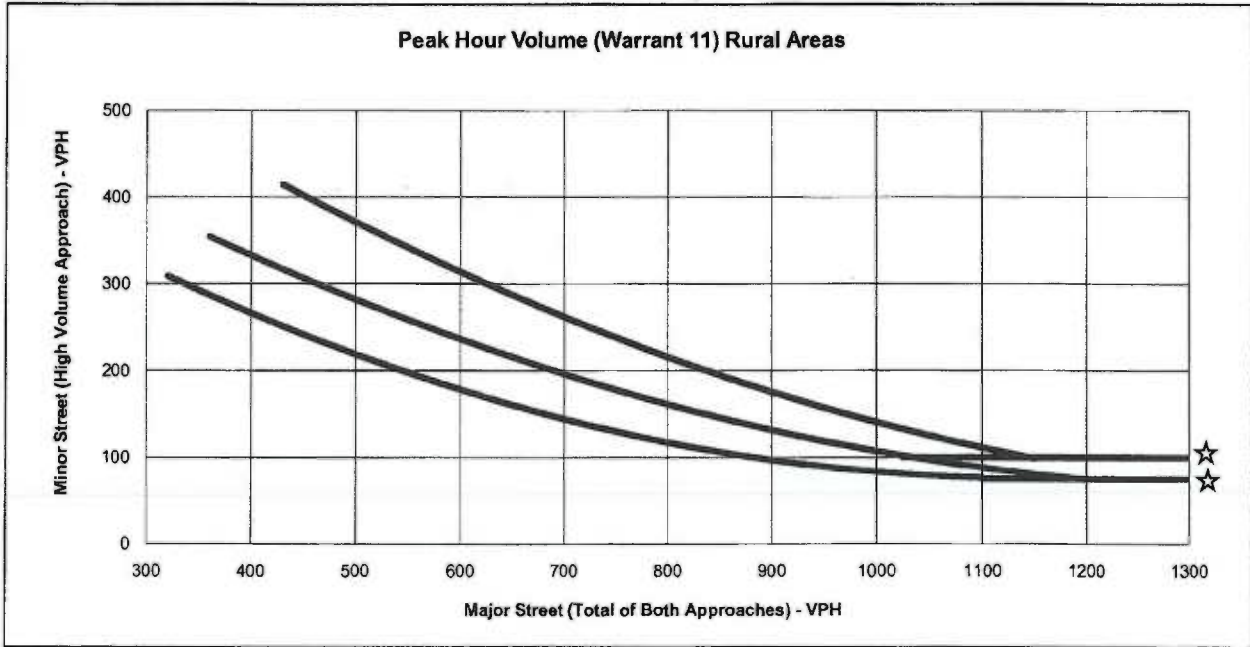
Average Delay	2.7		
Intersection Capacity Utilization	77.8%	ICU Level of Service	D
Analysis Period (min)	15		



MOVEMENT	EBT	EBR	WB	WLT	NB	NBR
Lane Configurations	↑			↓		↑
Sign Control	Free			Free	Stop	
Grade	0%			0%	0%	
Volume (veh/h)	484	36	42	484	19	39
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	526	39	46	526	21	42
Pedestrians						
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)						
Median type	None					
Median storage (veh)						
Upstream signal (ft)						
pX, platoon unblocked						
vC, conflicting volume			565		1163	546
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol			565		1163	546
tC, single (s)			4.1		6.4	6.2
tC, 2 stage (s)						
fF (s)			2.2		3.5	3.3
p0 queue free %			95		90	92
cm capacity (veh/h)			1007		205	538
Direction, Lane #						
	EB 1	WB 1	NB 1			
Volume Total	565	572	63			
Volume Left	0	46	21			
Volume Right	39	0	42			
cSH	1700	1007	352			
Volume to Capacity	0.33	0.05	0.18			
Queue Length 95th (ft)	0	4	16			
Control Delay (s)	0.0	1.2	17.5			
Lane LOS		A	C			
Approach Delay (s)	0.0	1.2	17.5			
Approach LOS			C			
Intersection Summary						
Average Delay			1.5			
Intersection Capacity Utilization			68.9%	ICU Level of Service	C	
Analysis Period (min)			15			

Both 1 Lane Approaches		2 or more Lane and One Lane Approaches		Both 2 or more Lane Approaches	
Major Street Total of Both Approaches	Minor Street High Volume Approach	Major Street Total of Both Approaches	Minor Street High Volume Approach	Major Street Total of Both Approaches	Minor Street High Volume Approach
370	280				
400	270	460	297	430	410
500	215	500	290	500	380
600	185	600	230	600	310
700	140	700	198	700	265
800	115	800	170	800	210
900	99	900	125	900	180
1000	85	1000	105	1000	140
1100	75	1100	90	1100	110
1200	75	1200	75	1150	100
1300	75	1300	75	1300	100

* Note: Values in Table are approximate, actual curves based upon 2nd order polynomial equation

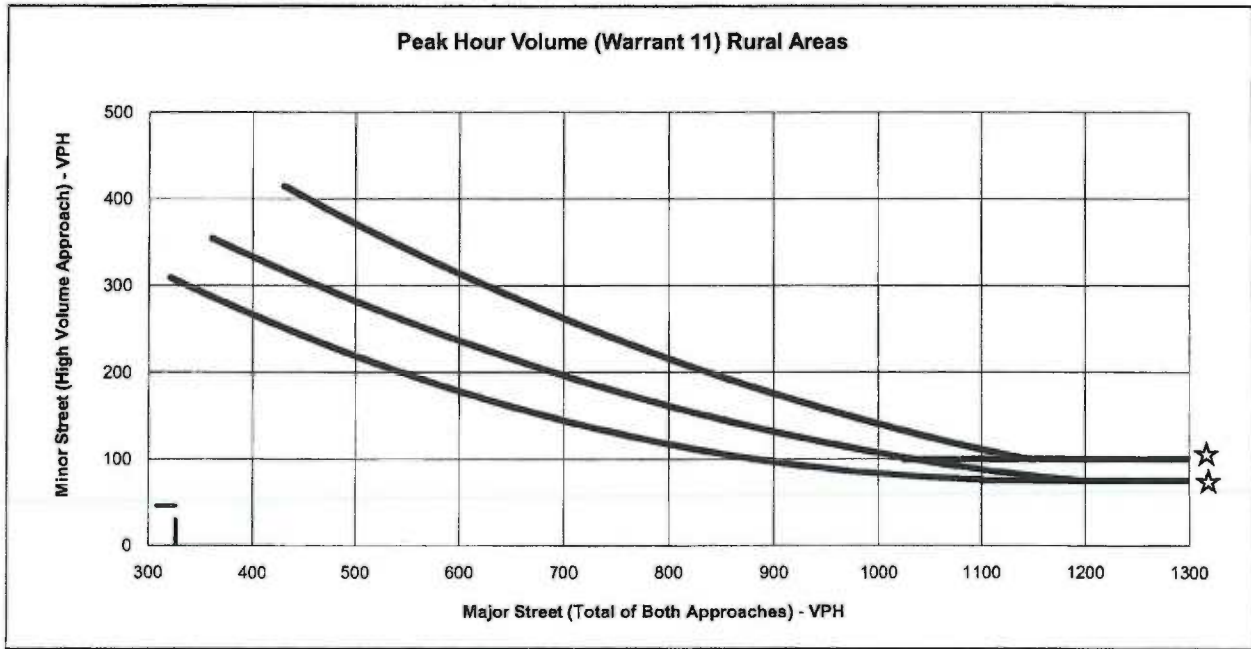


☆ NOTE:
 100 VPH APPLIES AS THE LOWER THRESHOLD VOLUME FOR MINOR STREET APPROACH WITH TWO OR MORE LANES AND 75 VPH APPLIES AS THE LOWER THRESHOLD VOLUME FOR A MINOR STREET APPROACHING WITH ONE LANE.

