October 16, 2014

Ms. Heather McCollister 1512 D Street Napa, CA 94559

### Traffic Impact Study for the Girard Winery Project

Dear Ms. McCollister;

Whitlock & Weinberger Transportation, Inc. (W-Trans) has completed a focused traffic analysis addressing potential traffic impacts and access needs for the proposed new winery to be located at 1077 Dunaweal Lane in the County of Napa. The traffic study was completed in accordance with the criteria established by the County of Napa, and is consistent with standard traffic engineering techniques. Comments from County staff have been addressed in preparing this final study.

#### Study Area

The project site is located on the east side of Dunaweal Lane between Silverado Trail and State Route (SR) 29, and is currently vacant. Dunaweal Lane is a two-lane roadway that runs north-south, and is designated as a local roadway. The posted speed limit on Dunaweal Lane is 45 miles per hour (mph).

Two intersections were identified by County staff for analysis.

Silverado Trail/Dunaweal Lane is a tee intersection with stop controls and flared right-turn lane on the northbound terminating Dunaweal Lane approach.

SR 29/Dunaweal Lane is stop-controlled with flared right-turn lanes on both the northbound and southbound Dunaweal Lane approaches.

#### **Project Description**

The proposed project would allow production of up to 200,000 gallons of wine annually, and operation of a tasting room for an average of 52 visitors on a weekday and 62 visitors on a weekend (or maximums of 75 and 90 visitors on a peak day, respectively. The project would have eight full-time employees and three part time employees on-site during weekdays as well as two full-time employees and four part-time employees on weekends. Vehicular access to the project site would be provided via a full access driveway on Dunaweal Lane. The most recent site plan, dated February 4, 2014 is enclosed.

#### **Existing Volumes**

Mechanical tube counts were collected on Dunaweal Lane near the project site on three consecutive days in March 2014 (Thursday through Saturday). Intersection counts were taken during the p.m. peak period in September 2014 at Silverado Trail/Dunaweal Lane and SR 29/Dunaweal Lane. The existing traffic volumes on Dunaweal Lane are summarized in Table I. The volume of traffic ranged from 1,484 on Thursday to 1,691 vehicles on Saturday; this would be considered relatively low and reflects the volumes that would be generated by a residential subdivision having fewer than 20 homes.

Existing Traffic Volumes									
Study Segment	Frie	lay	Sati	urday					
	Daily Trips	PM Peak	Daily Trips	Midday Peak					
	NB/SB	NB/SB	NB/SB	NB/SB					
Dunaweal Ln	828/746	68/90	880/811	101/77					
Total (NB+SB)	1,574	158	1,691	178					

# Table I

### **Existing Conditions**

#### Intersections

Using the turning movement data collected at the two study intersections together with the current configurations, existing operating conditions at each intersection were evaluated. As shown in Table 2, both intersections are currently operating at LOS A or B overall and on all approaches. Copies of the calculations for all scenarios are enclosed.

	Existing PM Peak Hour Intersection Levels of Service										
St	udy Intersection	Existing C	onditions	Existing pl	us Project						
	Approach	Delay	LOS	Delay	LOS						
١.	Silverado Trail/Dunaweal Ln	1.8	Α	1.8	А						
	Westbound (Silverado) Left-turn	7.6	А	7.6	А						
	Northbound (Dunaweal) Approach	8.9	А	8.9	А						
2.	SR 29/Dunaweal Ln	0.9	Α	0.9	Α						
	Northbound (Dunaweal) Approach	9.7	А	9.7	А						
	Southbound (Dunaweal) Approach	11.6	В	11.6	В						
	Eastbound (SR 29) Left-turn	8.9	А	8.9	А						
	Westbound (SR 29) Left-turn	8.1	А	8.1	А						

Table 2

Notes: Delay is measured in average seconds per vehicle; LOS = Level of Service

According to Policy CIR-16 of the Napa County General Plan, 2008, "No single level of service standard is appropriate for un-signalized intersections, which shall be evaluated on a case-by-case basis to determine if signal warrants are met." For analysis purposes it was assumed that the impact would be significant if project-added traffic caused operation to fall to LOS E or F on an approach for which the Peak Hour Volume Signal Warrant is met.

With all approaches at LOS A or B, the current operation of both intersections would be considered acceptable. While weekend operation was not evaluated, given the similarity of volumes on a weekday versus a weekend day together with the very low average delays currently being encountered, it appears reasonable to conclude that operation during the weekend peak period is also low and therefore acceptable.

Ms. Heather McCollister

#### Roadways

Information in the Napa County General Plan Update Draft Environmental Impact Report, February 2007 (GPUDEIR), indicates that under 2003 volumes SR 29 was operating at LOS D between Lodi Lane and Deer Park Road (this is the nearest segment included in the analysis). Silverado Trail is identified in the same document as operating at LOS C under 2003 volumes.

Policy CIR-16 of the Napa County General Plan also provides guidance for roadways, indicating that, "The County shall seek to maintain an arterial Level of Service D or better on all county roadways, except where maintaining this desired level of service would require the installation of more travel lanes than shown on the Circulation Map." Both SR 29 and Silverado Trail are shown as 2-lane Rural Collectors on the Circulation Map (Figure CIR-1). As a result, the LOS D standard does not apply and operation is therefore considered acceptable regardless of the service level.

