

Fantesca Winery c/o Ms. Donna Oldford Plans 4 Wine 2620 Pinot Way St. Helena, CA 94574 February 19, 2013

Subject:

Traffic Analysis of the Spring Mountain Road at Rockland Road (Fantesca Winery Access Road) Intersection in Napa County, CA.

Dear Ms. Oldford:

This letter report presents our analysis of the Spring Mountain Road at Rockland Road (Fantesca Winery access road) intersection. The analysis is based on field surveys of the existing roadway/intersection conditions including traffic volumes, vehicle speeds, and sight distances. The data was evaluated relative to Napa County, Caltrans, and other standards for access design. The analysis reflects the existing winery production of 30,000 gallons annually and the proposed use modification for 18 daily visitors by appointment and six full-time employees (plus four part-time employees during crush season). (Refer to Figure 1 for vicinity map and Figure 2 for aerial photograph of the intersection.)

The surveyed existing volumes on Spring Mountain Road and Rockland Road are low and the intersection operates efficiently at level of service 'A' with minimal delays and no vehicle queues. As outlined in the report, the project would generate a maximum of 31-34 new daily trips and 8-12 peak hour trips. The intersection would continue to operate at LOS 'A' (with delays less than 10 seconds), indicating efficient operation and no vehicle queuing. The combination of daily volumes on Spring Mountain Road and the winery access road would not warrant a left turn lane based on Napa County standards. Volumes would also be well below the thresholds at which a right turn lane would be needed.

Sight distances at the Spring Mountain Road/Rockland Road intersection were measured and compared to Caltrans recommended distances based on vehicle speeds. From a standard setback location on the crossroad of 13 feet from the edge of Spring Mountain Road, the sight distance looking south is 500 feet which exceeds the recommended distance of 300 feet. However, the view to the north is obstructed by a portion of the hillside next to the intersection which limits sight distance to approximately 50 feet, which is less than the recommended 250 feet. The sight distance increases to 350 feet when a motorist is positioned at the east side edge of Spring Mountain Road where the hillside does not encroach.

Due to the low volumes and adequate visibility to the south, our field observations indicate drivers can move forward to the east side edge of Spring Mountain Road in order to adequately see to the north. However, in order to maximize driver safety and awareness, several recommendations have been presented, including installing a convex mirror at the intersection and installing advance warning signs on Spring Mountain Road to alert drivers of the intersection's presence.

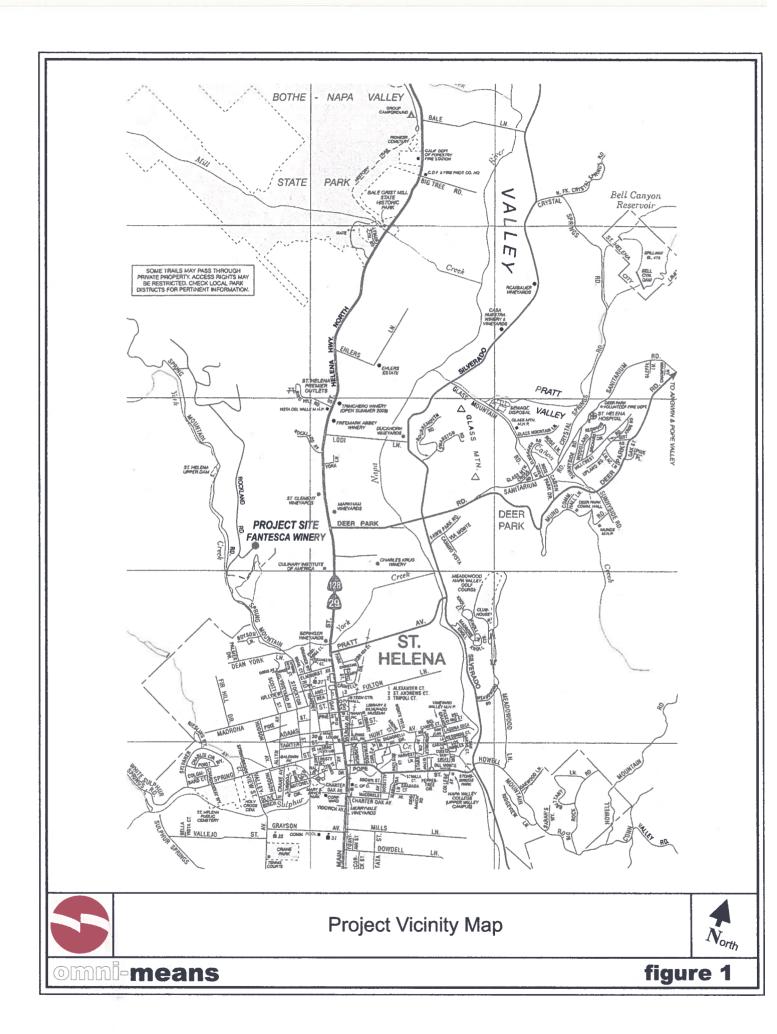
I trust that this report responds to your needs. Please feel free to contact us with any questions or comments.