Intersection: Frog's Leap Driveway / Conn Creek Road (S.R. 128)
 Scenario: Weekday PM Peak Hour----Near-Term plus Project Conditions (worst case)
 Minor St. Volume: 24
 Major St. Volume: 150
 Warrant Met?: NO

Both 1 Lane Approaches		2 or more Lane and One Lane Approaches		Both 2 or more Lane Approaches	
Major Street Total of Both Approaches	Minor Street High Volume Approach	Major Street Total of Both Approaches	Minor Street High Volume Approach	Major Street Total of Both Approaches	Minor Street High Volume Approach
370	280				
400	270	460	297	430	410
500	215	500	290	500	380
600	185	600	230	600	310
700	140	700	198	700	265
800	115	800	170	800	210
900	99	900	125	900	180
1000	85	1000	105	1000	140
1100	75	1100	90	1100	110
1200	75	1200	75	1150	100
1300	75	1300	75	1300	100

* Note: Values in Table are approximate, actual curves based upon 2nd order polynomial equation

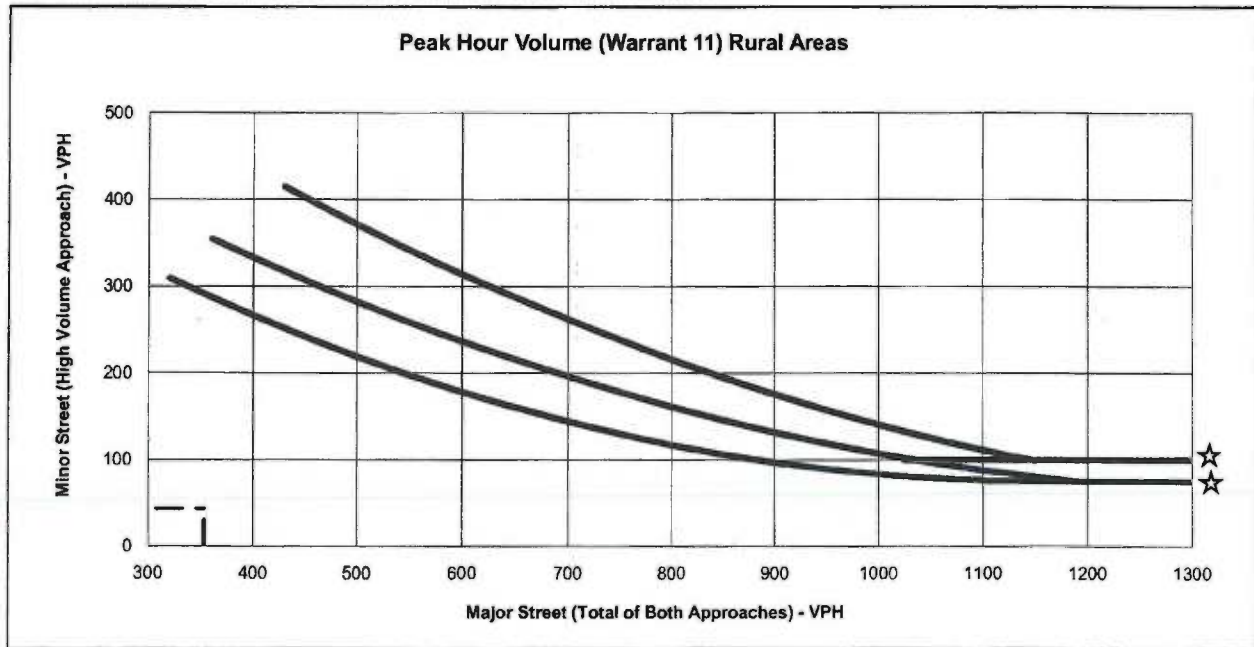


★ NOTE:
 100 VPH APPLIES AS THE LOWER THRESHOLD VOLUME FOR MINOR STREET APPROACH WITH TWO OR MORE LANES AND 75 VPH APPLIES AS THE LOWER THRESHOLD VOLUME FOR A MINOR STREET APPROACHING WITH ONE LANE.

Intersection: Frog's Leap Driveway / Conn Creek Road (S.R. 128)
 Scenario: Saturday mid-day peak hour -- Near-Term plus Project Conditions (worst case)
 Minor St. Volume: 46
 Major St. Volume: 326
 Warrant Met?: NO

Both 1 Lane Approaches		2 or more Lane and One Lane Approaches		Both 2 or more Lane Approaches	
Major Street Total of Both Approaches	Minor Street High Volume Approach	Major Street Total of Both Approaches	Minor Street High Volume Approach	Major Street Total of Both Approaches	Minor Street High Volume Approach
370	280				
400	270	460	297	430	410
500	215	500	290	500	380
600	185	600	230	600	310
700	140	700	198	700	265
800	115	800	170	800	210
900	99	900	125	900	180
1000	85	1000	105	1000	140
1100	75	1100	90	1100	110
1200	75	1200	75	1150	100
1300	75	1300	75	1300	100

* Note: Values in Table are approximate, actual curves based upon 2nd order polynomial equation

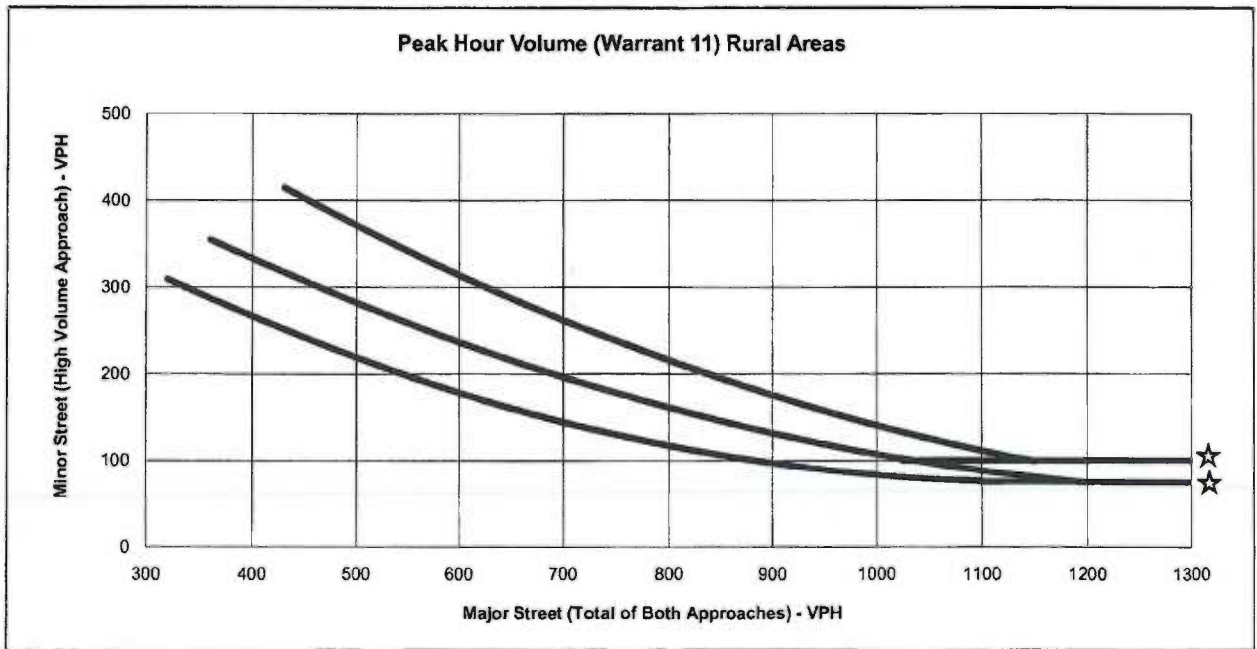


★ NOTE:
 100 VPH APPLIES AS THE LOWER THRESHOLD VOLUME FOR MINOR STREET APPROACH WITH TWO OR MORE LANES AND 75 VPH APPLIES AS THE LOWER THRESHOLD VOLUME FOR A MINOR STREET APPROACHING WITH ONE LANE.

Intersection: Rutherford Road / Conn Creek Road (S.R. 128)
 Scenario: Weekday PM Peak Hour --- Existing Conditions
 Minor St. Volume: 43
 Major St. Volume: 353
 Warrant Met?: NO

Both 1 Lane Approaches		2 or more Lane and One Lane Approaches		Both 2 or more Lane Approaches	
Major Street Total of Both Approaches	Minor Street High Volume Approach	Major Street Total of Both Approaches	Minor Street High Volume Approach	Major Street Total of Both Approaches	Minor Street High Volume Approach
370	280				
400	270	460	297	430	410
500	215	500	290	500	380
600	185	600	230	600	310
700	140	700	198	700	265
800	115	800	170	800	210
900	99	900	125	900	180
1000	85	1000	105	1000	140
1100	75	1100	90	1100	110
1200	75	1200	75	1150	100
1300	75	1300	75	1300	100

* Note: Values in Table are approximate, actual curves based upon 2nd order polynomial equation