### **Collision History**

The collision history along Dunaweal Lane between Silverado Trail and SR 29 was reviewed to determine any trends or patterns that may indicate a safety issue. Collision rates were calculated based on the collision data available from the California Highway Patrol as published in their *Statewide Integrated Traffic Records System* (SWITRS) reports during a five-year period between January 1, 2007, and December 31, 2011. The calculated collision rate for the study segment was compared to the average collision rate for similar facilities statewide, as indicated in 2010 Collision Data on California State Highways, California Department of Transportation (Caltrans).

The statewide average collision rate for a rural two-lane, flat road with a speed limit of 55 mph or less is 1.05 collisions/million vehicle miles (c/mvm). Over the five-year study period, seven collisions were reported on Dunaweal Lane between Silverado Trail and SR 29, for a calculated collision rate of 0.90 c/mvm, which is lower than the statewide average for similar facilities. Further, no injuries or fatalities were reported during the five-year study period. The collision rate calculation spreadsheet is enclosed.

#### **Future Volumes**

Future projected traffic volumes were obtained from the Solano Transportation Authority (STA) who maintains the joint Napa County/Solano County 2010-2030 Travel Demand Forecasting Model. The data used included directional segment volumes along SR 29 and Silverado Trail for the p.m. peak hour. Using the 2030 and 2010 model volumes a growth factor of 1.45 was determined for SR 29. This growth factor was applied to turning movements to and from Dunaweal Lane and the remainder of the future increase was added to the volumes for the through movements. It is noted that the 78 vehicle trips added to Dunaweal Lane during the p.m. peak hour would adequately represent increases associated with three new wineries or expansions to existing wineries along Dunaweal Lane.

#### **Future Conditions**

#### **Intersections**

Based on these projected future volumes, the two study intersections are expected to operate acceptably overall, though the northbound Dunaweal approach to Silverado Trail is expected to operate at LOS E and the southbound Dunaweal Lane approach to SR 29 is expected to operate at LOS F. These results are shown in Table 3.

St	udy Intersection	Future C	onditions	Future plus Project		
	Approach	Delay	LOS	Delay	LOS	
١.	Silverado Trail/Dunaweal Ln	3.9	А	4.9	А	
	Westbound (Silverado) Left-turn	9.5	А	9.6	А	
	Northbound (Dunaweal) Approach	38.7	Е	45.7	Е	
2.	SR 29/Dunaweal Ln	9.6	A	12.4	В	
	Northbound (Dunaweal) Approach	20.3	С	20.7	С	
	Southbound (Dunaweal) Approach	**	F	**	F	
	Eastbound (SR 29) Left-turn	11.4	В	11.4	В	
	Westbound (SR 29) Left-turn	8.7	А	8.7	А	

Table 3Future PM Peak Hour Intersection Levels of Service

Notes: Delay is measured in average seconds per vehicle; LOS = Level of Service; \*\* = delay greater than 120 seconds

#### <u>Roadways</u>

According to the GPUDEIR, under projected 2030 volumes SR 29 is expected to operate at LOS F in the study area and, despite substantial increases in traffic, Silverado Trail is expected to continue operating at LOS C. As previously noted, the County has exempted both of these roads from their operational standard, so the projected operation is considered acceptable.

#### **Trip Generation**

The anticipated trip generation for a proposed project is typically estimated using standard rates published by the Institute of Transportation Engineers (ITE) in *Trip Generation Manual*, 9<sup>th</sup> Edition, 2012. However, the publication contains no such information for a winery. Therefore, the County of Napa's Winery Traffic Information/Trip Generation Sheet was used to determine the anticipated traffic that would be generated by the proposed tasting room. A copy of this worksheet is enclosed.

Employee-related trips will be minimized by scheduling employee shifts that reduce the number of trips generated during the p.m. peak period. Production employees will work Monday through Friday from 7 a.m. to 3 p.m., hospitality and/or tasting room employees will work seven days per week from 9 a.m. to 6 p.m. and administrative employees will work Monday through Friday from 8 a.m. to 5 p.m. The resulting weekday p.m. peak hour trips will be associated with administrative employees and tasting visitors only.

The County of Napa's Winery Traffic Information/Trip Generation Sheet does not include guidance on inbound versus outbound trips, so it was assumed that 75 percent of trips at the winery would be outbound during the weekday p.m. peak hour since most of the trips would be associated with employees and customers leaving at closure of the winery. For the weekend midday peak hour it was assumed that inbound and outbound trips would be evenly split. A summary of the project's trip generation potential is provided in Table 4.

Ms. Heather McCollister

Project Trip Generation										
Land Use	Daily	M P	/eekd M Pea	ay Ik	Weekend Midday Peak					
	Weekday	Weekend	Trips	In	Out	Trips	In	Out		
Proposed Project										
Winery plus Tasting Room	74	58	26	6	20	29	15	14		
Total Trips on Driveway	74	58	26	6	20	29	15	14		

Table 4Project Trip Generation

### **Trip Distribution**

The pattern used to allocate new project trips to the street network was determined by reviewing existing average daily traffic volumes on Dunaweal Lane. It is understood that the winery will direct employees to take SR 29 when their origin/destination is the north and take Silverado Trail when their origin/destination is the south. This results in right-turns from Dunaweal Lane to the regional network, further reducing impacts at the study intersections due to project-related trips. It is recommended that clear signage that directs tasting room visitors in the same fashion be installed at the project driveway for exiting vehicles and similar directions be posted on the winery's website.