Sincerely,

Seorge W. Nickelson, P.E.
OMNI-MEANS, Ltd.

Engineers & Planners

rt / R1585TIA003.doc / 35-5137-02



I. EXISTING CONDITIONS

1. Site Location

The Fantesca Winery is located on the east side of Spring Mountain Road north of the city of St. Helena in Napa County. The property access road (Rockland Road) intersects Spring Mountain Road approximately one mile north of Elmhurst Avenue. Spring Mountain Road is a rural two lane road that winds through curved and hilly terrain between St. Helena and Santa Rosa. Spring Mountain Road near the project access intersection is relatively straight for 300 feet to the north and 500 feet to the south before curving. In this area it is also relatively flat, but there is little to no shoulder area. The adjacent hillside slants downward from east to west with Spring Mountain Road cut into the hillside. It is bordered by the hillside to the east and a river with dense foliage to the west.

The Fantesca Winery shares access via Rockland Road with several other properties (#2900, 3000, and 3100). Rockland Road is a private road extending from Spring Mountain Road up a moderately steep grade for approximately 400 feet and then splits into several directions at the top of the hill including the Fantesca Winery gate. Rockland Road does not intersect with Spring Mountain Road at a 90-degree (perpendicular) angle, but rather enters at a 25-30 degree angle. Our field surveys measured Rockland Road to be approximately 20 feet wide. The road is paved and there are are two speed humps on the incline between Spring Mountain Road and the winery gate.

The Spring Mountain Road/Rockland Road intersection has single lane approaches (no separate turning lanes). There are no speed limit signs near the intersection. There is a yellow warning 25-mph speed limit sign ("Narrow Winding Road Next 5 Miles") on Spring Mountain Road further to the south.

2. Existing LOS Operations

In order to identify existing peak hour conditions, traffic counts were conducted at the Spring Mountain Road/Rockland Road intersection during a weekday PM commute period. (1) The peak hour volumes for all the approaches are low. Spring Mountain Road had 57 total trips (23 northbound and 34 southbound) during the peak hour. The Rockland Road volumes were very low, with 8 trips (5 westbound and 3 eastbound). The volumes represent the combined total peak hour traffic generated by all of the properties on Rockland Road. The existing volumes are shown in Figure 3.

Traffic conditions are measured by Level of Service (LOS), which applies a letter ranking to successive levels of intersection performance. LOS 'A' represents optimum conditions with free-flow travel and no congestion. LOS 'F' represents severe congestion with long delays at the approaches. For intersections with minor street stop control, the LOS reflects the delays experienced by the minor street approach. The LOS were analyzed based on HCM 2000 methodology using Synchro-Simtraffic software. (LOS definitions and calculation worksheets are provided in the Appendix.)

All approaches operate at LOS 'A', with less than ten seconds of delay. (LOS conditions are shown in Table 1.) The existing Rockland Road volumes are very low. The intersection operates with minimal delays and no vehicle queuing. It is reasonable to assume that volumes and operating conditions during the weekend peak hour are similar to the weekday conditions.

Daily volumes on Spring Mountain Road at the access road intersection would be expected to be approximately 570 trips (using a standard multiplier of ten times the PM peak hour). These volumes are low and indicative of LOS 'A' conditions.