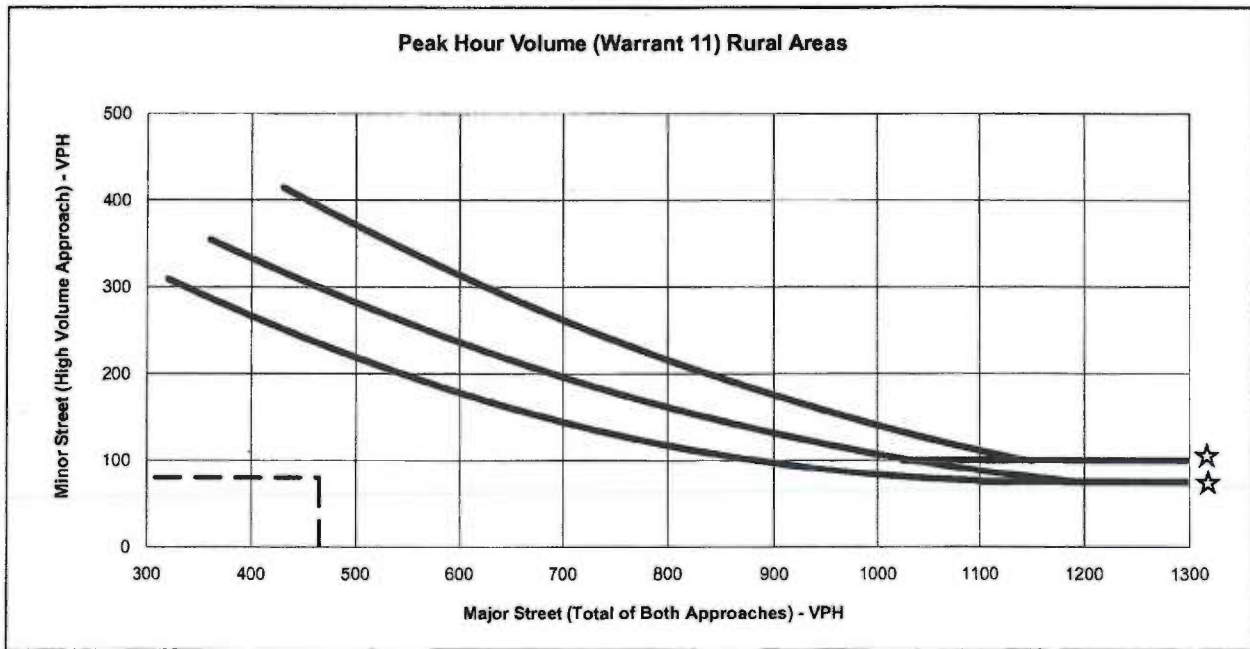


☆ NOTE:
 100 VPH APPLIES AS THE LOWER THRESHOLD VOLUME FOR MINOR STREET APPROACH WITH TWO OR MORE LANES AND 75 VPH APPLIES AS THE LOWER THRESHOLD VOLUME FOR A MINOR STREET APPROACHING WITH ONE LANE.

Intersection: Rutherford Road / Conn Creek Road (S.R. 128)
 Scenario: Saturday mid-day peak hour -- Existing Conditions
 Minor St. Volume: 30
 Major St. Volume: 226
 Warrant Met?: NO

Both 1 Lane Approaches		2 or more Lane and One Lane Approaches		Both 2 or more Lane Approaches	
Major Street Total of Both Approaches	Minor Street High Volume Approach	Major Street Total of Both Approaches	Minor Street High Volume Approach	Major Street Total of Both Approaches	Minor Street High Volume Approach
370	280				
400	270	460	297	430	410
500	215	500	290	500	380
600	185	600	230	600	310
700	140	700	198	700	265
800	115	800	170	800	210
900	99	900	125	900	180
1000	85	1000	105	1000	140
1100	75	1100	90	1100	110
1200	75	1200	75	1150	100
1300	75	1300	75	1300	100

* Note: Values in Table are approximate, actual curves based upon 2nd order polynomial equation

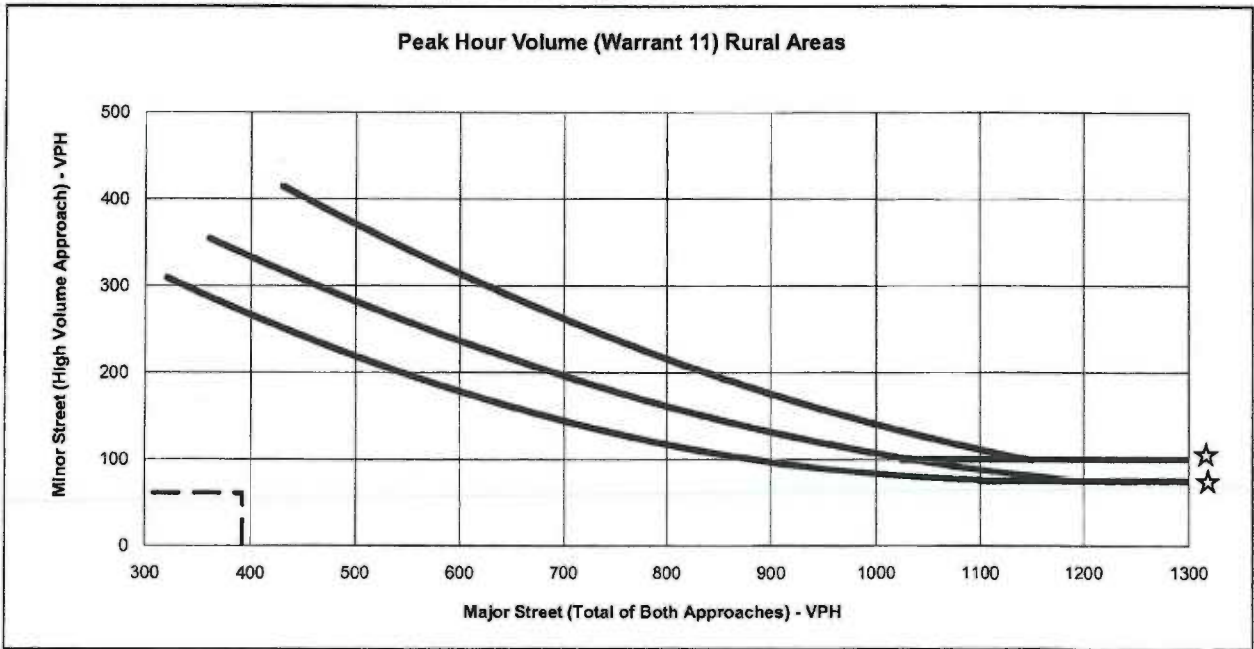


☆ NOTE:
 100 VPH APPLIES AS THE LOWER THRESHOLD VOLUME FOR MINOR STREET APPROACH WITH TWO OR MORE LANES AND 75 VPH APPLIES AS THE LOWER THRESHOLD VOLUME FOR A MINOR STREET APPROACHING WITH ONE LANE.

Intersection: Rutherford Road / Conn Creek Road (S.R. 128)
 Scenario: Weekday PM Peak Hour --- Near-Term plus Project Conditions (Worst Case)
 Minor St. Volume: 80
 Major St. Volume: 465
 Warrant Met?: NO

Both 1 Lane Approaches		2 or more Lane and One Lane Approaches		Both 2 or more Lane Approaches	
Major Street Total of Both Approaches	Minor Street High Volume Approach	Major Street Total of Both Approaches	Minor Street High Volume Approach	Major Street Total of Both Approaches	Minor Street High Volume Approach
370	280				
400	270	460	297	430	410
500	215	500	290	500	380
600	185	600	230	600	310
700	140	700	198	700	265
800	115	800	170	800	210
900	99	900	125	900	180
1000	85	1000	105	1000	140
1100	75	1100	90	1100	110
1200	75	1200	75	1150	100
1300	75	1300	75	1300	100

* Note: Values in Table are approximate, actual curves based upon 2nd order polynomial equation

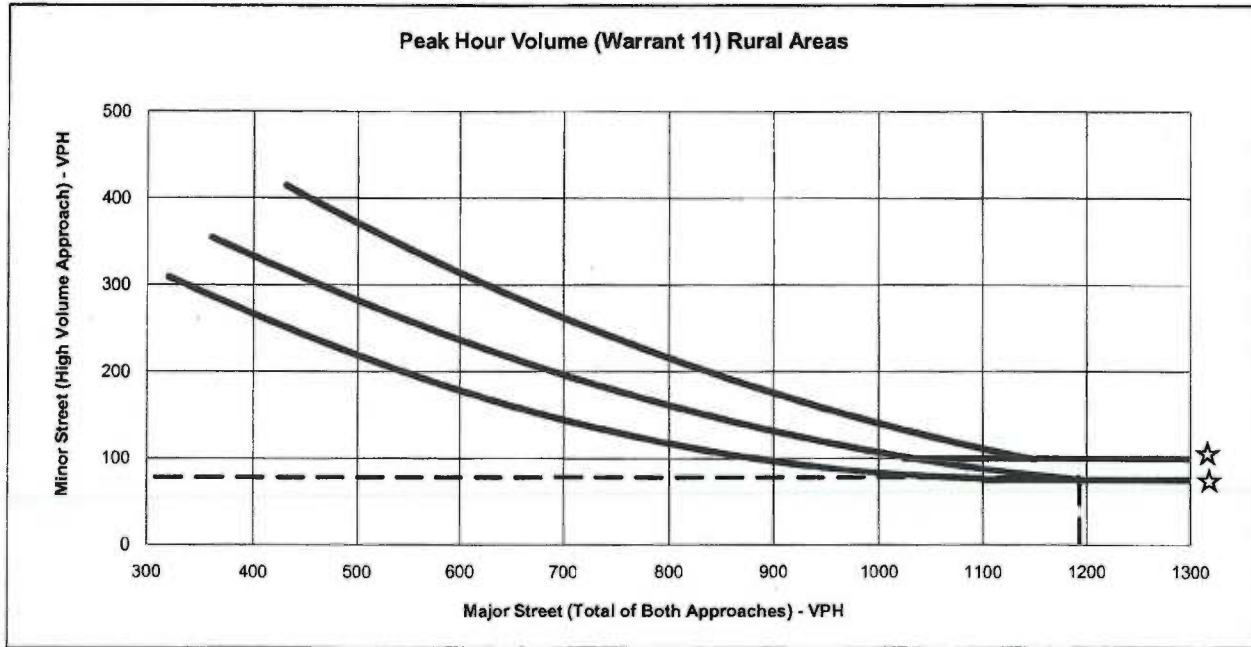


★ NOTE:
 100 VPH APPLIES AS THE LOWER THRESHOLD VOLUME FOR MINOR STREET APPROACH WITH TWO OR MORE LANES AND 75 VPH APPLIES AS THE LOWER THRESHOLD VOLUME FOR A MINOR STREET APPROACHING WITH ONE LANE.