Visitor traffic accessing the site from the north via Silverado Trail and from the south via SR 29 was assumed to have an even split, while all employee trips from the north take SR 29 and from the south were assumed to take Silverado Trail. Evening peak hour counts recently obtained at Dunaweal Lane together with the anticipated travel pattern specific to this project were used to estimate the splits at SR 29 and Silverado Trail. The resulting trip distribution is shown in Table 4.

Origin/Destination	PercentDaily/Weekendof TripsTrips		PM Peak Trips	Weekend Peak Trips
SR 29 south of Dunaweal				
Employee Trips	0	0/0	0	0
Visitor & Truck Trips	15	7/7	2	4
SR 29 north of Dunaweal				
Employee Trips	70	21/10	7	3
Visitor & Truck Trips	35	15/15	6	9
Silverado Trail south of Dunaweal				
Employee Trips	0	0/0	0	0
Visitor & Truck Trips	35	15/15	6	9
Silverado Trail north of Dunaweal				
Employee Trips	30	9/4	3	I
Visitor & Truck Trips	15	7/7	2	4
TOTAL		74/58	26	30*

 Table 4

 Trip Distribution Assumptions and Project-Added Trips

Note: \* Value does not equal trip generation exactly due to rounding

#### **Plus Project Conditions**

#### Intersections

Upon adding project-generated trips to existing volumes, both study intersections are expected to continue operating at LOS A or B overall as well as on all approaches. Because operation will remain acceptable, the impact is considered less-than-significant.

Under Future plus Project conditions both study intersections are projected to continue operating at the same levels of service both overall and on individual approaches except that the overall operation at SR 29/ Dunaweal Lane changes from LOS A to LOS B.

#### <u>Roadways</u>

The additional traffic that the project would generate would reasonably be expected to be included in the growth projected by the County's traffic model. Further, since both study roadways are exempt from the County's operational standard, the added trips can be considered to have a less-than-significant impact.

Recommendation: Steps should be taken to direct winery traffic in such a way as to minimize impacts and support efforts to maintain LOS D operation on the SR 29 study intersection and roadway segments.

#### Site Access

#### Left-Turn Lane Warrants

The need for a left-turn lane on Dunaweal Lane at the proposed project driveway was evaluated based on criteria contained in the *Napa County Road and Street Standards*, 2011. Because future average daily traffic volumes on Dunaweal Lane are not available, recently obtained counts for both the weekday and weekend were used for this analysis.

Using the County's criteria, for the daily Friday traffic volume of 1575 vehicles and 1875 vehicles on a weekend, a left-turn lane would not be warranted for the projected driveway ADT of 74 vehicles on a weekday and 60 vehicles or more on a weekend. The proposed project would generate a weekday average of 74 trips and weekend average of 58 trips. Based on these traffic levels, a left-turn lane would not be warranted at the project driveway. The left-turn lane warrant graphs are enclosed for reference.

#### Sight Distance

At driveways, a substantially clear line of sight should be maintained between the driver of a vehicle waiting on the driveway and the driver of an approaching vehicle. Adequate time must be provided for the waiting vehicle to either cross, turn left, or turn right, without requiring the through traffic to radically alter their speed.

Sight distance along Dunaweal Lane at the proposed driveway was evaluated based on sight distance criteria contained in the *Highway Design Manual* published by Caltrans. The recommended sight distance for minor street approaches that are driveways is based on stopping sight distance, with the approach travel speeds as the basis for determining the recommended sight distance. For a 45-mph posted speed limit on Dunaweal Lane, the recommended stopping sight distance for a private driveway is 360 feet.

Dunaweal Lane is relatively flat and straight on both sides of the proposed driveway. Based on a review of the site plan, proposed driveway and Google Earth, sight lines are more than adequate and meet the recommended distance for the prevailing travel speeds.

Ms. Heather McCollister

#### **Conclusions and Recommendations**

- The proposed project would generate an average of 74 new daily trips, including 26 weekday p.m. peak hour trips and 29 weekend p.m. peak hour trips.
- The calculated collision rate for the study segment was lower than the statewide average for similar facilities.
- The study intersections and roadways are operating acceptably under existing volumes, and are expected to continue to do so with project trips added.
- Under projected future volumes the study intersections are expected to continue operating acceptably overall, though due to excessive delays anticipated at SR 29/Dunaweal Lane signalization may be warranted.
- SR 29 and Silverado Trail will continue to operate acceptably based on the applicable standards under projected Future volumes.
- It is recommended that the schedule for employee shifts be set to minimize the amount of traffic generated during the weekday p.m. peak hour.
- Clear signage that directs visitors to use SR 29 when destined to the north and Silverado Trail when destined to the south should be placed at the driveway. Similar information should be provided on the winery's website as well.
- A left-turn lane is not warranted at the project driveway based on Napa County's Left-Turn Lane Warrant criterion.
- Acceptable clear sight lines are available in both directions along Dunaweal Lane from the proposed driveway.
- The applicant should take steps to minimize traffic impacts and support efforts to maintain LOS D operation on SR 29 and its intersection with Dunaweal Lane.