II. PROPOSED PROJECT CONDITIONS

1. Project Description

Existing winery operations consist of 30,000 gallons annual wine production with no formal public services. There is no planned increase in winery production from the existing 30,000 annual gallons. Traffic generating components of the project comprise the following:⁽²⁾

- Employees: 6 Full-time during non-harvest;
 - 4 Part-time additional during harvest;
- Visitors: 18 daily visitors maximum (by appointment);
- Hospitality and Events: Four per month with up to 20 people; Six per year with 75 people; Two per year with up to 125 people (offsite parking and shuttle buses utilized).

2. LOS Operations With The Project

The winery's traffic was generated using standard Napa County use permit calculations.⁽³⁾ The calculated trips are shown in Table 2. The winery's existing wine production generates a small number of truck trips. Since no winery-related truck trips were observed during the intersection counts, they have been added to the project trip calculations to ensure they are accounted for in the LOS analysis. We have added one truck per day to remain conservative, but the calculation equates to one truck per week.

The project would be expected to generate a maximum of 31-34 daily trips and 8-12 peak hour trips during a typical weekday/weekend peak hour. During the harvest season, a maximum of 43 daily trips and 11 peak hour trips would be expected. The project trips were distributed at the Spring Mountain Road intersection based on existing volumes, with 75% to/from the south and 25% to/from the north. The project volumes are shown in Figure 3. It is noted that the added project volumes are likely conservatively high, since there is some existing informal visitation/winery activity occurring which would already be included in the existing volume counts.

Project conditions would be essentially unchanged from existing conditions, with all intersection approaches continuing to operate at LOS 'A' and delays remaining less than ten seconds during the peak hour. The volumes would remain low and no vehicle queuing would be expected. The existing plus project LOS are shown in Table 1.

The daily volume of 604 trips (570 existing + 34 project trips) on Spring Mountain Road at the site access road would remain well within the capacity of a two lane rural road with conditions equivalent to LOS 'A'.

TABLE 1
PEAK HOUR INTERSECTION LEVEL OF SERVICE AND DELAY

	Weekday PM Peak Hour					
Intersection	Existing LOS Delay	Existing + Project LOS Delay				
Spring Mountain Road / Rockland Road Unsignalized (Rockland Road stop control)						
Rockland Road Westbound approach:	A 8.7"	A 8.8"				
Spring Mountain Road southbound approach:	A <1"	A <1"				

Based on Highway Capacity Manual (HCM) 2000, Operations methodology for stop-sign controlled (unsignalized) intersections using Synchro-Simtraffic software. Intersection calculation yields a Level-of-Service (LOS) and vehicle delay (in seconds).



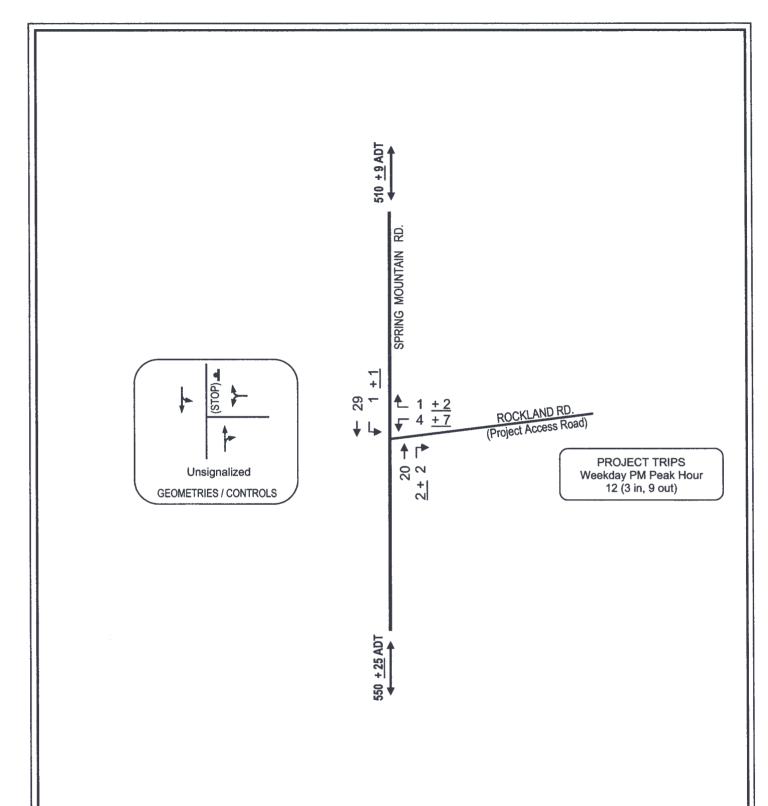
TABLE 2 TRIP GENERATION: PROPOSED FANTESCA WINERY EXPANSION

