"G"

Wastewater Feasibility Study



RSA+ | CONSULTING CIVIL ENGINEERS + SURVEYORS +

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#4116014.0 October 12, 2016

Kim Withrow Napa County 1195 Third Street, Suite 201 Napa, CA 94559

RECEIVED

NOV 08 2016

Napa County Planning, Building & Environmental Services

RE:

B Cellars Vineyards and Winery Supplemental Septic Design Report

APN 031-070-026

This supplemental letter illustrates that the existing septic wastewater treatment system is capable of treating and dispersing the combined flows of the existing residence and winery with the proposed increase in visitation and employees.

The existing system as described in the attached Septic System Design Report, prepared by Riechers Spence & Associates, dated March 21, 2014, has a treatment and dispersal capacity of 2,913 gpd with an allowable storage on event days of 590 gallons per day.

The proposed modifications will result in a typical peak daily flow of 1,973 gpd and a large event day peak flow of 3,443 gpd, as shown in the table below. The daily peak flow of 1,973 gpd is within the system's treatment and dispersal capacity of 2,913 gpd. The peak flow expected on an event day was reduced from the existing system design parameters of 3,505 gpd to 3,443 gpd.

Table 1. Projected Wastewater Flows for Use Permit Modification

Use	Source	Number	Projected Flow (gpd)	Typical Day Flow (gpd)	Total Flow Large Event (gpd)
	Full-time employees	12	15	180	180
~	Part-time employees	7.5	15	113	112.5
WINERY	Part-time event employees	4	15		60
S	Visitors	80	3	240	**
	Event Day Visitors	50	3		150
	Private Promotional w/ meals (catered)	150	10		1500
	Proposed 2 Bedroom Guest House	2	120	240	240
	Existing Residence	10	120	1200	1200
	Grand Total		Total Peak Flow	1973	3443

We conclude that the existing system meets the treatment and dispersal needs of the proposed Use Permit modification.

Respectfully,

Bruce Fenton, P.E. Project Manager

MS/sb

Encl.



SEPTIC SYSTEM DESIGN REPORT

VINTAGE OAKVILLE CROSS RESIDENCE 703 OAKVILLE CROSS ROAD NAPA, CALIFORNIA

APN 031-070-026



Prepared for:

Vintage Oakville Cross, LLC c/o American Fidelity Realty 2000 North Classen Boulevard Oklahoma City, OK 73106

February 5, 2014 *Revised March 21, 2014* 4113067.0



SEPTIC SYSTEM DESIGN REPORT VINTAGE OAKVILLE CROSS RESIDENCE

TABLE OF CONTENTS

INTRODUCTION AND PROJECT DESCRIPTION	1
SITE EVALUATION	1
WASTEWATER VOLUME ESTIMATE	2
WASTEWATER TREATMENT SYSTEM DESIGN	2
CONCLUSION	6

Appendices

- 1. Vicinity Map & USGS Quad Map
- 2. Site Evaluation
- 3. Septic System Calculations



INTRODUCTION AND PROJECT DESCRIPTION

The owner intends to demolish an existing 3 bedroom residence and replace it with a new 10 bedroom residence in the same location. The residence is located on a 11.53 +/- acre parcel, APN 031-070-026 at 703 Oakville Cross Road, Napa. As well as the proposed 10 bedroom residence there is a 3 bedroom farm labor dwelling and a 45,000 gallon per year winery on the parcel. The topography on the parcel is mostly flat on the valley floor and has one knoll, which is where the existing residence is located. Three wells exist on the property. Appendix 1 contains a Site Location Map and a USGS Quad Map showing the parcel topography, features and boundary.

This report is for the addition of a septic tank and a subsurface drip dispersal lines to the existing wastewater system to accommodate the increase in bedrooms.

EXISTING SEPTIC SYSTEM

Information from Napa County Environmental Management files for the parcel shows an existing septic system subsurface drip septic system that serves the existing 3 bedroom residence, the existing 3 bedroom farm labor dwelling and the sanitary flows from the existing winery. The main residence gravity flows to the field while the farm labor dwelling and the winery are pumped to the field.

SITE EVALUATION

A site evaluation was conducted on May 28, 2013 by Lisa Blanc from Riechers Spence & Associates and observed by Rebecca Setliff of Napa County Environmental Management. Appendix 2 contains the Site Evaluation results. This report shows that the test pits contain silty clay and that the pits have at least an acceptable soil to a depth of 24".

A site evaluation was also conducted on June 8, 2012 by Lisa Blanc from Riechers Spence & Associates and observed by Sheldon Sapoznik of Napa County Environmental Management. Appendix 2 contains the Site Evaluation results. This report shows that the test pits contain silty clay loam and that the pits have at least an acceptable soil to a depth of 24".

Another site evaluation was done on the property on January 13, 2004 by Pridmore, and approved by Kim Withrow. These test pits were utilized in the design of the proposed system. The report shows that the test pits contain silty loam and that each pit has an acceptable soil to a depth of 48". Although this area has a greater application rate, the more conservative, silty clay loam, will be used for design purposes. Appendix 2 contains the Site Evaluation results.

Primary dispersal area for the residences and winery are located in the area represented by test pit #'s 1, 2, 3, and 4 from the 2004 site evaluation. The reserve area for the residences and winery are located in the area represented by test pit #'s 1, 2, 3, and 4 from the 2013 site evaluation. All test pits for the primary dispersal area have greater than the required 30" soil



depth. Sanitary wastewater will be pretreated to meet Napa County Requirements for pretreated effluent prior to dispersal.

WASTEWATER VOLUME ESTIMATE

The estimated peak wastewater volume is based on Napa County Environmental Health design standard of 120 gallons per day (gpd) per bedroom for a residence with low flow plumbing fixtures. The existing residence is 3 bedrooms and the proposed residence is 10 bedrooms, including study and exercise room which is an increase in bedrooms of 7. This equates to an additional 840 gpd for the proposed 10 bedroom residence.

WASTEWATER TREATMENT SYSTEM DESIGN

Wastewater will undergo primary and pre-treatment in three Hoot H-1000 Aerobic Treatment Systems. Final disposal from the Hoot system will be to a subsurface Geoflow Drip Dispersal system. The treatment goal is to meet Napa County discharge limits for discharge of pre-treated effluent to a subsurface drip dispersal system of 30 mg/l BOD $_{\rm S}$ and 30 mg/l TSS. The following tables are summaries of the flows for the existing septic system flows and the proposed septic system flows.