Thank you for giving W-Trans the opportunity to provide these services. Please call if you have any questions.

Sincerely,

Dalene J. Whitlock, PE, PTOE Principal

DJW/djw/NAX077.L2

Enclosures: Site Plan Level of Service Calculations Collision Rate Calculation Spreadsheet Winery Traffic Information/Trip Generation Sheet Napa County Left-Turn Lane Warrant



SEGMENT	SEGMENT COLLISION RATE CALCULATIONS								
Vintage Wine Estates Project									
Loca	ition:	1077 Du	naweal Lane						
Date of Co	ount: ADT:	Thursday 1,500	y, March 06,	2014	ļ				
Number of Collisi Number of Inju Number of Fatali Start I End I Number of Yo	Number of Collisions: Number of Injuries: Number of Fatalities: Start Date: End Date: Number of Years:								
Highway T Design Sp Ter Segment Ler Direc	Type: Area: beed: train: ngth: ction:	Conventi Rural ≤55 Flat 0.8 North/Sc	ional 2 lanes miles outh	or le	SS				
	Nur	mber of C	ollisions x 1 M	Aillior	n				
ADT x 365 Da	ays pei	r Year x S	egment Leng	gth x	Number o	of Years			
	2	x	1,000,000	)					
1,500	х	365	x 0.8	31	х	5			
Study Segment	Collisi	on Rate	Fatality Ra	te	Injury F	Rate			
otady obginoint	1.05	c/mym	2.4%		40.1	<u> </u>			l

## Winery Traffic Information / Trip Generation Sheet

Traffic during a Typical We	ekday			
Number of FT employees: <u>8</u>	x 3.05 one-way trips per employee	=	24	daily trips.
Number of PT employees: <u>3</u>	x 1.90 one-way trips per employee	=	6	daily trips.
Average number of weekday visitors: <u>52</u>	/ 2.6 visitors per vehicle x 2 one-way trips	=	40	daily trips.
Gallons of production: 200,000	/ 1,000 x .009 truck trips daily <sup>3</sup> x 2 one-way trips	=	4	daily trips.
	Total	=	74	daily trips.
(№ of FT employees) + (№ of PT	employees/2) + (sum of visitor and truck <u>trips</u> x .38)	=	26	PM peak trips.
Traffic during a Typical Sat	urday			
Number of FT employees (on Saturdays):	2 x 3.05 one-way trips per employee	=	6	daily trips.
Number of PT employees (on Saturdays): <u>'</u>	4 x 1.90 one-way trips per employee	=	8	daily trips.
Average number of Saturday visitors: $62$	/ 2. 8 visitors per vehicle x 2 one-way trips	=	44	daily trips.
	Total	=	58	daily trips.
(№ of FT en	nployees) + (№ of PT employees/2) + (visitor <u>trips</u> x .57)	=	29	PM peak trips.
Traffic during a Crush Satu	rday			
Number of FT employees (during crush): 2	x 3.05 one-way trips per employee	=	61	daily trips.
Number of PT employees (during crush):	x 1.90 one-way trips per employee	=	19	daily trips.
Average number of Saturday visitors: <u>62</u>	/ 2. 8 visitors per vehicle x 2 one-way trips	=	44	daily trips.
Gallons of production: 200,000	/ 1,000 x .009 truck trips daily x 2 one-way trips	=	4	daily trips.
Avg. annual tons of grape on-haul: $\underline{1,000}$	/ 144 truck trips daily <sup>4</sup> x 2 one-way trips	=	14	daily trips.
	Total	=	142	daily trips.
Largest Marketing Event- A	dditional Traffic			
Number of event staff (largest event): 30	x 2 one-way trips per staff person	=	60	trips.
Number of visitors (largest event): 500	/ 2.8 visitors per vehicle x 2 one-way trips	=	357	trips.
Number of special event truck trips (largest	event): 10 x 2 one-way trips	=	20	trips.

<sup>&</sup>lt;sup>3</sup> Assumes 1.47 materials & supplies trips + 0.8 case goods trips per 1,000 gallons of production / 250 days per year (see *Traffic Information Sheet Addendum* for reference). <sup>4</sup> Assumes 4 tons per trip / 36 crush days per year (see *Traffic Information Sheet Addendum* for reference).