Typical Weekday Daily Traffic:	
18 visitors/2.6 per vehicle x 2 one-way trips	= 14 daily trips
6 full time employees x 3.05 one-way trips	= 18 daily trips
30,000 glns / 1,000 x .009 truck trips daily x 2 one-way trips	= 2 daily trips*
(*Conservatively assumes 1 truck per day; calculation equates to 1 tru	ick per week.)
Total Weekday Daily Trips	= 34 daily trips
Typical Weekday PM Peak Hour Traffic:	
(14 daily visitor trips + 2 daily truck trips) x .38	= 6 peak hour trips
6 full time employees	= 6 peak hour trips
Total Weekday Peak Hour Trips	= 12 total trips (3 in, 9 out)
Typical Weekend (Saturday) Daily Traffic:	
Project: 18 visitors/2.8 per vehicle x 2 one-way trips	= 13 daily trips
6 full time employees x 3.05 one-way trips	= 18 daily trips
Total Weekend (Saturday) Daily Trips	= 31 daily trips
Typical Weekend (Saturday) Peak Hour Traffic:	
Total Weekend Peak Hour Trips (31 daily trips x 25%)	= 8 total trips (4 in, 4 out)
Weekend (Saturday) Daily Traffic During Crush:	
Project: 18 visitors/2.8 per vehicle x 2 one-way trips	= 13 daily trips
6 full time employees x 3.05 one-way trips	= 18 daily trips
4 part time employees x 1.90 one-way trips	= 8 daily trips
1 service truck:	= 2 daily trips*
23 tons on-haul grapes/144 truck trips daily x 2 trips	= 2 daily trips*
Assumes 10% offsite grapes (23 tons out of 225 tons total grapes);	
(*Conservatively assumes 1 truck per day; calculations equate to 1 to	truck per week.)
Total Weekend (Saturday) Daily Harvest/Crush Trips	= 43 total daily trips
Weekend (Saturday) Peak Hour Traffic During Crush:	
Total Weekend Peak Hour Harvest Trips (43 daily trips x 25%)	= 11 total trips (5 in, 6 out)

Production, visitor, and employee data provided by Ms. Donna Oldford (project representative).

Trip equations for daily and weekday peak hour derived from Napa County, Conservation, Planning, & Development Department, "Use Permit Application Package", Napa County Winery Traffic Generation Characteristics, 2012. Trip equation for weekend peak hour based on conservative assumption that 25% of daily trips occur in peak hour.







Existing and <u>+ Project</u> Volumes Weekday PM Peak Hour



omni-means

figure 3

III. SITE ACCESS

1. Sight Distances

Vehicle sight distance at the Spring Mountain Road/Rockland Road intersection was evaluated. The recommended vehicle visibility or "corner sight distance" is a function of speeds on Spring Mountain Road.

As stated above, Spring Mountain Road has a 25 mph speed limit through the area due to curves and grades. Radar speed surveys were also conducted as a part of this study which identified 85th percentile speeds (the speed at which 85% of all surveyed vehicles travel at or below) of 38 mph northbound and 35 mph southbound. Based on Caltrans' design standards, a sight distance of about 300 feet is recommended for northbound vehicles and 250 feet is recommended for southbound vehicles. So

Caltrans guidelines use a standard setback distance for the vehicle waiting at the crossroad of 13 feet from the edge of the main road. From this location on the access road, our field measurements indicate the sight distance looking south from the driveway is approximately 500 feet, which exceeds the 300 feet recommendation for that direction. The sight distance looking to the north from the setback location is obstructed by the hillside slope next to the intersection and is limited to approximately 50 feet, which is less than the recommended 250 feet. However, as the driver on the crossroad moves forward to the east side edge of Spring Mountain Road, the sight distance increases to 350 feet (where the hillside no longer obstructs and a curve in Spring Mountain Road defines the maximum obtainable sight distance). Volumes are very low on Spring Mountain Road and the motorists we observed were able to move forward in order to see to the north and complete the turning maneuver.