Intersection: Rutherford Road / Conn Creek Road (S.R. 128)
 Scenario: Saturday mid-day peak hour – Near-Term plus Project Conditions (Worst Case)
 Minor St. Volume: 61
 Major St. Volume: 392
 Warrant Met?: **NO**

Both 1 Lane Approaches		2 or more Lane and One Lane Approaches		Both 2 or more Lane Approaches	
Major Street Total of Both Approaches	Minor Street High Volume Approach	Major Street Total of Both Approaches	Minor Street High Volume Approach	Major Street Total of Both Approaches	Minor Street High Volume Approach
370	280				
400	270	460	297	430	410
500	215	500	290	500	380
600	185	600	230	600	310
700	140	700	198	700	265
800	115	800	170	800	210
900	99	900	125	900	180
1000	85	1000	105	1000	140
1100	75	1100	90	1100	110
1200	75	1200	75	1150	100
1300	75	1300	75	1300	100

* Note: Values in Table are approximate, actual curves based upon 2nd order polynomial equation

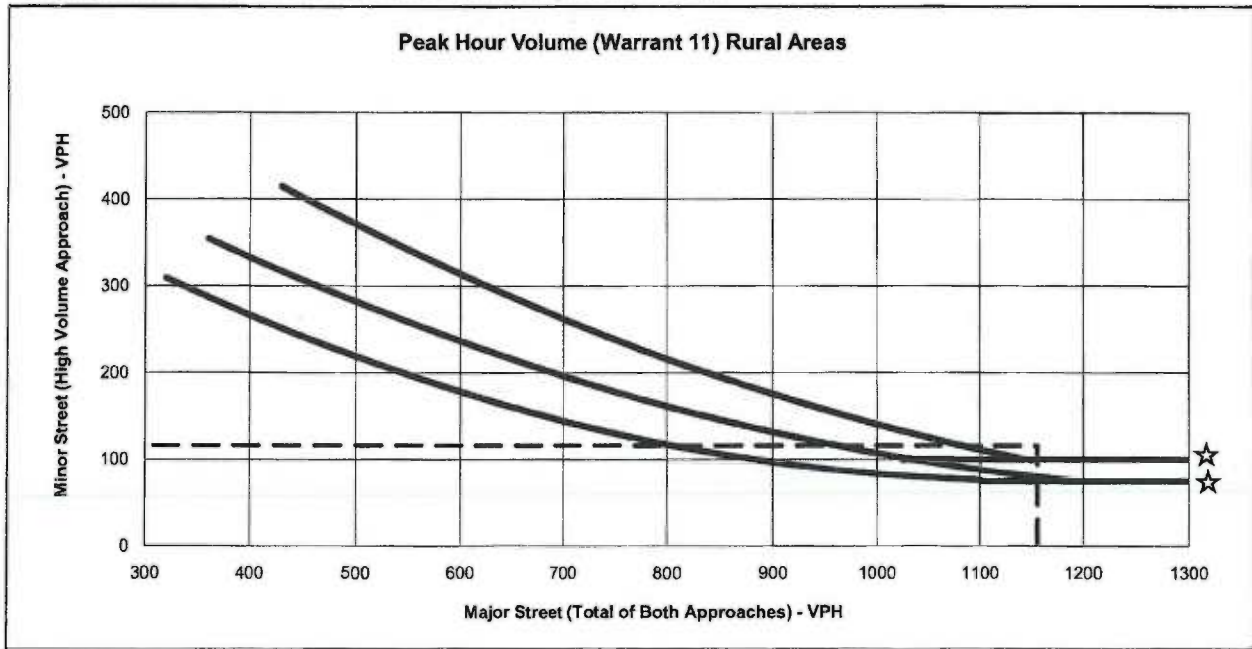


☆ NOTE:
 100 VPH APPLIES AS THE LOWER THRESHOLD VOLUME FOR MINOR STREET APPROACH WITH TWO OR MORE LANES AND 75 VPH APPLIES AS THE LOWER THRESHOLD VOLUME FOR A MINOR STREET APPROACHING WITH ONE LANE.

Intersection: Silverado Trail / Conn Creek Road (S.R. 128)
 Scenario: Weekday PM Peak Hour --- Existing Conditions
 Minor St. Volume: 78
 Major St. Volume: 1193
 Warrant Met?: YES

Both 1 Lane Approaches		2 or more Lane and One Lane Approaches		Both 2 or more Lane Approaches	
Major Street Total of Both Approaches	Minor Street High Volume Approach	Major Street Total of Both Approaches	Minor Street High Volume Approach	Major Street Total of Both Approaches	Minor Street High Volume Approach
370	280				
400	270	460	297	430	410
500	215	500	290	500	380
600	185	600	230	600	310
700	140	700	198	700	265
800	115	800	170	800	210
900	99	900	125	900	180
1000	85	1000	105	1000	140
1100	75	1100	90	1100	110
1200	75	1200	75	1150	100
1300	75	1300	75	1300	100

* Note: Values in Table are approximate, actual curves based upon 2nd order polynomial equation

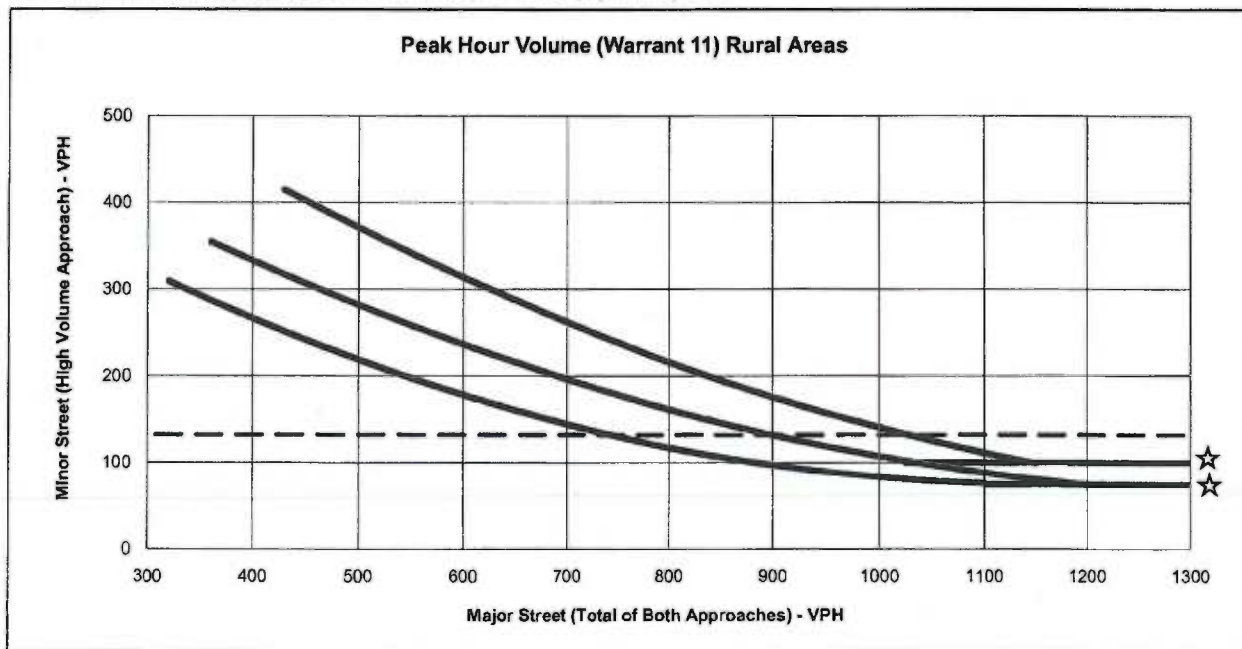


★ NOTE:
 100 VPH APPLIES AS THE LOWER THRESHOLD VOLUME FOR MINOR STREET APPROACH WITH TWO OR MORE LANES AND 75 VPH APPLIES AS THE LOWER THRESHOLD VOLUME FOR A MINOR STREET APPROACHING WITH ONE LANE.