PM Existing		Wed Oct 1, 2014 15:07:56 Page 2-1										
PM Peak Hour - Existing Conditions Vintage Wine Estates TIS County of Nara												
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Level Of Service Computation Report												
	2000 HCM Unsignalized Method (Base Volume Alternative)											
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<pre>intersection #1 Silverado Trail/Dunaweal Ln</pre>												
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veraye beray (Sec/ven): I.o WOISE Case Level OF Service: A[ 8.9]												
Street Name:			Dunawe	eal Ln				S	ilverad	do Tra	il	
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Initial bse:	1 00	1 00	1 00	1 00	1 00	1 00	1 00	1 00	1 00	1 00	1 00	1 00
OUF Adj.	0 94	0.94	0.94	0 94	0 94	0 94	0 94	0 94	0.94	0.94	0 94	0 94
HF Volume.	17	0.54	89	0.54	0.54	0.54	0.54	177	29	16	264	0.54
Reduct Vol:		Ő	0	0	0	0	0		0	10	0	0
FinalVolume:	17	0	89	Ő	0	0	0	177	29	16	264	0
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love Cap.:	538	478	855	XXXX	XXXX	XXXXX	XXXX	XXXX	XXXXX	1377	XXXX	XXXXX
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Shrd ConDel:	xxxxx	8.9	XXXXX	XXXXX	XXXX	XXXXX	XXXXX	XXXX	XXXXX	7.6	XXXX	XXXXX
Shared LOS:	*	A	*	*	*	*	*	*	*	A	*	*
ApproachDel:		8.9		X	XXXXX		X	XXXXX		X	XXXXX	
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Intersection #2 SR 29/Dunaweal Ln												
Average Delay (sec/veh): 0.9 Worst Case Level Of Service: B[ 11.6]												
Street Name:			Dunawe	eal Ln					SR	29		
Approach:	Noi	rth B	ound	Sot	uth Bo	ound	Εa	ast B	ound	We	est Bo	ound
Movement:	L -	- т	- R	L ·	- т	- R	L ·	- т	– R	L ·	- т	- R
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Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	2	0	2	47	0	25	14	382	2	2	558	64
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93
PHF Volume:	2	0	2	51	0	27	15	412	2	2	601	69
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
FinalVolume:	. 2	0	2	51	0	27	. 15	412	2	2	601	69
Critical Car	Modu	1										
Critical Gap	MOQU. 7 1	Le: 6 5	6.2	7 1	6 5	6.2	4 1	~~~~	~~~~~	4 1	~~~~	~~~~
FollowUpTim.	3.5	4 0	33	35	4 0	33	2 2	××××	*****	2 2	××××	*****
Capacity Mod	ule:											
Cnflict Vol:	1096	1117	413	1084	1084	636	670	XXXX	XXXXX	414	XXXX	XXXXX
Potent Cap.:	192	209	644	196	219	482	930	XXXX	XXXXX	1156	XXXX	XXXXX
Move Cap.:	179	205	644	193	215	482	930	XXXX	XXXXX	1156	XXXX	XXXXX
Volume/Cap:	0.01	0.00	0.00	0.26	0.00	0.06	0.02	XXXX	XXXX	0.00	XXXX	XXXX
Level Of Ser	vice N	Modul	e:							~ ~		
2Way95thQ:	XXXX	XXXX	XXXXX	XXXX	XXXX	XXXXX	0.0	XXXX	XXXXX	0.0	XXXX	XXXXX
LOS by Move:	* *	* *	* *	* *	* *	*	8.9	* *	* *	0.1	* *	*
Movement:	T.T.	- T.TP	- PT	LT .	- T.TP	- PT	т.т	- T.TP	- PT	т.т т.т	- T.TP	- PT
Shared Can .	XXXX	769	XXXXXX	XXXX	624	XXXXXX	XXXX	XXXX	XXXXXX	XXXX	XXXX	XXXXX
SharedOueue:	XXXXX	0.0	XXXXX	XXXXX	0.4	XXXXXX	XXXXX	XXXX	XXXXX	XXXXX	XXXX	XXXXX
Shrd ConDel:	XXXXX	9.7	XXXXX	XXXXX	11.6	XXXXX	XXXXX	XXXX	XXXXX	XXXXX	XXXX	XXXXX
Shared LOS:	*	A	*	*	В	*	*	*	*	*	*	*
ApproachDel:		9.7			11.6		XX	xxxxx		X	xxxxx	
ApproachLOS:		A			В			*			*	
********	* * * * * *	*****	*****	*****	*****	* * * * * * *	*****	*****	*****	*****	* * * * * *	* * * * * * *
Note: Queue	report *****	ted i: *****	s the r	1umber	of ca *****	ars pei ******	1ane	•	* * * * * * * *	*****	* * * * * *	* * * * * * *

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PM Existing plus Project Wed Oct 15, 2014 09:12:31 Page 3-1 \_\_\_\_\_

County of Napa \_\_\_\_\_ Level Of Service Computation Report 2000 HCM Unsignalized Method (Future Volume Alternative) \*\*\*\*\* Intersection #1 Silverado Trail/Dunaweal Ln Average Delay (sec/veh): 2.0 Worst Case Level Of Service: A[ 8.9] \*\*\*\*\* Street Name: Dunaweal Ln Silverado Trail Approach: North Bound South Bound East Bound West Bound Novement: L - T - R L - T - R L - T - R -----||-----||------|| Control:Stop SignStop SignUncontrolledUncontrolledRights:IncludeIncludeIncludeInclude 
 Rights:
 Include
 <t Volume Module: >> Count Date: 17 Sep 2014 << 4:45 - 5:45 pm Base Vol: 16 0 84 0 0 0 0 167 27 15 248 0 Initial Bse: 16 0 84 0 0 0 0 167 27 15 248 0 Added Vol: 3 0 7 0 0 0 0 0 1 2 0 0 PasserByVol: 0 0 0 0 0 0 0 0 0 0 0 0 Initial Fut: 19 0 91 0 0 0 0 167 28 17 248 0 PHF Volume: 20 0 97 0 0 0 0 177 30 18 264 0 Reduct Vol: 0 0 0 0 0 0 0 0 0 0 0 0 0 FinalVolume: 20 0 97 0 0 0 0 177 30 18 264 0 -----||-----||------||------|| Critical Gap Module: Capacity Module: Potent Cap.: 540 481 854 xxxx xxxx xxxx xxxx xxxx 1376 xxxx xxxxx Move Cap.: 534 474 854 xxxx xxxx xxxx xxxx xxxx 1376 xxxx xxxx ------||-----||------||-------|| Level Of Service Module: LOS by Move: \* \* \* \* \* \* \* \* \* \* A \* \* Movement: LT - LTR - RT SharedQueue:xxxxx 0.4 xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx 0.0 xxxx xxxxx Shrd ConDel:xxxxx 8.9 xxxxx xxxxx xxxxx xxxxx xxxxx 7.7 xxxx xxxxx Shared LOS: \* A \* \* \* \* \* \* \* \* \* \* \* \* \* \* \* \* ApproachDel: 8.9 xxxxx xxxx xxxx xxxx ApproachLOS: A \* \* \*