Table 1 - Existing Flows

Use	Source	Number	Projected Flow (gpd)	Typical Day Flow (gpd)	Total Flow Large Even (gpd)	
	Full-time employees	8	15	120	120	
R	Part-time harvest employees	5	15	75	75	
WINERY	Part-time event employees	4.5	15		67.5	
3	Visitors	60	3	180	180	
11.2.3	Private Promotional w/ meals (catered)	150	10		1500	
	Existing Farm Labor Dwelling	3	120	360	360	
- 11	Existing Residence	3	120	360	360	
G	rand Total	Total Peak Flow	1095	2663		



Table 2 - Proposed Flows

Use	Source	Number		Typical Day Flow (gpd)	Total Flow Large Event (gpd)
	Full-time employees	8	15	120	120
RY	Part-time harvest employees	5	15	75	75
WINERY	Part-time event employees	4.5	15		67.5
3	Visitors	60	3	180	180
	Private Promotional w/ meals (catered)	150	10	/	1500
	Existing Farm Labor Dwelling	3	120	360	360
	Proposed Residence	10	120	1200	1200
G	rand Total		Total Peak Flow	1935	3503

Hoot Aerobic Treatment System (H-1000 BNR)

The existing system consist of an 8,000 gallon septic/pump tank for the winery, a 1,200 gallon septic tank for the farm labor dwelling and a 2,000 gallon septic tank for the existing residence. Per Napa County Code, Table 13.44.020, a 10 bedroom residence requires a 2,500 gallon septic tank. Because there is already a 2,000 gallon septic tank at the residence, a 750 gallon holding tank will be added after the septic tank to achieve the same retention time as a 2,500 gallon tank would provide. All tanks flow into a 5,000 gallon equalization tank, and then to 3 - 1,000 gallon per day Hoot Aerobic Treatment System tanks (H-1000 BNR) and final distribution to a Geoflow subsurface drip dispersal field. The HOOT tanks operate in parallel with each other.

The existing design loading rate for the septic system is 2,313 gpd and stores 350 gallons from a large event at the winery in the equalization tank. The system doses a maximum of 2,313 gallons to the drip dispersal field. The proposed residence will add 7 bedrooms, which equates to 840 gpd. Due to the increase in bedroom count, the design loading rate will be increased to dose a maximum of 2,913 gpd to the dispersal field and will require the large winery events to store 590 gpd in the equalization tank.

System sizing, tank sizing, and treatment system settings are based on HOOT manufacturer's specifications to achieve the design treatment goals of 30 mg/l BOD₅ and 30 mg/l TSS. Pump sizing, timer settings and treatment system calculations are found in Appendix 3 of this report.

Geoflow Drip Dispersal System

The Geoflow field and reserve area are currently located as shown on sheet SS2.0 of the attached plans. Soil depth is at least 30 inches in the primary dispersal field area and will require no fill placement. The reserve dispersal field areas have a soil depth of at least 24" and will require 6 inches of silty clay fill.



The most restricting soil horizon for the primary area is silty clay loam with moderate sub-angular blocky structure. Referring to Table 2 of the Geoflow Design, Installation and Maintenance Guidelines, a Geoflow system installed in silty clay loam soil with moderate, sub-angular blocky structure will accept 0.6 gpd/sf/day. For a total daily flow of 2,913 gpd this equates to base dispersal area of 4,855 square feet.

Drip Dispersal Field Area =
$$\frac{2,913 \text{ gpd}}{0.6 \text{ gpd/SF}}$$
 = 4,855 square feet

The Geoflow field will therefore need to consist of 2,428 lineal feet of drip line. The existing system consists of 1,952 lineal feet of drip line, therefore an additional 476 lineal feet will have to be added to accommodate the proposed residence. Four additional lines at 122' will be added to the system. The new lines will be oriented along the contours and attached to the existing supply and flush manifolds as shown on sheet SS2.0 of the attached plans. The field is separated into two zones. Both fields currently consist of 8 - 122 lineal feet of drip line. Each zone will add 2 - 122 lineal feet of drip line. Each zone will have 8 existing and 2 new, 122 foot geoflow drip lines lines.

In addition to the primary dispersal area of 4,855 square feet, a 200% reserve area is required. A 200% reserve area has already been proven for the existing residences and winery and therefore will be only for the additional daily discharge from the proposed residence, 600 gpd. The reserve area will be located where the soil application rate is of 0.6 gallons/square foot/day.

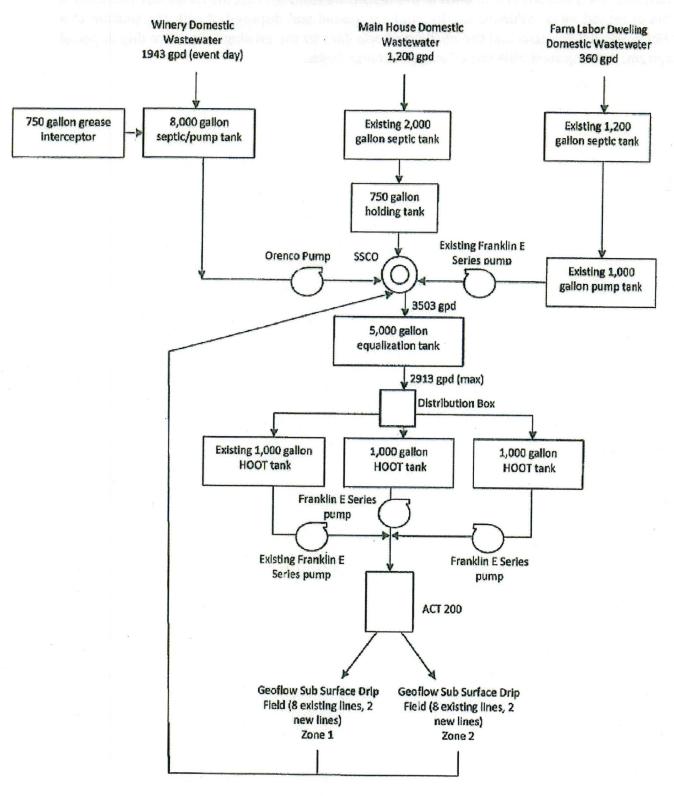
Drip Dispersal Field Area (reserve area) =
$$\frac{600 \text{ gpd}}{0.6 \text{ gpd} / SF} \times 200 \% = 2,000 \text{ square feet}$$

The total requirement for additional wastewater reserve dispersal area is 2,000 square feet.

All monitoring wells that are currently installed will cover the additional lines. Geoflow field calculations are found in Appendix 6 of this report. Figure 1 shows a schematic of the process wastewater system.