Intersection: Silverado Trail / Conn Creek Road (S.R. 128)
 Scenario: Weekend Mid-Day PM Peak Hour --- Existing Conditions
 Minor St. Volume: 116
 Major St. Volume: 1156
 Warrant Met?: YES

Both 1 Lane Approaches		2 or more Lane and One Lane Approaches		Both 2 or more Lane Approaches	
Major Street Total of Both Approaches	Minor Street High Volume Approach	Major Street Total of Both Approaches	Minor Street High Volume Approach	Major Street Total of Both Approaches	Minor Street High Volume Approach
370	280				
400	270	460	297	430	410
500	215	500	290	500	380
600	185	600	230	600	310
700	140	700	198	700	265
800	115	800	170	800	210
900	99	900	125	900	180
1000	85	1000	105	1000	140
1100	75	1100	90	1100	110
1200	75	1200	75	1150	100
1300	75	1300	75	1300	100

* Note: Values in Table are approximate, actual curves based upon 2nd order polynomial equation

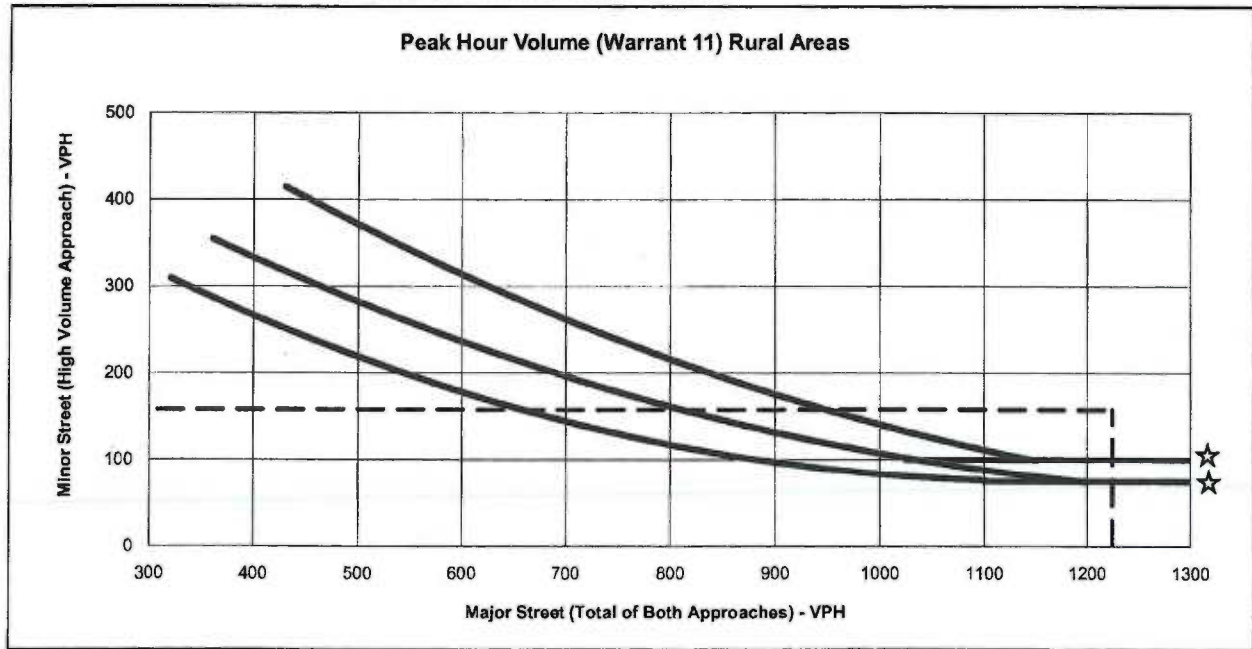


★ NOTE:
 100 VPH APPLIES AS THE LOWER THRESHOLD VOLUME FOR MINOR STREET APPROACH WITH TWO OR MORE LANES AND 75 VPH APPLIES AS THE LOWER THRESHOLD VOLUME FOR A MINOR STREET APPROACHING WITH ONE LANE.

Intersection: Silverado Trail / Conn Creek Road (S.R. 128)
 Scenario: Weekday PM Peak Hour --- Near-Term plus Project Conditions
 Minor St. Volume: 132
 Major St. Volume: 1379
 Warrant Met?: YES

Both 1 Lane Approaches		2 or more Lane and One Lane Approaches		Both 2 or more Lane Approaches	
Major Street Total of Both Approaches	Minor Street High Volume Approach	Major Street Total of Both Approaches	Minor Street High Volume Approach	Major Street Total of Both Approaches	Minor Street High Volume Approach
370	280				
400	270	460	297	430	410
500	215	500	290	500	380
600	185	600	230	600	310
700	140	700	198	700	265
800	115	800	170	800	210
900	99	900	125	900	180
1000	85	1000	105	1000	140
1100	75	1100	90	1100	110
1200	75	1200	75	1150	100
1300	75	1300	75	1300	100

* Note: Values in Table are approximate, actual curves based upon 2nd order polynomial equation



☆ NOTE:
 100 VPH APPLIES AS THE LOWER THRESHOLD VOLUME FOR MINOR STREET APPROACH WITH TWO OR MORE LANES AND 75 VPH APPLIES AS THE LOWER THRESHOLD VOLUME FOR A MINOR STREET APPROACHING WITH ONE LANE.

Intersection: Silverado Trail / Conn Creek Road (S.R. 128)
 Scenario: Weekend Mid-Day PM Peak Hour --- Near-Term plus Project Conditions
 Minor St. Volume: 158
 Major St. Volume: 1224
 Warrant Met?: YES

RADAR SPEED SURVEY

OMNI-MEANS LTD.

Conn Creek Rd. approaching Frogs Leap Winery Access

DATE: 11/16/13 TIME START: 1:30pm TIME END: 3:00pm WEATHER: Clear ROAD TYPE: 2 lanes; Rural
 DIRECTION: Both SPEED LIMIT: Not Posted OBSERVER: o-m CALIBRATION TEST: Yes

SPEED	FREQUENCY	ACUM %	PERCENTAGE BREAKDOWN
33	2	2.0	1**
34	1	3.0	1***
35	3	6.0	1****5*
36	5	11.0	1****5****1*
37	6	17.0	1****5****1****5**
38	6	23.0	1****5****1****5****2***
39	3	26.0	1****5****1****5****2****5*
40	3	29.0	1****5****1****5****2****5****
41	4	33.0	1****5****1****5****2****5****3***
42	5	38.0	1****5****1****5****2****5****3****5***
43	8	46.0	1****5****1****5****2****5****3****5****4****5*
44	5	51.0	1****5****1****5****2****5****3****5****4****5****5*
45	10	61.0	1****5****1****5****2****5****3****5****4****5****5****5****6*
46	8	69.0	1****5****1****5****2****5****3****5****4****5****5****5****6****5***
47	10	79.0	1****5****1****5****2****5****3****5****4****5****5****5****6****5****7****5****
48	6	85.0	1****5****1****5****2****5****3****5****4****5****5****5****6****5****7****5****8****5
49	2	87.0	1****5****1****5****2****5****3****5****4****5****5****5****6****5****7****5****8****5****
50	2	89.0	1****5****1****5****2****5****3****5****4****5****5****5****6****5****7****5****8****5****
51	3	92.0	1****5****1****5****2****5****3****5****4****5****5****5****6****5****7****5****8****5****9***
52	6	98.0	1****5****1****5****2****5****3****5****4****5****5****5****6****5****7****5****8****5****9****5***
53	1	99.0	1****5****1****5****2****5****3****5****4****5****5****5****6****5****7****5****8****5****9****5****
54	1	100.0	1****5****1****5****2****5****3****5****4****5****5****5****6****5****7****5****8****5****9****5****0

100

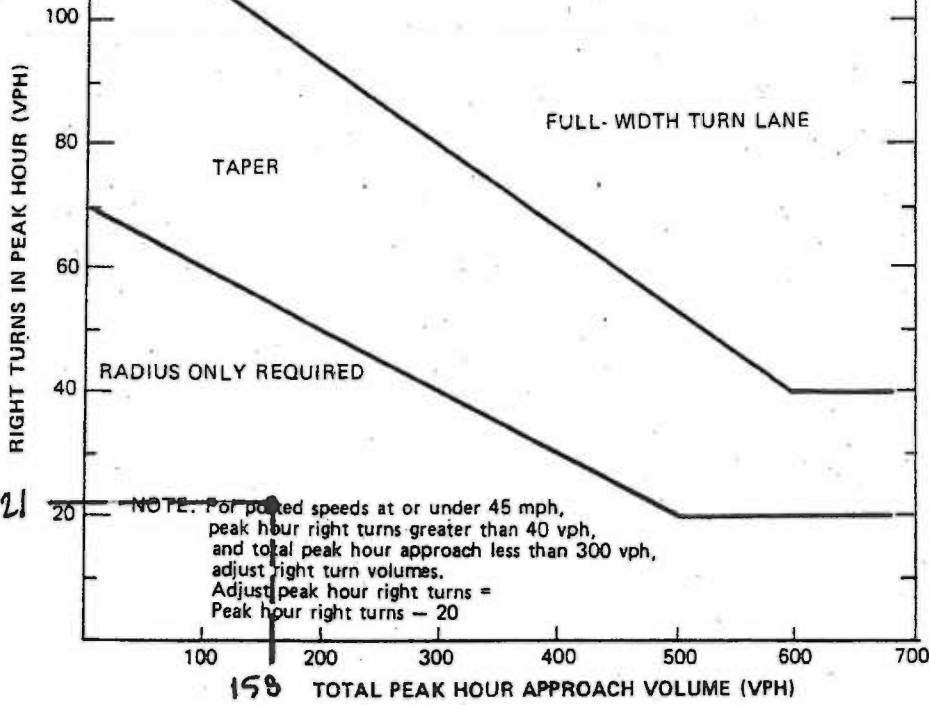
AVERAGE SPEED = 43.5
 50th PERCENTILE = 43.8
 85th PERCENTILE = 48
 90th PERCENTILE = 50.3
 95th PERCENTILE = 51.5