Note: Oueue reported is the number of cars per lane.

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PM Existing plus Project Wed Oct 15, 2014 09:12:31 Page 4-1 PM Peak Hour - Existing plus Project Conditions Vintage Wine Estates TIS County of Napa \_\_\_\_\_ Level Of Service Computation Report 2000 HCM Unsignalized Method (Future Volume Alternative) \*\*\*\*\* Intersection #2 SR 29/Dunaweal Ln Average Delay (sec/veh): 1.0 Worst Case Level Of Service: B[ 11.3] Street Name: Dunaweal Ln SR 29 Approach: North Bound South Bound East Bound West Bound Novement: L - T - R L - T - R L - T - R Control:Stop SignStop SignUncontrolledUncontrolledRights:IncludeIncludeIncludeInclude 
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 ------||------||-------|| Volume Module: >> Count Date: 16 Sep 2014 << 4:00 - 5:00 pm Base Vol: 2 0 2 47 0 25 14 382 2 2 558 64 Initial Bse: 2 0 2 47 0 25 14 382 2 2 558 64 Added Vol: 0 0 0 3 0 7 2 0 0 0 0 1 PasserByVol: 0 0 0 0 0 0 0 0 0 0 0 0 Initial Fut: 2 0 2 50 0 32 16 382 2 2 558 65 PHF Volume: 2 0 2 54 0 34 17 412 2 2 601 70 Reduct Vol: 0 0 0 0 0 0 0 0 0 0 0 0 0 FinalVolume: 2 0 2 54 0 34 17 412 2 2 601 70 -----!!-----!!------!!------!! Critical Gap Module: Critical Gp: 7.1 6.5 6.2 7.1 6.5 6.2 4.1 xxxx xxxxx 4.1 xxxx xxxxx FollowUpTim: 3.5 4.0 3.3 3.5 4.0 3.3 2.2 xxxx xxxxx 2.2 xxxx xxxxx Capacity Module: Cnflict Vol: 1105 1123 413 1089 1089 636 671 xxxx xxxxx 414 xxxx xxxxx Potent Cap.: 190 207 644 195 217 481 929 xxxx xxxxx 1156 xxxx xxxxx Move Cap.: 173 203 644 191 213 481 929 xxxx xxxxx 1156 xxxx xxxxx Volume/Cap: 0.01 0.00 0.00 0.28 0.00 0.07 0.02 xxxx xxxx 0.00 xxxx xxxx Level Of Service Module: LOS by Move: \* \* \* \* \* \* \* \* A \* \* A \* \* Movement: LT - LTR - RT Shared LOS: \* A \* \* B \* \* \* \* \* \* \* \* ApproachDel: 9.8 11.3 xxxxx ApproachLOS: A B \* XXXXXXX \* 

Note: Oueue reported is the number of cars per lane. 