FIGURE 1
PROCESS FLOW DIAGRAM
SANITARY WASTEWATER





CONCLUSION

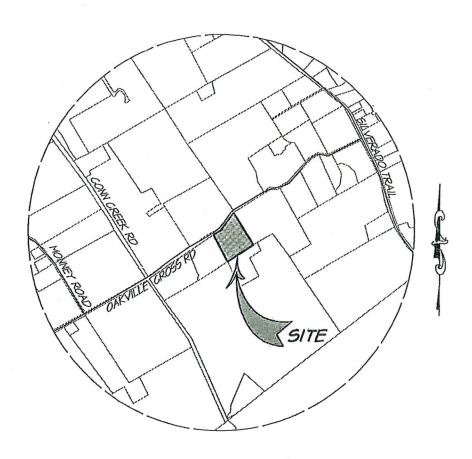
Based on the information contained in this report, we conclude that the additional bedrooms in the proposed main residence can be properly treated and disposed of with the addition of a 750 gallon holding tank and the addition of drip lines to the existing subsurface drip dispersal system, meeting applicable Napa County discharge limits.



Appendix 1

Vicinity Map USGS Quad Map

VINTAGE OAKVILLE CROSS RESIDENCE VICINITY MAP OAKVILLE CALIFORNIA

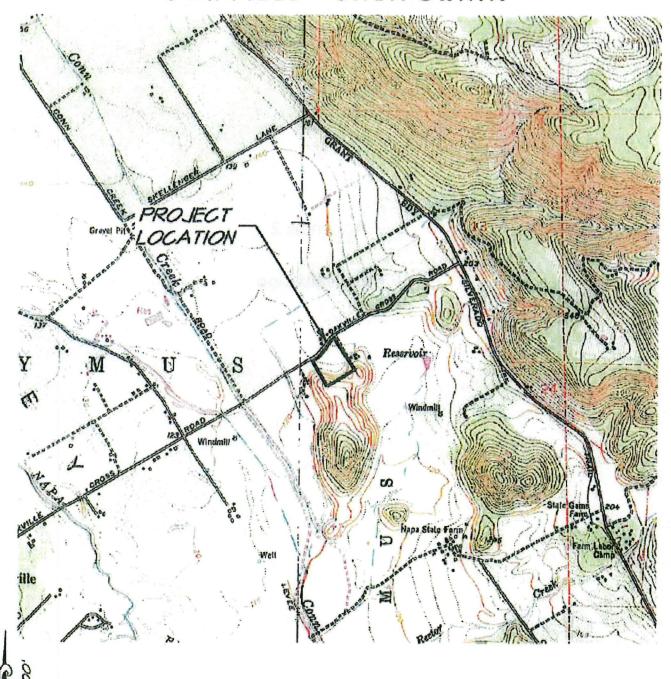


SCALE: I" = 2000'



VINTAGE OAKVILLE CROSS RESIDENCE USGS QUAD MAP

OAKVILLE CALIFORNIA





S 1515 Fourth Street Napa, Calit. 94559 v 707.252.3301 f 707.252.4966 JANUARY 16, 2014



Appendix 2

Site Evaluation

hrysta

864107-15148

REQUEST FOR SITE EVALUATION INSPECTION

ENVIRONMENTAL HEALTH DEFT. USE ONLY
PRE: -318.00 FARCEL NUMBER: 31-070-26
DATE: 1 13 04 JOB ADDRESS: 701 Charille Crashettal
RECEIPT: 32986 ONNER: Gira Willer
BY: TEST CONDUCTED BY: Thiduse
TYPE OF TEST: YIELD ANALYSIS FERCOLATION TEST
To be run on 6 14 st 1:00 aufpm To be run on fromau/pn to pu
The state of the s
PURPOSE OF TEST: HOUSE: WINERY: X OTHER:
PROJECTED WASTEWATER FLOWS: 10,000 gpy = 500 gpd + dom gpd
PERCULATION TEST INSPECTION RESULTS
Pre-soak checked? yes no Length of pre-soaks
Checked by: Date:
Rate at time of inspections Stabilized perc rates
Gravel and Pipe Used? yes no If so, take the perc rare x .6 = in/hr
Graver and ripe used; yes no it so, take the jett take x

TYPE OF SYSTEM APPROVED
TYPE OF STRIEN APPROVED
TYPE OF STRIEN APPROVED
STANDARD SYSTEM Acceptable soil to: UD" / Assigned perc range: 1-3 / 3-6) / 6-12 Depth of trenchest 24" / Rock under pipes 12" ov 9 36 over over rock: 12"
STANDARD SYSTEM Acceptable soil to: UD" / Assigned perc range: 1-3 / 3-6 / 5-12 Depth of trenches: 24" / Rock under pipes 12"0V \$36 over over rock: 12" Lineal feet of leachline required: TBD / Plot plan received: U-14-04
TYPE OF STETEN APPROVED STANDARD SYSTEM Acceptable soil to: UD" / Assigned perc range: 1-3 / 3-6 / 5-12 Depth of trenches: 24" / Rock under pipes 12" OV \$36 Cover over rock: 12" Lineal feet of leachline required: TBD / Flot plan received: U-14-04 Slope: 5-10%/ Surface drainage problems:
TYPE OF STETEN APPROVED STANDARD SYSTEM Acceptable soil to: UD" / Assigned perc range: 1-3 / 3-6 / 5-12 Depth of trenches: 24" / Rock under pipes 12" OF GOVER over rock: 12" Lineal feet of leachline required: TBD / Flot pies received: U-14-04 Slope: 5-10%/ Surface drainage problems: Additional information: Prohibit harses from gang Mto 44,54cm; Stang.
STANDARD SYSTEM Acceptable soil to: UD" / Assigned perc range: 1-3 / 3-6 / 5-12 Depth of trenches: 24" / Rock under pipe: 12" or \$636 Lineal feet of leachline required: TBD / Plot plan received: U-14-04 Slope: 5-10%/ Surface drainage problems: Additional information: Prohibit horses from gang and system; Stary LOWER M WHILE SERVED.
TYPE OF SYSTEM APPROVED STANDARD SYSTEM Acceptable soil to: UD" / Assigned perc range: 1-3 / 3-6 / 5-12 Depth of trenches: 24" / Rock under piper 12" OV \$436 Lineal feet of leachline required: TBD / Plot plan received: U-14-04 Slope: 5-10%/ Surface drainage problems: Additional information: Prohibit horses from gong into system; Stary LOWER M MM (At the Constraints:
STANDARD SYSTEM Acceptable soil to: UD" / Assigned perc range: 1-3 / 3-6 / 5-12 Depth of trenches: 24" / Rock under pipe: 12" or \$636 Lineal feet of leachline required: TBD / Flot pian received: U-14-04 Slope: 5-106/ Surface drainage problems: Additional information: Prohibit horses from gary Mto 445tem; Stary LOWOV M MINIAT FOR CO. SLOPE. SPECIAL DESIGN SYSTEM DUE TO THE FOLLOWING - Size constraints: Perc rate too Blow: /Perc rate too fast: /Steep slope:
STANDARD SYSTEM Acceptable soil to: UD" Assigned perc range: 1-3 3-6 5-12 Depth of trenches: 24" Rock under pipe: 12" OV \$36 Over over rock: 12" Lineal feet of leachline required: TBD Plot plan received: U-14-04 Slope: 5-10% Surface drainage problems: Additional information: Prohibit horses from gong Mto 445term; Story LONCY M MM At the G Slope. SPECIAL DESIGN SYSTEM DUB TO THE FOLLOWING - Size constraints: Perc rate too slow: /Perc rate too fasts ISteep slope: Insufficient soil depth: /High seasonal groundwater:
STANDARD SYSTEM Acceptable soil to: UD" / Assigned perc range: 1-3 / 3-4 / 5-12 Depth of trenches: 24" / Rock under pipe: 12" ov \$6.36 over over rock: 12" Lineal feet of leachline required: TBD / Flot pian received: U-14-04 Slope: 5-106/ Surface drainage problems: Additional information: Prohibit horses from gong Mto 44 Stem; Story LOWOV M MINIAT FOR CO. SLOPE. SPECIAL DESIGN SYSTEM DUE TO THE FOLLOWING - Size constraints: Perc rate too Blow: /Perc rate too fast: /Steep slope:
STANDARD SYSTEM Acceptable soil to: UD" Assigned perc range: 1-3 3-6 5-12 Depth of trenches: 24" Rock under pipe: 12" OV \$36 Over over rock: 12" Lineal feet of leachline required: TBD Plot plan received: U-14-04 Slope: 5-10% Surface drainage problems: Additional information: Prohibit horses from gong Mto 445term; Story LONCY M MM At the G Slope. SPECIAL DESIGN SYSTEM DUB TO THE FOLLOWING - Size constraints: Perc rate too slow: /Perc rate too fasts ISteep slope: Insufficient soil depth: /High seasonal groundwater:

TEXTURE (In the proposed trent	th rope)	1.0179
CLAY CONTENT Core Hole	Core Hole 1 2 3 4 5 6 High (>50) Hod (20-50) X V V V V Low (<20)	GRAVEL, GOBBLE, STOR Core Mole 1 2 3 Very High (>60) High (35-60) Mod (15-35) Low (<15) X Y Y
STRUCTURE		* * * * * * * * * * * * * * * * * * * *
SOIL DENSITY WHEN PICKED (Core Hole pick sluffs or caves soil in pick bites and soil sluffs pick bites/ little or no soil states	1 2 3 6 5 6	CONSISTENCE (Circle w or (a)) Core Hole 1 2 3 4 5 6 Easy Hoderate X Y Y Y Y Y H
STRUCTURE	HODIFIER C	CHARACTERISTICS
Core Hole 1 2 3 4 5 6 Granular	1) Soil Survey Name:	The state of the s
Blocky Prism		
Platy	2) Horizon Boundaries: Diffuse	
Massive Cemented	3) Topography: Concave	
	4) Vegetation: Type NOVOL	M. S. Condition:
******	******	***
HOLE #1 EST.	CORE HOLE RECORD HOLE #2 EST.	HOLE #3 BUT.
0 to 48" soil loam 3-6	O to 54" silt kann 3-6	The state of the s
48" to 16" yocky 2505-		
	54 to 166 rocky lam34	一种和大约的
to	to	toto
Roots: Some at 100 Color: bright / dull,	Roots: Some at top	Roots: 30me at 10
Hater Table: not noted	Water Tables not noted	Color: Gright / duly Water Table: VIO 10000
Dug: may) / hard / dusty /smear Acceptable Soil To: 48"	Dugieasy / bard / dusty / smear Acceptable Soil To: (66"	Dug: 6asy /hard /dusty /smear Acceptable Soil To: 101/5"
		wegeheapte 2011 191 70 .10
HOLE #4 EST.	HOLE #5 A EST.	HOLE #6 BET.
o to 60" silt laun 36	O to lob self loam 3-6	PERC
to	to	to
to	to	to
Roots: At surface Color: bright / dull /	Color: Crishy / dell	Rootes
Hater Tables NOT NOTED	Water Table: Vol noted	Color: bright / doil Water Table:
Acceptable Soil To: (00"	Dug(edsy)/ hard / dusty / smear Acceptable Soil To: 66"	Dugiessy /hard /dusty /essaut Acceptable Soil To:

APN 031-070-026

RSA Project Number: 4112023.0

Date:

June 12, 2012 Page 1 of 4

Napa County Department of Environmental Management

SITE EVALUATION REPORT

Please attach an 8.5" x 11" plot map showing the locations of all test pits triangulated from permanent landmarks or known property corners. The map must be drawn to scale and include a North arrow, surrounding geographic and topographic features, direction and % slope, distance to drainages, water bodies, potential areas for flooding, unstable landforms, existing or proposed roads, structures, utilities, domestic water supplies, wells, ponds, existing wastowater treatment systems and facilities.

Permit#: E12-00298		
AFN: 031-070-026	A. Marie	
(County Use Only) Reviewed by:	Date:	