PACE = 38 - 47
 % IN PACE = 62
 VEHICLES IN PACE = 62

SAMPLE VARIANCE = 26.71354
 STANDARD DEVIATION = 5.168515
 RANGE 1* σ = 62
 RANGE 2* σ = 97
 RANGE 3* σ = 100

Frog's Leap Driveway / Cann Creek Rd. (S.R. 128)

2 - LANE HIGHWAYS



SB RIGHT-TURNS S.R. 128

N-D W/END. N-T + Project (worst case)

158 TOTAL PEAK HOUR APPROACH VOLUME (VPH)
SB APPROACH VOLUME S.R. 128

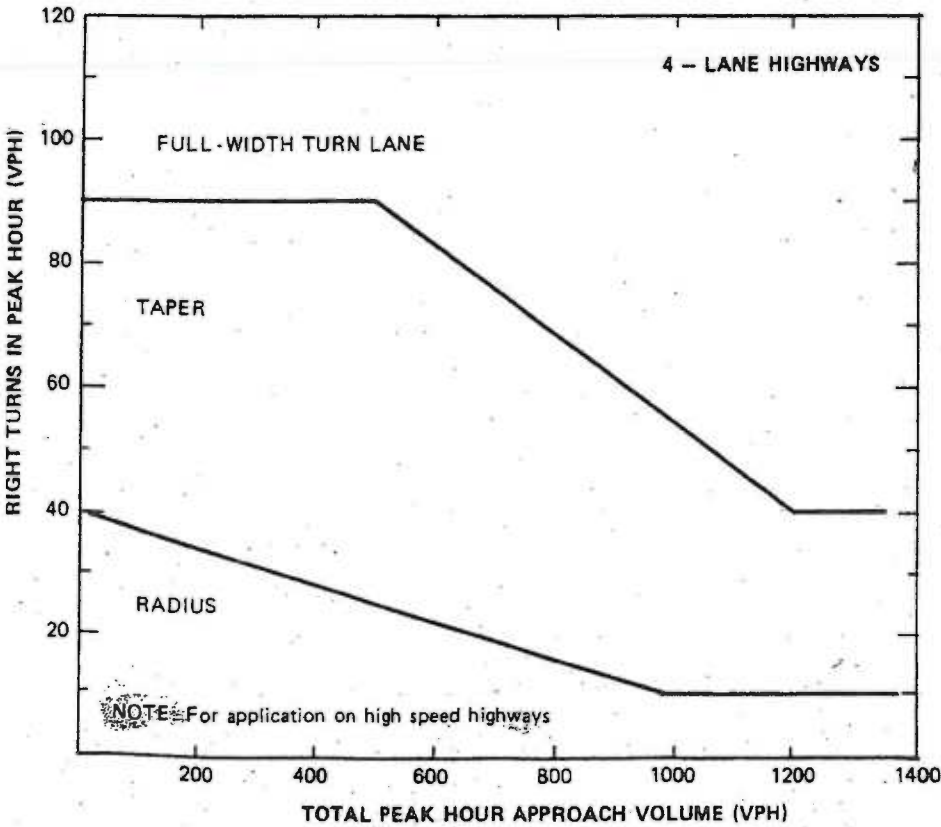
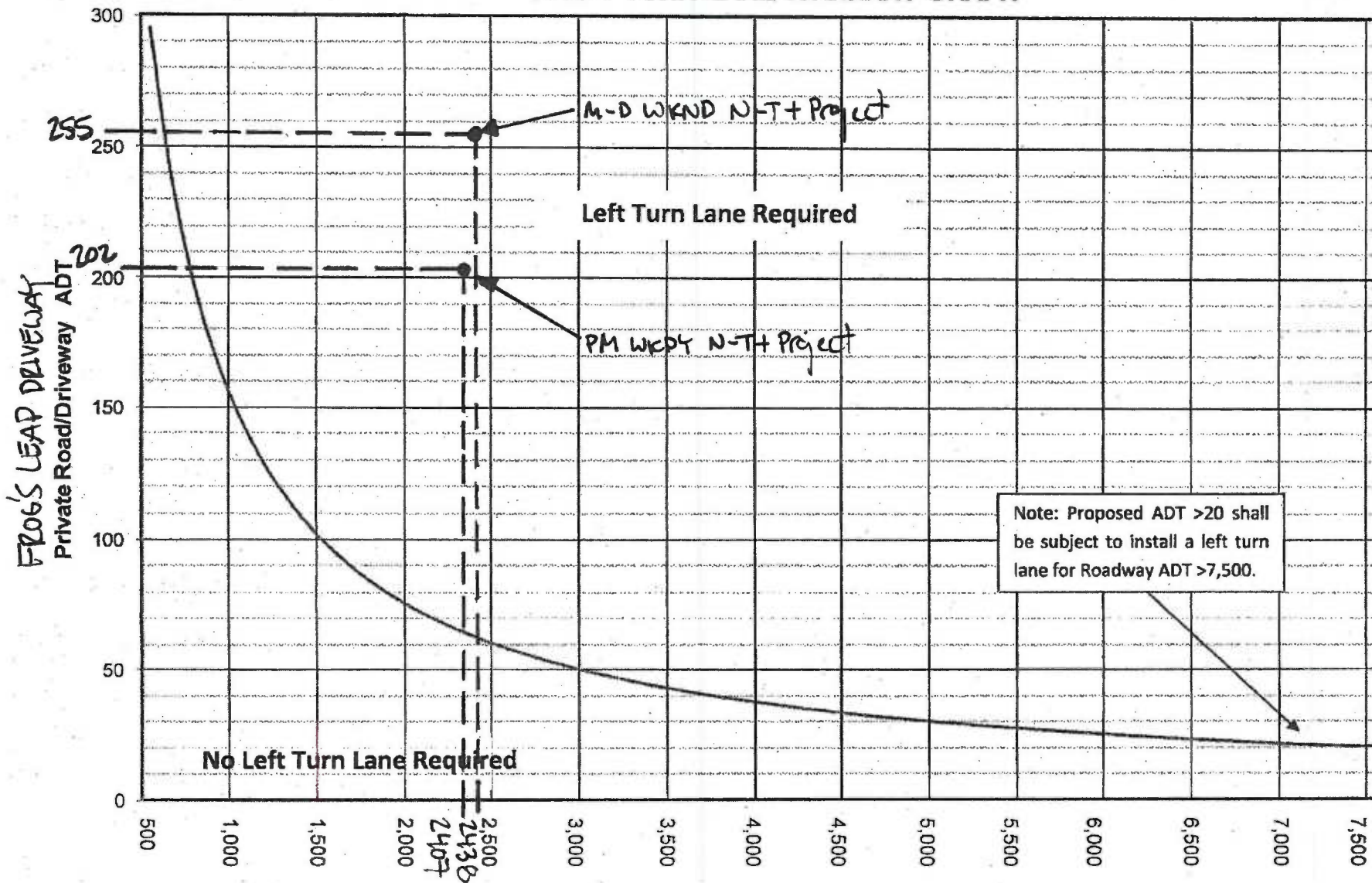


Figure 4-23. Traffic volume guidelines for design of right-turn lanes. (Source: Ref. 4-11)