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PM Future			We	ed Oct	1, 20	014 15	:08:03				Page	2-1
			PM Pea	ak Hou: intage	r - Fi Wine	uture ( Estat	Condit:	ions				
			۷.	Coi	unty o	of Nap	a 110					
	2000	ICM II	Level (	Di Ser	vice (	Comput.	ation 1 a Waluu	Report	t torrati			
**********	*****	*****	*******	******	*****	1 (DdS)	+ ******	118 AI *****	*******	L V ビ ) * * * * * * * *	****	******
Intersection #1 Silverado Trail/Dunaweal Ln												
Average Delay (sec/veh): 3.9 Worst Case Level Of Service: E[ 38.7]												
Street Name:			Dunawe	eal Ln				S	ilverad	do Trai	1	
Approach:	No	rth Bo	ound	Soi	uth Bo	ound	Εa	ast B	ound	We	est Bo	ound
Movement:	L	- т	– R	L ·	- T	- R	L ·	- T	- R	L -	- Т	- R
Control: Rights:	S:	top S: Inclu	ign ude	St	top S: Inclu	ign ude	Un	contro Incl	olled ude	Unc	contro Inclu	olled ude
Volume Module	e:											
Base Vol:	122	0	23	0	0	0	0	786	39	22	494	0
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	122	0	23	0	0	0	0	786	39	22	494	0
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	122	0	23	0	0	0	0	/86	39	22	494	0
Reduct Vol:	100	0	0	0	0	0	0	700	0	0	0	0
Finalvolume:	122			U 	0		0	/86	39	22     <b></b>	494	I
Critical Gap	Modul	le:					11			1 1		1
Critical Gp:	6.4	6.5	6.2	XXXXX	XXXX	XXXXX	XXXXX	XXXX	XXXXX	4.1	XXXX	XXXXX
FollowUpTim:	3.5	4.0	3.3	XXXXX	XXXX	XXXXX	XXXXX	XXXX	XXXXX	2.2	XXXX	XXXXX
Capacity Mod	ule:	1044	0.0.0							005		
Chilict Vol:	1344	1344	806	XXXX	XXXX	XXXXX	XXXX	XXXX	XXXXX	825	XXXX	XXXXX
Moure Corr :	169	140	205		××××	*****		××××	*****	014	XXXX	*****
Move cap.: Volume/Cap:	0 74	149	0 06	~~~~~	~~~~	~~~~~	~~~~~	~~~~	~~~~~	0 03	~~~~	~~~~~
Level Of Ser	vice l	Module	e:									
2Way95thQ:	XXXX	XXXX	XXXXX	XXXX	XXXX	XXXXX	XXXX	XXXX	XXXXX	0.1	XXXX	XXXXX
Control Del:	XXXXX	XXXX	XXXXX	XXXXX	XXXX	XXXXX	XXXXX	XXXX	XXXXX	9.5	XXXX	XXXXX
LOS by Move:	*	*	*	*	*	*	*	*	*	A	*	*
Movement:	LT ·	- LTR	- RT	LT ·	- LTR	- RT	LT ·	- LTR	- RT	LT -	- LTR	- RT
Shared Cap.:	XXXX	246	XXXXX	XXXX	XXXX	XXXXX	XXXX	XXXX	XXXXX	XXXX	XXXX	XXXXX
SharedQueue:	XXXXX	3.4	XXXXX	XXXXX	XXXX	XXXXX	XXXXX	XXXX	XXXXX	0.1	XXXX	XXXXX
Shra ConDel:	XXXXX	38./	XXXXX	XXXXX	XXXX	XXXXX	XXXXX	XXXX	XXXXX	9.5	XXXX	XXXXX
Shared LUS:	×	20 7	*	*	×	*	*	×	*	A	×	*
Approachioe:		/ .oc ج		X	* ***		X	×××∧∧		X2	×××∧.	
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Note: Queue	**************************************											

PM Future	Wed	Oct 1, 20	)14 17:	22:24		Page	3-1				
PM Peak Hour - Future Conditions Vintage Wine Estates TIS County of Napa											
	Level Of Service Computation Report										
2000 HCM ******	: UNSIGNAII2	2ea Melnoo *********	1 (Base	* * * * * * * * * * * * * * * * * * *	*******	∨e) **********	*****				
Intersection #2 SR 2	**************************************										
Average Delay (sec/v ******	eh):	9.6 ********	Worst	Case Level	Of Ser ******	vice: F[177 *******	.3] *****				
Street Name: Approach: North Movement: L -	Dunaweal Bound T - R	l Ln South Bo L - T	ound - R	East E L - T	SR 2 ound - R	29 West Bc L - T	und - R				
Control: Stop Rights: In Lanes: 0 0	- Sign uclude 1! 0 0	Stop Si Inclu 0 0 1!	 ign 1de 0 0	Uncontr Incl 1 0 0	olled ude 1 0	Uncontro Inclu 1 0 0	lled de 1 0				
Volume Module: Base Vol: 2	0 2	68 0		20 613		2 1113	93				
Growth Adj: 1.00 1. Initial Bse: 2 User Adj: 1.00 1. PHF Adj: 1.00 1. PHF Volume: 2 Reduct Vol: 0	$\begin{array}{cccccccccccccccccccccccccccccccccccc$	$ \begin{array}{cccccccccccccccccccccccccccccccccccc$	1.00 36 1.00 1.00 36 0	1.00 1.00 20 613 1.00 1.00 1.00 1.00 20 613 0 0	1.00 2 1.00 1.00 2 0	$\begin{array}{c} 1 & 1 & 1 & 0 \\ 1 & 0 & 1 & 1 & 0 \\ 2 & 1 & 1 & 3 \\ 1 & 0 & 1 & 0 & 0 \\ 1 & 0 & 1 & 0 & 0 \\ 2 & 1 & 1 & 3 \\ 0 & 0 & 0 \\ 2 & 1 & 1 & 1 \\ 0 & 2 & 1 & 1 \\ \end{array}$	1.00 93 1.00 1.00 93 0				
Finalvolume: 2	-	68 U		20 613		2 1113	93				
Critical Gap Module: Critical Gp: 7.1 6 FollowUpTim: 3.5 4	.5 6.2 .0 3.3	7.1 6.5 3.5 4.0	6.2 3.3	4.1 xxxx 2.2 xxxx	xxxxx xxxxx	4.1 xxxx 2.2 xxxx	xxxxx xxxxx				
Capacity Module: Cnflict Vol: 1836 18 Potent Cap.: 59 Move Cap.: 49 Volume/Cap: 0.04 0.	64 614 1 74 496 71 496 00 0.00 1	L819 1819 61 79 59 76 L.16 0.00	1160 240 240 0.15	1206 xxxx 586 xxxx 586 xxxx 0.03 xxxx	XXXXX XXXXX XXXXX XXXXX	615 xxxx 974 xxxx 974 xxxx 0.00 xxxx	XXXXX XXXXX XXXXX XXXXX				
Level Of Service Mod 2Way95thO: XXXX XX	ule:	·····	 xxxxx	0 1 xxxx		0 0 xxxx	×××××				
Control Del:xxxxx xx LOS by Move: * Movement: LT - L	XXX XXXXX XX * * .TR - RT	XXXX XXXX * * LT - LTR	* * - RT	11.4 xxxx B * LT - LTF	* - RT	8.7 xxxx A * LT - LTR	- RT				
Shared Cap.: xxxx 2 SharedQueue:xxxxx 0 Shrd ConDel:xxxxx 20 Shared LOS: *	40 xxxxx x .1 xxxx xx .3 xxxxx xx	xxxx 101 xxxx 6.5 xxxx 177 * "	***** *****	XXXX XXXX XXXXX XXXX XXXXX XXXX *	×××××× ×××××× ×××××××××	XXXX XXXX XXXXX XXXX XXXXX XXXX * *	XXXXX XXXXX XXXXX *				
ApproachDel: 20 ApproachLOS:	C ************	177.3 F		XXXXXX *	*****	XXXXXX *	*****				
Note: Queue reported	l is the num	nber of ca	ars per	lane.	*****	* * * * * * * * * * * *	*****				