The Caltrans setback distance is conservative and acknowledges that providing the recommended sight distance may not always be possible. Our experience indicates a driver within a vehicle can be located six or seven feet from the main road without encroaching into the approaching lane of travel. From a six-to-seven foot setback on Rockland Road the sight distance to the north is about 100 feet.

Based on the low volumes on Spring Mountain Road, our observations found that drivers on Rockland Road can first look to the south, then safely move forward when the northbound Spring Mountain Road approach is clear and look to the north with adequate sight distance to complete a turn. Nevertheless, any improvements to maximize sight distance and/or increase driver awareness of an intersection are always desirable if possible. Therefore several improvements are recommended as follows (and shown in Figure 4):

1a. Sight Distance Recommendations

- Installing a convex mirror on the west side of Spring Mountain Road facing east toward drivers on the Rockland Road approach would allow drivers on Rockland Road to see to the north (via the mirror) while still at the setback location east of the intersection.
- Installing advance "intersection ahead" symbol signs (Type W2-3 per CaMUTCD) on Spring Mountain Road north and south of the intersection would give drivers notice of the intersection's presence. (An existing sign of this type is located on Spring Mountain Road for a different intersection further south.) Consideration could be given to adding supplemental warning plaques (Type W16-2P) stating the distance is "____ feet ahead" to the intersection.
- If signs advertising the winery are allowed, such signs could be posted on Spring Mountain Road to the north and south instructing motorists to turn at the intersection.
- Maintaining low level vegetation on the north side of the intersection within the line of sight would maximize available sight distance.



2. Left-turn Lane

The access road intersects Spring Mountain Road at a point where no left turn lane exists.

The intersection was evaluated for a potential left turn lane on Spring Mountain Road based on Napa County design guidelines. (6) The Napa County standards utilize daily traffic volumes on the major road and access driveway. The existing and calculated daily volumes associated with the project are below the Napa County minimum thresholds. Therefore a left turn lane would not be warranted (a left turn lane warrant graph is provided in the Appendix).

The projected right turn volumes at the intersection are also well below minimum thresholds at which right turn lanes would be required (a right turn lane warrant graph is included in the Appendix.)⁽⁷⁾

3. Project Access Road (Rockland Road) Vehicle Circulation

The access road is sloped (eastbound uphill) for approximately 400 feet to the top of the hill where the winery entrance is located. It is paved and was field measured to be approximately 20 feet in width. This appears to meet the Napa County standard of 18 feet of pavement for two-way traffic flow. (8)