FROG'S LEAP DRIVEWAY / CONN CREEK ROAD LEFT TURN LANE WARRANT GRAPH

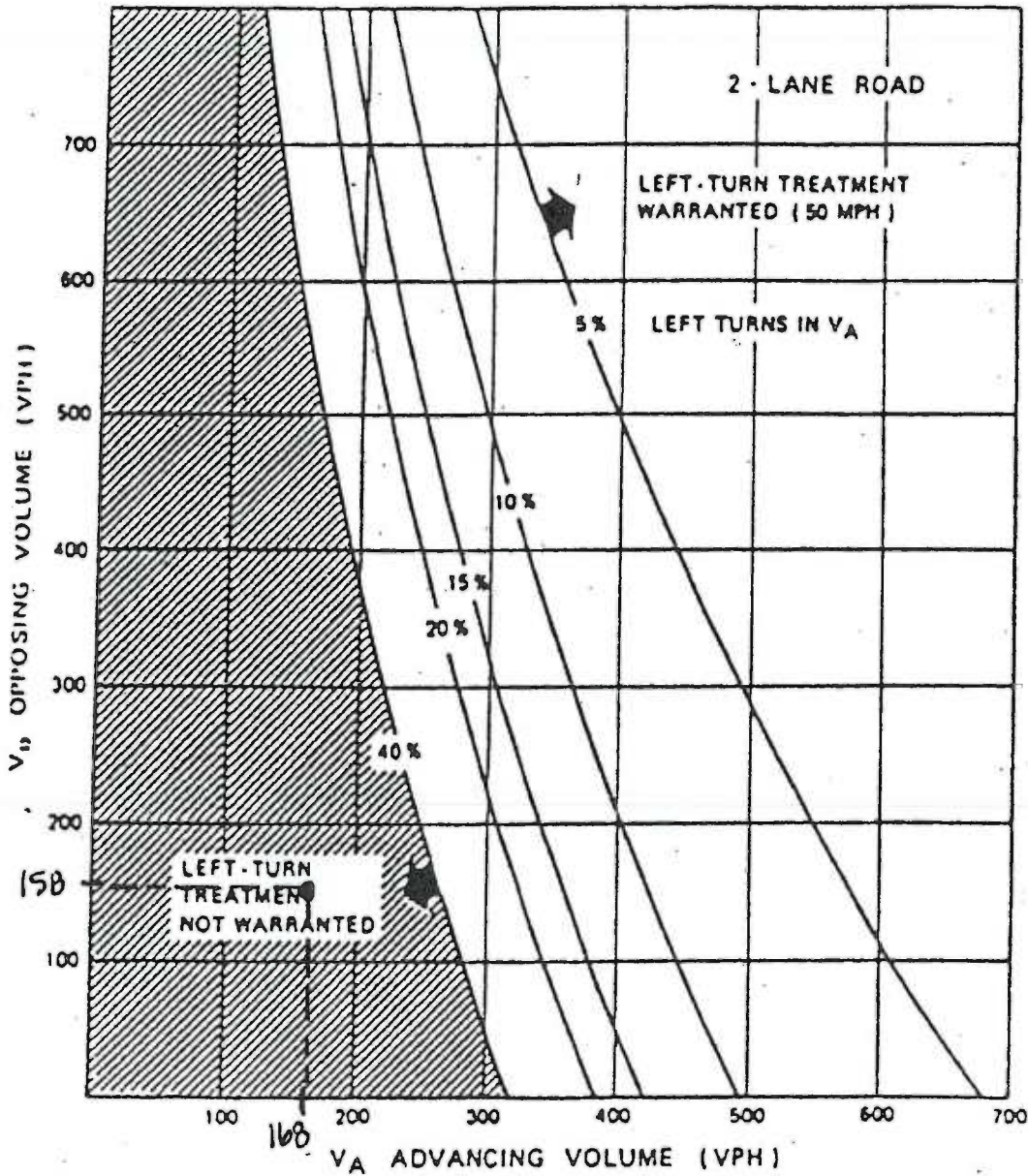


Roadway ADT
CONN CREEK ROAD (S.R. 128)

CALTRAN'S LEFT-TURN WARRANT

FROG'S LEAP DRIVEWAY / CONN CREEK RD. (S.R. 128)

CONN CREEK RD. SB 137 through 21 right-turns



CONN CREEK RD. NB 149 through 15 left-turns



A Tradition of Stewardship
A Commitment to Service

Planning, Building & Environmental Services

1195 Third Street, Suite 210
Napa, CA 94559
www.countyofnapa.org

Pete Parkinson
Interim Director

MEMORANDUM

To: Shaveta Sharma, Planning Division	From: Peter Corelis, Engineering and Conservation Division <i>P.S.C.</i>
Date: October 23 rd , 2014	Re: Frog's Leap Ag. Processing Facility Use Permit: P14-00054 APN: 030-090-033

The Engineering Division received a submittal of a proposal for a major modification to a use permit generally requesting the following:

To approve the use of a new 2,902 square foot combined agricultural processing facility (APC) and tasting room with an attached restroom and porch. The facility will be used to process fruit not associated with wine production and serve an expanded marketing and visitation plan and an increase in employees. The proposed project is located at 8815 Conn Creek Road in the County of Napa.

The Engineering Division reviewed the submitted August 13th, 2014 submission of the left turn lane exhibits and request for an exception to the Napa County Road and Street Standards (NCRSS). The submitted information has shown that a left turn lane mitigation is required by County development standards due to the increase in average daily trips (ADT) to and from the facility. The exception request concerns roadway improvements on land owned and operated by the State of California under the authority of Caltrans. Site constraints and findings interfering with design standards for a left turn lane configuration must be addressed through the permittee of the left turn lane improvements. Please direct design exception requests to Caltrans for an equivalent mitigation.

Should you have any questions of me, please feel free to contact me at (707) 259-8757 or peter.corelis@countyofnapa.org

Queuing and Blocking Report
Vehicle Queuing Report

MD WKND. N-T+Prj. Conditions
12/4/2014

Intersection: 1: Frog's Leap & Conn Creek Rd.

Movement	EB	EB	WB
Directions Served	L	R	LT
Maximum Queue (ft)	55	55	53
Average Queue (ft)	17	27	6
95th Queue (ft)	49	58	30
Link Distance (ft)	4660		3454
Upstream Blk Time (%)			
Queuing Penalty (veh)			
Storage Bay Dist (ft)		25	
Storage Blk Time (%)	2	3	
Queuing Penalty (veh)	0	1	

Network Summary

Network wide Queuing Penalty: 1

Intersection: 1: Frog's Leap & Conn Creek Rd.

Movement	EB	WB	NS
Directions Served	L	R	LT
Maximum Queue (ft)	69	52	39
Average Queue (ft)	18	25	3
95th Queue (ft)	53	57	20
Link Distance (ft)	4660		3454
Upstream Blk Time (%)			
Queuing Penalty (veh)			
Storage Bay Dist (ft)		25	
Storage Blk Time (%)	2	2	
Queuing Penalty (veh)	0	0	

Network Summary

Network wide Queuing Penalty: 1

http://napavalleyregister.com/news/local/dui-stats-show-that-wine-country-loves-beer/article_28f78957-8b1e-57d2-9b7f-2fae356368f7.html

law enforcement

DUI stats show that wine country loves beer

CHLOE F. JOHNSON cjohnson@napanews.com Jul 25, 2015



Submitted image

Nine local law enforcement agencies have waged an "Avoid the Nine" campaign to deter DUI drivers.

Wine may be the alcoholic beverage that has made Napa Valley famous, but beer is what the locals most prefer to drink.

That's one of the latest information nuggets from a survey of people arrested locally for suspected Driving Under the Influence (DUI).

The survey found that most arrests for drunk driving are of beer drinkers, and most of them are Napa locals, not tourists -- facts that run counter to many people's assumptions.

The reasons are many: wineries are careful to prevent their customers from driving drunk, wine drinkers are more likely to have a designated driver or taxi set up, and despite the name "wine country," beer drinkers predominate in Napa, members of the DUI Prevention Coalition report.

According to data compiled by the DUI Prevention Coalition, in 2014 there were 361 beer drinkers arrested for suspected DUI, 174 who had drunk hard liquor, and only 114 who had drunk wine. Overall, beer-related DUI's were more than three times as prevalent as wine-related ones.

"Court records will show that the majority of people arrested are Napa residents," said Napa police Lt. Debbie Peecook. "These are not our visitors."

However, some Napers also say they think that the arrests of local beer drinkers, most of whom work in the service industry, reflect unfair targeting of locals rather than the wine-drinking tourists.

"We were kind of sick of wine," Napa resident Bryan Donovan, who formerly worked as a barback in a winery, said of himself and his co-workers. "We would just go to some dive bar, where they were usually serving beer."

Donovan witnessed the beer-versus-wine divide firsthand when he worked for the winery, and sees it as a class issue, although not one that was being perpetuated deliberately.

"I did see the highway patrol doing a good job doing traffic stops. However, I see more traffic stops in the inner city. It's simply easier. I could see how a local would feel like they're being unfairly scrutinized."

The DUI Prevention Coalition allows those arrested for suspected drunken driving to fill out an optional survey at DUI education classes. Among the questions are the type of alcohol consumed, the place of last drink, and the person's age and occupation.

According to health education specialist Nancy Wynne de Rivera, most survey participants worked in the service industry, mainly as restaurant servers and bartenders. A little over half were between 26 and 46 years old, and most were driving from their homes or friends' houses.

Peecook said the data reflects historic trends in Napa for the past several years, and believes it is mainly because not only do locals stick around town longer than tourists, but that tourists take more precautions and have a different mindset about drinking.

"A lot of them come in buses," Peecook said. "If they come with a bunch of people, they usually come with a designated driver. They know they're drinking."

The Napa wine industry says it tries hard to make sure winery visitors do not drive under the influence.

"We take responsible hospitality very seriously, and encourage our winery members to do the same," said Patsy McGaughy of Napa Valley Vintners, an organization that represents more than 500 wineries. "Since 2008, the Napa Valley Vintners has hosted quarterly workshops to certify local winery staff in responsible beverage service."

McGaughy said that more winery visitors are choosing to take tour buses, taxis, or the VINE bus service rather than driving.

Meanwhile, local Napa drivers continue to work, drive around town, and sometimes have a few beers with friends.

"You're going to drive around town because it's your home town," said Napa resident Maria Lorenzana. "People who make the trip to the wine country probably arrange something."