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PM Future plus Project Wed Oct 15, 2014 09:12:36	Page 2-1	PM Future plus Project Wed Oct 15, 2014 09:12:36 Page 3-1
PM Peak Hour - Future plus Project Conditio Vintage Wine Estates TIS County of Napa	ns	PM Peak Hour - Future plus Project Conditions Vintage Wine Estates TIS County of Napa
Level Of Service Computation Report 2000 HCM Unsignalized Method (Future Volume Alter ************************************	native) ************************************	Level Of Service Computation Report 2000 HCM Unsignalized Method (Future Volume Alternative) ************************************
Average Delay (sec/veh): 4.9 Worst Case Level Of	Service: E[ 45.7]	Average Delay (sec/veh): 12.4 Worst Case Level Of Service: F[209.8]
Street Name:       Dunaweal Ln       Silve         Approach:       North Bound       South Bound       East Bound         Movement:       L       T       R       L       T       -	**************************************	<pre>************************************</pre>
Control:Stop SignStop SignUncontrolleRights:IncludeIncludeIncludeLanes:0010000001	d Uncontrolled Include 0 0 1 0 0 0	Control:       Stop Sign       Uncontrolled       Uncontrolled         Rights:       Include       Include       Include         Lanes:       0       0       0       1       0       1       0       1       0
Volume Module:         Base Vol:       122       0       23       0       0       0       786         Growth Adj:       1.00       0 <t< td=""><td><math display="block">\begin{array}{cccccccccccccccccccccccccccccccccccc</math></td><td></td></t<>	$\begin{array}{cccccccccccccccccccccccccccccccccccc$	
Critical Gap Module: Critical Gp: 6.4 6.5 6.2 xxxxx xxxx xxxx xxxx xxxx xxxx FollowUpTim: 3.5 4.0 3.3 xxxxx xxxx xxxx xxxx xxxx xxxx	xx 4.1 xxxx xxxxx xx 2.2 xxxx xxxxx	Critical Gap Module: Critical Gp: 7.1 6.5 6.2 7.1 6.5 6.2 4.1 xxxx xxxxx 4.1 xxxx xxxxx FollowUpTim: 3.5 4.0 3.3 3.5 4.0 3.3 2.2 xxxx xxxxx 2.2 xxxx xxxxx
Capacity Module: Cnflict Vol: 1348 1348 806 xxxx xxxx xxxx xxxx xxxx xxxx Potent Cap.: 168 152 385 xxxx xxxx xxxx xxxx xxxx xxxx Move Cap.: 164 148 385 xxxx xxxx xxxx xxxx xxxx xxxx xxx Volume/Cap: 0.76 0.00 0.08 xxxx xxxx xxxx xxxx xxxx xxxx	xx 826 xxxx xxxxx xx 813 xxxx xxxxx xx 813 xxxx xxxxx xx 0.03 xxxx xxxxx xx 0.03 xxxx xxxx	Capacity Module: Cnflict Vol: 1844 1869 614 1823 1823 1160 1207 xxxx xxxxx 615 xxxx xxxxx Potent Cap.: 58 73 496 60 78 240 585 xxxx xxxxx 974 xxxx xxxxx Move Cap.: 46 70 496 58 75 240 585 xxxx xxxxx 974 xxxx xxxxx Volume/Cap: 0.04 0.00 0.00 1.22 0.00 0.18 0.04 xxxx xxxx 0.00 xxxx xxxx
Level Of Service Module: 2Way95thQ: xxxx xxxx xxxx xxxx xxxx xxxx xxxx x	xx 0.1 xxxx xxxxx xx 9.6 xxxx xxxxx * A * * T LT - LTR - RT xx xxxx xxxx xxxxx xx 0.1 xxxx xxxxx * A * * xxxxxxx * *	Level Of Service Module: 2Way95thQ: xxxx xxxx xxxx xxxx xxxx xxxx 0.1 xxxx xxxx
Note. Queue reported is the number of Cars per lane.	* * * * * * * * * * * * * * * * * * * *	Note. Queue reporteu is the humber of Cars per lane. ************************************

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