PLEASE PRINT OR TYPE ALL INFORMATION

Property Owner		I Name of the state of the stat	- P3 4400 - P3				
Elissa Miller		⊠ New Construction □ Other:	on [] Addition [] Remodel [] Relocation				
Property Owner Mailing Address 701 Oakville Cross Road		Residential - #	of Bedrooms: Design Flow: gpd				
City Oakville	State Zip CA 94662	☑ Commercial –	Гуре:				
Sile Address/Location	The state of the s	Sanitary Waste:	800 god Process Waste: 1167 god				
701 Oakville Cross Road Oakville, CA 94562		Other:					
		Sanilary Waste	gpd Process Waste: gpd				
Evaluation Conducted By	:						
Company Name Riechers Spence & Associates	Evaluator's Name Lisa Blanc		Signature (Civil (1991) per R.E.H.S., Geologisi, Soil Scientiet)				
Malling Address: 1515 Fourth Street			Telephone Number 707-252-3301				
City Napa		ip 559					
Primary Area	The state of the s	Expansion Area					
Acceptable Soll Depth: 60 in. To	est pit If's: From Site Eval in 2004	Acceptable Soil Depti	n: 24 in. Test pit #'s: 2, 3, 5, 6				
Soll Application Rate (gal. /sq. ft. /d	fay): 0.33	Soil Application Rate	(gal. /sq. ft. /day): 0.60				
System Type(s) Recommended: C	hambers with Pre Treatment		mmended: Geoflow with Pre Treatment				
Slope: 1-9 %. Distance to neare	est water source: < 100 ft.	Slope: 5-10 %. D	elistance to nearest water source: < 100 ft.				
Hydrometer test performed?	No ☑ Yes ☐ (altach results)	Hydromoter test perfo	rmed? No ⊠ Yes □ (attach results)				
Bulk Density test performed?	No ⊠ Yes ☐ (allach results)	Bulk Density test perfe	ormed? No 🖾 Yes 🖂 (attach results)				
Percolation test performed?	No ☑ Yes ☐ (atlach results)	Percolation test perfor					
Groundwater Monitoring Performed	d? No ⊠ Yes □ (altach results)	Groundwater Monitori	ng Performed? No ☑ Yes ☐ (attach results)				
conducted in 2004.	wasto will be in same location		s from 2009. Site evaluation was evaluation. Reserve area to stay away				

APN 031-070-026

RSA Project Number: 4112023.0

Date:

June 12, 2012 Page 2 of 4

Test Pit # 1

Horizon	Daumdami	0/0			С	onsisten	ce	a Laurenia		1
Depth (Inches)	Boundary	%Rock	Texture	Structure (Grade / Shape)	Side Wall	Ped	Wet	Pores (QTY / Size)	Roots (QTY / Size)	Mottling (QTY / Size/ Contrast)
0-30"	G	0	SIC	S/SB	Н	F	S	F/VF	0	N/A
30"-48"										YES
48"- BOTTOM	GROUND	WATER						1754	Hall Corp.	

Test Pit#

2

Doundon: 0	0/5		Berakeno io	C	onsisten	ce			
Boundary	%Rock	Texture Structure (Grade / Shape)	Side Wali	Ped	Wet	Pores (QTY / Size)	(QTY / Size)	Mottling (QTY / Size/ Contrast)	
G	0	SiC	S/SB	Н	F	s	F/VF	FNF	N/A
						1			YES
GROUND	WATER							Au Auger	l New Lo
		,ag/a (-1.5) (658, 17)	G 0 SiC	G 0 SiC S/SB	Boundary %Rock Texture Structure (Grade / Shape) Side Wall G 0 SiC S/SB H	Boundary %Rock Texture Structure (Grade / Shape) Side Wall Ped Wall G O SiC S/SB H F	G O SiC S/SB H F S	Boundary %Rock Texture (Grade / Shape) Side Wall Ped Wet (QTY / Size) G 0 SiC S/SB H F S F/VF	Boundary %Rock Texture (Grade / Shape) Side Wall Ped Wet (QTY/Size) Roots (QTY/Size) G 0 SiC S/SB H F S F/VF F/VF

Test Pit#

3

Danmalama	Roundary	9/ Book	9/ Pools	9/ Pools	0/ Dools	0/ 0 1-	0/ D = = 1-	0/ 0 1-			C	onsisten	ce			
Воипаату	%ROCK	Texture	(Grade / Shape)	Side Wali	Ped	Wet	Pores (QTY / Size)	(QTY / Size)	Mottling (QTY / Size/ Contrast)							
С	0	SICL	S/SB	Н	F	ss	C/VF-F	M/VF-F	N/A							
						ě		13.30	YES							
GROUND	WATER															
		1896 1702 1960 18	C 0 SICL	C 0 SiCL S/SB	Boundary %Rock Texture (Grade / Shape) Side Wall C 0 SiCL S/SB H	Boundary %Rock Texture (Grade / Shape) Side Wall C 0 SiCL S/SB H F	Boundary %Rock Texture Structure (Grade / Shape) Side Wall Ped Wet C 0 SICL S/SB H F SS	Boundary %Rock Texture Structure (Grade / Shape) Side Wall Ped Wet (QTY / Size) C 0 SiCL S/SB H F SS C/VF-F	Boundary %Rock Texture (Grade / Shape) Side Wall Ped Wet (QTY / Size) Roots (QTY / Size) C 0 SiCL S/SB H F SS C/VF-F M/VF-F							

APN 031-070-026

RSA Project Number: 4112023.0

Date:

June 12, 2012 Page 3 of 4

Test Pit # 4

Horizon Depth (Inches)	Boundary %Rock	0/51	Toytura Chrustura	Consistence				Doots		
		Texture	Structure (Grade / Shape)	Side Wall	Ped	Wet	Pores (QTY / Size)	Roots (QTY / Size)	Mottling (QTY / Size/ Contrast)	
0-26"	G	0	SICL	S/SB	Н	F	ss	C/VF-F	M/VF-F	N/A
26"-48"										YES
48"- BOTTOM	GROUND	WATER							. 12 ⁸ 1 11 1	
								1.		

Test Pit#

5

	Harlman Davidani	Davindami (V David	VD - 1 - 1 - 1 - 1		С	onsisten	ce			
Horizon Depth (inches)	Boundary	%Rock	Texture	Structure (Grade / Shape)	Side Wall	Ped	Wet	Pores (QTY / Size)	Roots (QTY / Size)	Mottling (QTY / Size/ Contrast)
0-30"	G	0	SICL	S/SB	Н	F	SS	C/VF-F	F/VF-F	N/A
30"-58"	<u>~</u>									YES
58"- BOTTOM	GROUND	WATER								8812
				į.	A I	=			i i	À
	. P					×				

Test Pit#

6

11	D	0/7			C	onsisten	ce			
Horizon Depth (Inches)	Boundary	%Rock	Texture	Structure (Grade / Shape)	Side Wali	Ped	Wet	Pores (QTY / Size)	Roots (QTY / Size)	Mottling (QTY / Size/ Contrast)
0-26"	С	0	SICL	S/SB	Н	F	SS	C/VF-F	M/VF-F	N/A
26"-48"	воттом	2		a a					4	YES
		1,								
			3						-	

APN 031-070-026 RSA Project Number: 4112023.0

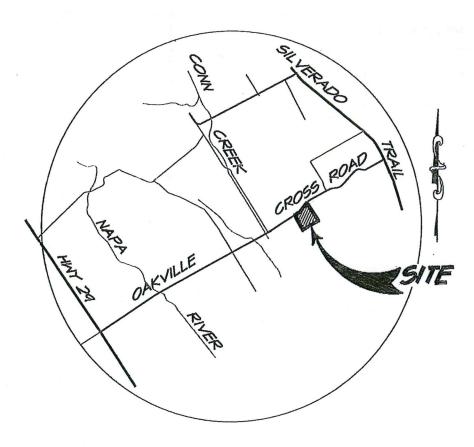
Date:

June 12, 2012 Page 4 of 4

Test Pit#

Horizon Boundary %F		~ .		Consistence		CO .	_			
	%Rock Texture Structure (Grade / Shape)	Side Wall	Ped	Wet	Pores (QTY/Size)	Roots (QTY / Size)	Mottling (QTY / Size/ Contrast)			
0-30"	С	0	SIC	S/SB	Н	F	s	F/F	F/F	N/A
30"-36"										YES
36"- BOTTOM	GROUND	WATER							3	**************************************
						2				
	Page 18 of the State of the Sta						****			

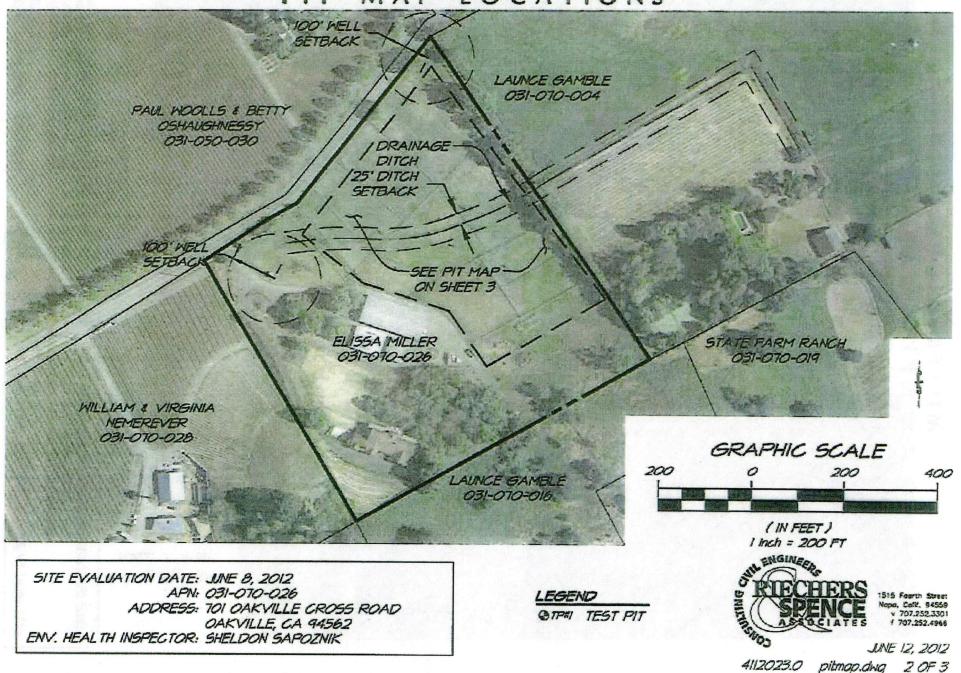
MILLER PROPERTY VICINITY MAP OAKVILLE CALIFORNIA



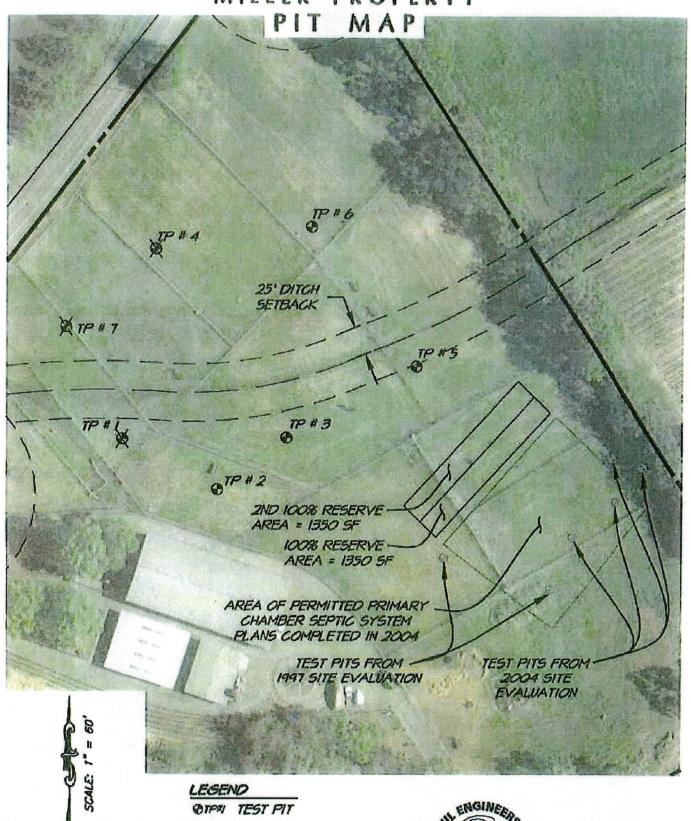
LOCATION MAP



MILLER PROPERTY PIT MAP LOCATIONS



MILLER PROPERTY



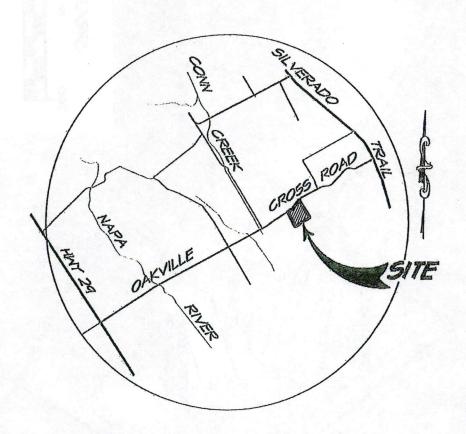
SITE EVALUATION DATE: JUNE 8, 2012 APN: 031-070-026 ADDRESS: 701 OAKVILLE CROSS ROAD OAKVILLE, GA 94562 ENV. HEALTH INSPECTOR: SHELDON SAPOZNIK