Some Napans say they believe the disparity is due not to class divides or unequal enforcement, but simply because of bad behavior.

"DUI is DUI," said Napa local Theresa Andrews. "Maybe it's just because locals are being more irresponsible."

Myths and facts

Myths and facts

Myth: The most popular alcoholic drink in Napa is wine

Fact: More Napans drink beer than wine. Beer is cheaper, and is a more popular drink among all Americans, according to Napa Police Lt. Debbie Peecook.

Myth: Most drunk drivers are coming from bars or wineries

Fact: Most DUIs happen to people driving from home or a friend's house. Because of strict responsible beverage service rules in Napa County, there are few drunk drivers coming from drinking establishments, Patsy McGaughy of Napa Valley Vintners said.

Myth: People who have had a lot to drink are more dangerous on the roads than those who have only had one or two drinks

Fact: People who have only had a couple of drinks are actually the most dangerous because they do not realize their impairment. They are the most likely to run red lights and take other risks on the road. Those who have drunk more are usually aware that they should not drive, Peecook said.

Myth: Most drunk drivers are young people in their teens or early 20s

Fact: The majority of arrests are of people between 26 and 46 years old, according to data from the DUI Prevention Coalition.

Transportation Setting

Operational Analysis

Study Area and Periods

The study area consists of the SR 128/Conn Creek Road intersection as well as SR 128/SR 29 and SR 128-Conn Creek Road/Silverado Trail. Due to the unusual configuration of SR 128/Conn Creek Road, it was analyzed as three intersections as follows:

1. SR 128/Conn Creek Road (north)
2. SR 128/Conn Creek Road (south)
3. SR 128/Conn Creek Road (west)
4. SR 128/SR 29
5. SR 128-Conn Creek Road/Silverado Trail South

Operating conditions during the weekday p.m. and weekend midday peak periods were evaluated as these time periods reflect the highest traffic volumes area-wide and for the proposed project. The evening peak hour occurs between 4:00 and 6:00 p.m. and typically reflects the highest level of congestion of the day during the homeward bound commute, while the weekend midday peak occurs between 12:00 noon and 2:00 p.m.

Study Intersections

SR 128/Conn Creek Road (north) is an unsignalized tee intersection with the SR 128 southbound and eastbound approaches being free and the northbound Conn Creek Road being stop controlled.

SR 128/Conn Creek Road (south) is an unsignalized tee intersection with the Conn Creek Road southbound approach being stop controlled and the SR 128 eastbound and Conn Creek Road northbound approaches being free.

SR 128/Conn Creek Road (west) is an unsignalized tee intersection with the SR 128 eastbound and southbound approaches being free and the Conn Creek Road northbound approach being stop controlled.

SR 128/SR 29 is an unsignalized four-legged offset intersection with the eastbound and westbound legs stop-controlled while the SR 29 approaches are free.

SR 128-Conn Creek Road/Silverado Trail South is a four-legged intersection with stop-controlled eastbound and westbound Conn Creek Road approaches while the Silverado Trail approaches are free. The westbound approach is a driveway that provides access to Rutherford Ranch.

The locations of the study intersections and the existing lane configurations and controls are shown in Figures 1 and 2.

Collision History

The collision history along SR 128 in the vicinity of the project site was reviewed to determine any trends or patterns that may indicate a safety issue. Collisions that occurred within one-half mile on either side of the project site during a five-year period between January 1, 2007, and December 31, 2011, were included in the analysis. Collision rates were calculated based on the collision data available from the California Highway Patrol as published in their Statewide Integrated Traffic Records System (SWITRS) reports. The calculated collision rate for the study segment was compared to the average

collision rate for similar facilities statewide, as indicated in *2009 Accident Data on California State Highways*, California Department of Transportation (Caltrans).

Over the five-year study period, five collisions were reported for a calculated collision rate of 1.10 collisions per million vehicle miles (c/mvm). The average statewide collision rate for a two-lane rural roadway with a speed limit equal to or less than 55 mph is 1.09 c/mvm. It should be noted that the calculated collision rate was slightly higher than the statewide average for similar facilities. ~~None of the collisions reported involved an injury; however, one fatality, which involved an intoxicated driver, was reported along this segment during the five-year period studied.~~

The collision data was further examined to determine any apparent trends in collision types. Of the five collisions reported along the segment, four collisions (including the fatal collision) were single-vehicle crashes involving a fixed object. The primary collision factor associated with single-vehicle crashes, again including the fatal crash, was "driving under influence." Additionally, a head-on collision was reported at the SR 128/Conn Creek Road intersection with "auto right-of way violation" being the primary collision factor. The collision rate calculations are provided in Appendix A.

Alternative Modes

Bicycle Facilities

The *Highway Design Manual*, Caltrans, 2012, classifies bikeways into three categories:

- *Class I Multi-Use Path*: a completely separated right-of-way for the exclusive use of bicycles and pedestrians with cross flows of motorized traffic minimized.
- *Class II Bike Lane*: a striped and signed lane for one-way bike travel on a street or highway.
- *Class III Bike Route*: signing only for shared use with motor vehicles within the same travel lane on a street or highway.

In the project area, Class II bike lanes exist on both sides of Conn Creek Road south of the project driveways. Bicyclists ride in the roadway along SR 128 within the project study area. Per the *Napa County Bicycle Plan*, Napa County Transportation and Planning Agency, 2012, long range plans include providing Class II bike lanes on SR 128.

SEGMENT COLLISION RATE CALCULATIONS

Traffic Impact Study for Caymus Winery in Napa County

Location: SR 128 near Rutherford Road/Conn Creek Road

Date of Count: 2010 and 2011 Counts from Caltrans
ADT: 2,500

Number of Collisions: 5
Number of Injuries: 0
Number of Fatalities: 1
Start Date: January 1, 2007
End Date: December 31, 2011
Number of Years: 5

Highway Type: Conventional 2 lanes or less
Area: Rural
Design Speed: <=55
Terrain: Flat

Segment Length: 1.0 miles
Direction: East/West

Number of Collisions x 1 Million				
ADT x 365 Days per Year x Segment Length x Number of Years				
2,500	x	365	x	1
				x 5
				x 1,000,000
				x 5

	Collision Rate	Fatality Rate	Injury Rate
Study Segment	1.10 c/mvm	20.0%	0.0%
Statewide Average*	1.05 c/mvm	2.4%	38.0%

ADT = average daily traffic volume
c/mvm = collisions per million vehicle miles
* 2009 Collision Data on California State Highways, Caltrans

Finding: The intersection of SR 128/SR 29 currently operates at an unacceptable LOS E during the weekend midday peak hour. The County of Napa has expressed in its General Plan that widening of the highway is not encouraged, though Policy AG/LU-98 identifies the need for improvements at the intersection (called Highway 29 and Rutherford Cross Road in the General Plan) to improve safety and accessibility. Further, it has been determined that roundabouts are infeasible where SR 29 is paralleled closely by the Wine Train tracks, as is the case at SR 128/SR 29.

Recommendation: The County should include improvements to SR 128/SR 29, as called for in the General Plan, in the traffic impact fee structure that is currently being developed in accord with Action Item CIR-19.1.

Roadways

Information in the *Napa County General Plan Update Environmental Impact Report, 2008 (GPUFEIR)*, indicates that under 2003 volumes SR 29 was operating at LOS F between Rutherford Road and Oakville Cross Road (this is the nearest segment included in the analysis). Silverado Trail between Sage Canyon Road and Yountville Cross Road is identified in the same document as operating at LOS C under 2003 volumes. Both SR 29 and Silverado Trail are shown as two-lane Rural Throughways on the Circulation Map (Figure CIR-1).

Cumulative Conditions

Cumulative operating conditions were determined with traffic for other approved projects in the same vicinity added to existing volumes. As directed by County staff, the following projects were included to evaluate Cumulative Conditions.

- *Frog's Leap Winery* – revised use permit with 240,000 gallons of production annually; 30 full-time and five part-time employees on weekdays, 10 full-time and five part-time employees on weekends; 125 visitors on weekdays, 300 visitors on weekends; two trucks per day on weekdays and weekends
- *Frank's Family Vineyards Winery* – use permit update with 475,000 gallons of production annually, 14 full-time employees, 5 part-time employees and 50 visitors per day

Project volumes for Frog's Leap were taken from the *Revised Focused Traffic Analysis for the Proposed Frog's Leap Winery Modifications Project, December 15, 2014* by Omni-Means and were calculated for Frank's Family Vineyards Winery using the County's trip generation form. These volumes were added to volumes for Existing Conditions to achieve Cumulative Conditions volumes.

Under Cumulative Conditions the study intersections, with the exception of SR 128/SR 29, are expected to continue to operate acceptably at LOS A or B overall during both peak hours. At SR 128/SR 29, the eastbound and westbound approaches are expected to operate at LOS F during all peak hours. The eastbound and westbound approaches at SR 128-Conn Creek Road/Silverado Trail South are expected to operate at LOS F during the weekday peak hour and at LOS D or better during the weekend peak hour. The Cumulative volumes are shown in Figures 1 and 3, and the resulting levels of service are summarized in Table 3.