1515 Fourth Street Napa, Colif. 94559 v 707.252.3301 f 707.252,4966

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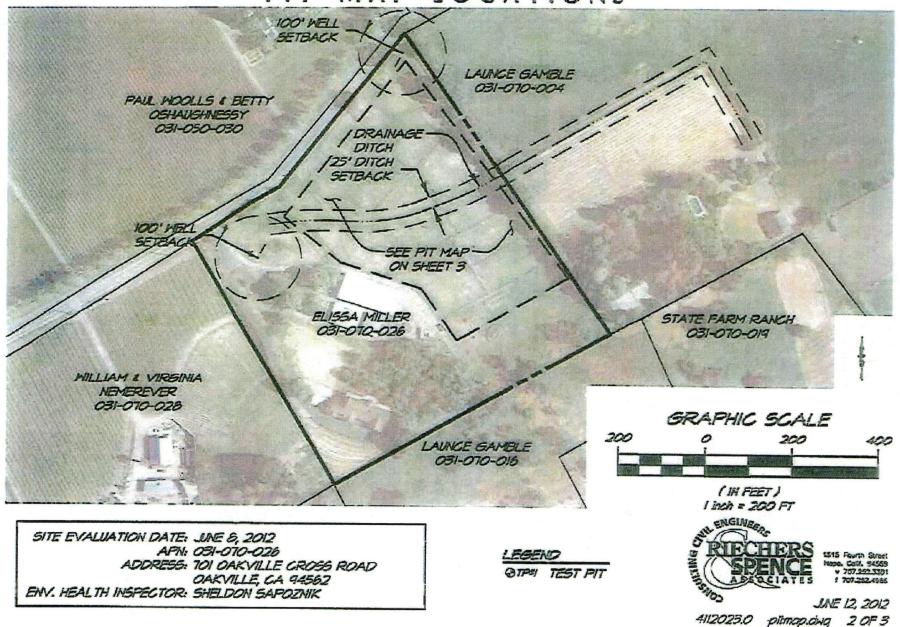
MILLER PROPERTY VICINITY MAP OAKVILLE CALIFORNIA



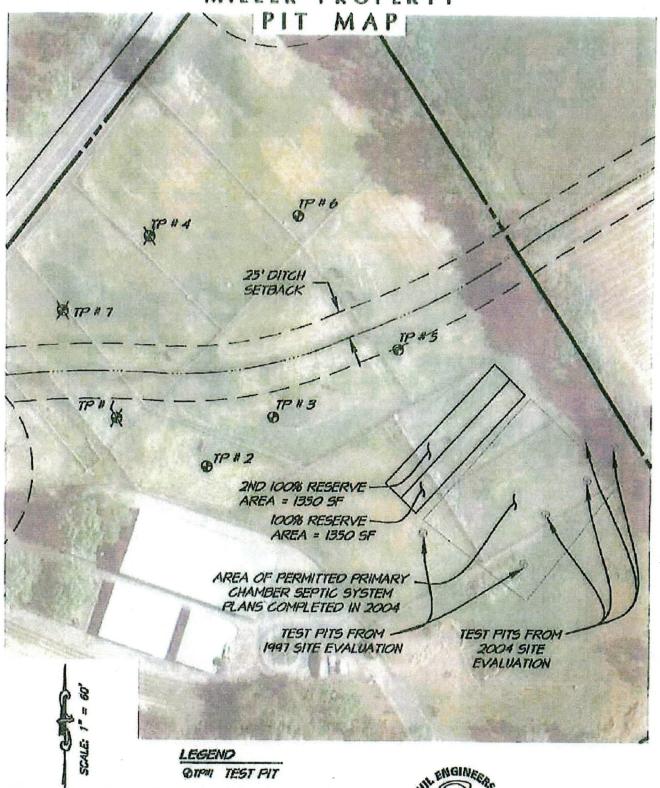
LOCATION MAP



MILLER PROPERTY PIT MAP LOCATIONS



MILLER PROPERTY



SITE EVALUATION DATE: JUNE 8, 2012 APN: 031-070-026 ADDRESS: 701 OAKVILLE CROSS ROAD OAKVILLE, CA 94562 ENV. HEALTH INSPECTOR: SHELDON SAPOZNIK



1515 Fourth Street Napo, Calif. 94859 v 707.252.3301 f 707.252.4966

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APPENDIX 3

Septic System Calculations

	the section of the section of	Field Flow
Job Description	Yintage Oukville Cross Residence	
Contact:	Bruce Feston	
Prepared by:	Lisa Olimic	
Date:	22 Jan-14	
	haded areas and drop down menta; cerves as a guide, and is not a complete bydraulie desi	

Total field

Total Chambiy of affluent to be disposed per day	2,913	gallous / day
Hydraulie loading rate	0.6	gallens / sq.ft. / day
Atinhum Dispersal Field Area	4,855	signisee Ct.
Falul Dispersal Pick! Asya	4,855	square ft.

Flow per zone

Number of Zones		zone(s)
Dispersal area per zone	The second secon	rejume ft.
Chaose has aptions between WASTRINOW lines	1	n.
Choose college que between WASTEFLOW emfilees		n.
Total linear fe per 2020 (minimum required)	1,214	ft. per zone
Total number of enditors per zone	607	emilters per zone
Select Wasteflow dripline (16mm)	Wastellow PC - 1/2gph	THE PERSON NAMED IN COLUMN TWO IS NOT THE OWNER.
Pressure at the beginning of the dripfield	40	pri
Feet of Head at the beginning of the dripfield	92.4	n.
What is the flow rate per emitter in aph?	0.53	uph
Dose Dun per zono	5.36	

Main of his State is a Kromitian complemental translation for the data of the six bank year. For a complement in the other six of the data of the six of t

If required, choose thick velocity	- 2	Il/sec
How many lines of WASTEFLOW per zone?	10	lines
Fill in the actual length of longest dripline lateral	122	n.
Flish flow required at the end of each dripline	1.48	ēbstī
Total Flow required to achieve flushing velocity	[4.80	gpm
Fotal Pino per zone-worst case scenario	20.16	gpm

Select Filters and zone valves

Select Filter Type	BiaDisa Pilier	7 Marian Santa Company
Recommended l'alter (item no.)	BioDisc-150 / Max flow 30 gpm	Select
Select Zone Valve Type	Electric Saleriold	
Recommended Zone Value (frem no.)	SVLVB-100	1-in. Solomid valve

Dosing

Number of dates per day I zone:	48	dotes
Timer ON. Pamp ran time per dossizone:	5.40	enins:sees
Timer OFF. Pump off time between doses	0:24	hrs:mins
Per Zone - Pump sun time per day/zone:	4:31	lu simins
All Zones - Number of doses per day / all zones	96	doses / day
Allow time for field to pressurize	0.00:30	lusiminaisces
Filter flush fimer	0:00:20	lara:mina:sees
Oralo three	0.05:00	bisiminaxecs
Field Bush timer	0.01;00	lus minsusces
Fleid flush counter		cycles
Time required to complete all functions per day	19:59	besendes .
Dase valuina per xano	30	gallons per done

(SEPTION	Pump Size
Job Description:	Vinnago Oakville Cross Residence
Contact:	Bruce Fenton
Peepared by:	Lips Diane
Dates	1/22/2014

Pressure losses may be grossly overstated, particularly if designing with WASTEFLOW Classic The letters on the diagram(right) match the letters in accition 2 below,

Worksheet - Pump Sizing

Flow required to doss field	5.36. gpm
Flow required to flush field	14.80 gpm
Flow required to class & flush field	20.16 npm
Filter	DioDisc-150 / Max flow 30 gpm
No. of Zones	2 zones
Zong valvo	SVLVB-100
Dripline	Wasteflow PC - 1/2gph
Dripline longest lateral	122.00 ft.

Section 2	Froffiead	Pressu	ate
A. Flush line - Losses through return line			
Size of flush line in inches	1.25 inch		HENDER
Length of return line	310 n.	CONTRACTOR OF THE PARTY OF THE	
Equivalent length of fittings	62 ft.		and the same
Elevation change. (If downfill enter 0)	9 0.		
Pressure loss in 100 ft of pips	5.55 ft.	2.40	ps
Total pressure loss from end of dripline to return tank	29.6 ft.	12.83	
B. Dripline - Losses through Wastellow dripline			
Length of longest dripline lateral	129 ft.		
Minimum dosing pressure required at end of dripline	23.10 ft.	10.00	ps
Loss through dripline during flushing	37.93 ft.	16.42	13
Total minimum required dripline pressure	61.03 fl.	16.42 #	25
A+B. Minimum Pressure required at keplantog of displield		- 1-1	-
CALCULATED pressure required at beginning of dripfield	90.66 ft.	39.25 p	75
SPECIFIED pressure at beginning of dripfield (from workshi 1)	92,4 ft.	40.00 p	ינט
Great! SPECIFIED Pressure is greater that CALCULATED Pressure	-	-	
C. Drip components - Losses through headworks	Technical International Control of the Control of t		-
Filtor	4.6 R.	2.00 p	psi
Zone valve pressure loss (not in diagram)	6.93 A.	3.00 p	08
Flow nucler pressure loss (not in diagram)	2.00 €	0.87 p	25
Other pressure losses	n.	- p	ps
Total loss through drip components	13.55 ft.	5.87 p	13
D. Supply line - Minimum Pressure head required to get from		ripfield	
Sizu of supply flao in inches	1.25 Inch		-
Length of supply line	105 R.		-
Equivalent length of fittings	21 0.		_
Height from pump to tank outlet	5 A.		
Blovation change. (if downfill enter 0)	10.		-
Pressure loss gain in 100 ft. of pipe	9.83 n.	4.26 p	33
Total gain or loss from pump to field	18.4 ft.	7.96 p	35
Total dynamic head	124,3 (1.	53.83 p	75
Pump capacity *	20.2 gpm		
Pump Model Number	20FE07P4-2W230		
Voltz/Hp/phase	230 V / 3/4 HP / 1 Ph	330	

^{*} Note: Pump capacity flow assumes flow in dripling does not change during a dozo cycle. With Wastellow Class For more accurate flows please see Goullow's Flushing worksheet.

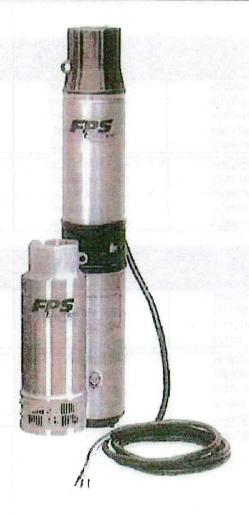
If you need assistance designing for this additional flow, pieces

a. See Geoflow flushing worksheet or

b. Contact Geofforv at 800-828-3388.

Submersible Pumps

E-Series Environmental Pumps Features





Applications:

- Filtered effluent service
- M Aerallon
- Ornamental fountains/waterfalls

Features:

- ldeal for effluent pumping applications.
- Stainless steel or thermoplastic discharge and motor bracket are tough and non-corrosive. Both materials are highly resistant to damage by minerals, metals and other substances typically found in water.
- Heavy duly, 300 V, 10' SJOOW motor leads.
- Ceramic bearing sleeve has time proven durability for years of reliable service.
- Hex rubber bearing has extra large surface assuring shaft stability and multiple flow channels keeping particles away from bearing surfaces.
- Proven hydraulic staging allows close tolerances and increased performance.
- Carbon phenolic up thrust washer prevents excessive wear in severe applications.
- Hemovable built in check valve on 10-20 gpm pumps. No built in check valves on high capacity pumps.
- Powered by Franklin Electric submersible motor.

Submersible Pumps

E-Series Environmental Pumps Ordering Information (See pgs. 2-3 for model number explanation)

Thermoplastic Ordering Information

1/2 - 1.5 HP Single-Phase Units										
GPM	HP	Volt	Wife	Order No.	Model	W				
10	1/2	115	2	94741005	10FE05P4+2W115	25				
	1/2	530	2	94741010	10FE05P4-2W230	25				
	3/4	230	2	94741015	10FE07F4-2W230	29				
	1	230	2	94741020	10FE1P4-2W230	33				
	1.5	230	2	94741025	10FE15P4-2W230	41				
20	1/2	115	2	94742005	20FE06P4-2W115	24				
	1/2	230	2	94742010	20FE05P4-2W230	24				
	3/4	230	2	94742015	20FE07P4-2W230	28				
	1	230	2	94742020	20FE1P4-2W230	31				
	1.6	230	2	94742025	20FE16P4-2W230	39				

1/2 - 2/HP Pomp Ends									
CPM	Hb	Volt	Wire	Order No.	World	W			
10	1/2	N/A	2	94751005	10FE08P4-PE	7			
	3/4	N/A	N/A	94751010	10FE07P4-PE	8			
	1	NIA	N/A	94751015	10FE1P4-PE	9			
	1.5	N/A	N/A	94751020	10FE15P4-PE	10			
20	1/2	N/A	N/A	94752005	20FE05P4-PE	6			
	3/4	N/A	N/A	94752010	20FE07P4-PE	6			
	1	N/A	N/A	94752018	20FE1P4-PE	7			
	1,6	N/A	N/A	94752020	20FE15P4-PE	8			
	2	NVA	N/A	94752025	20FE2P4-PE	9			

Notes: Discharge is 1-1/4" NPT. Maximum diameter across cable guard is 3.90" on all models. Weight in pounds.

Submersible Pumps

E-Series Environmental Pumps Thermoplastic - 20 GPW Performance Curves