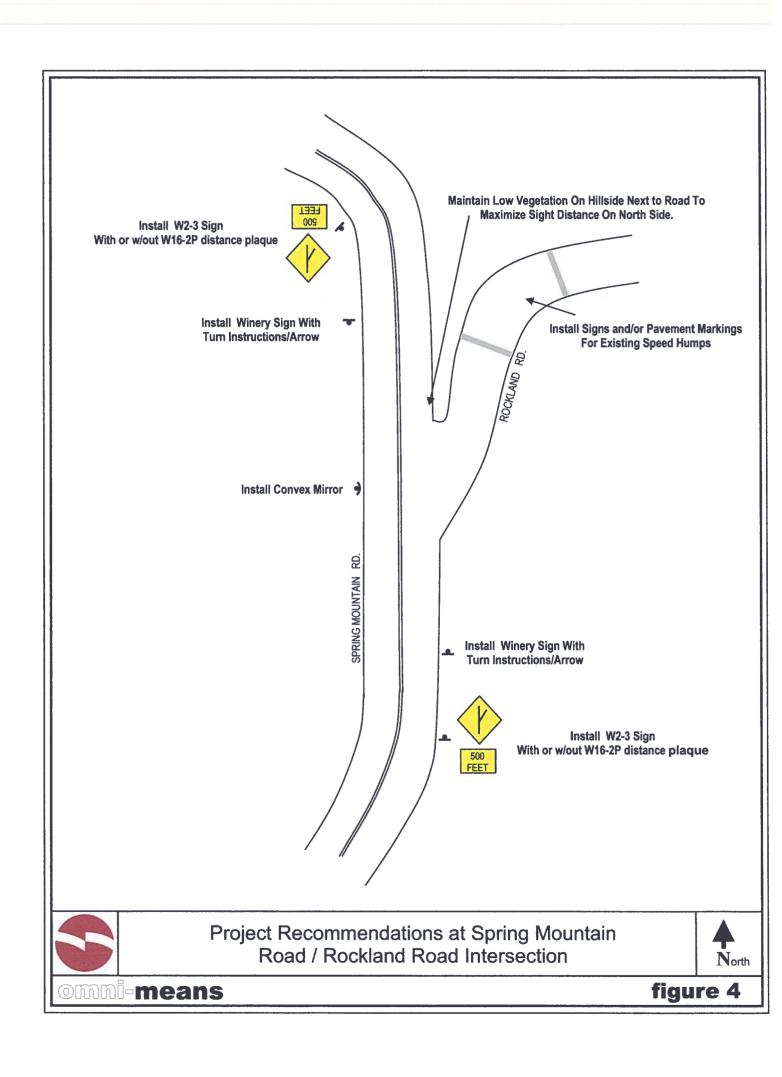
There is no proposed increase in winery production, and therefore no anticipated increase in truck trips to/from the project site. Presumably the access road's turning radius paths are adequately serving existing truck traffic.

There are two speed humps located on the sloped portion of the access road. However, there are no signs or pavement markings advising motorists of the speed humps' presence. Existing traffic is low and limited mostly to the local residents who are aware of the speed humps. The winery project would be visited by drivers unfamiliar with the access road, therefore the following is recommended:

3a. Access Road Recommendation

• Installing warning signs and/or pavement markings on Rockland Road to alert motorists of the speed humps would increase awareness for visitors unfamiliar with the access road.





IV. SUMMARY AND PROJECT RECOMMENDATIONS

The winery access road (Rockland Road) at Spring Mountain Road intersection was evaluated for operating and safety/sight distance conditions in light of the proposed Fantesca Winery expansion project.

Existing volumes are low on Spring Mountain Road (57 weekday PM peak hour trips) and the winery access road (8 peak hour trips). The project would add a maximum of 34 daily trips and 12 peak hour trips on typical days. The Spring Mountain Road/Rockland Road intersection operates at LOS 'A' conditions (less than 10 seconds delay) and would continue to operate at LOS 'A' (less than 10 seconds delay) with the project trips.

The winery's volumes would not warrant a left turn lane on Spring Mountain Road based on Napa County standards. The volumes would also be below the thresholds at which right turn lanes would be needed.

Vehicle sight distance at the Spring Mountain Road/Rockland Road intersection was evaluated. Based on the surveyed vehicle speeds, a sight distance of about 300 feet looking south (northbound vehicles) and 250 feet looking north (southbound vehicles) is recommended. Using the standard Caltrans setback distance of 13 feet for a vehicle waiting at the crossroad, our field measurements indicate the sight distance looking south from the driveway is approximately 500 feet, which exceeds the recommendation of 300 feet. Sight distance looking to the north from the setback location is obstructed by a portion of the hillside adjacent to the intersection which limits visibility to approximately 50 feet. However, as a driver on the crossroad moves forward to the east side edge of Spring Mountain Road, the sight distance is clear of the hillside and increases to 350 feet, which exceeds the recommended minimum sight distance.

Volumes are low on Spring Mountain Road and our observations indicate drivers on the Rockland Road approach were able to move forward, look north, and complete the turning maneuver. Although the intersection appears to function adequately, it is desirable to maximize sight distances and/or driver awareness of an intersection whenever possible. Therefore several recommendations are presented:

- Installing a convex mirror on the west side of Spring Mountain Road would allow drivers on Rockland Road to see toward the north from the setback location on the east side of the intersection.
- Installing "intersection ahead" signs (Type W2-3 and W16-2P) on Spring Mountain Road would alert motorists of the approaching intersection.
- Signs advertising the winery (if allowed) could also be posted on Spring Mountain Road north and south of the intersection directing drivers to turn at the intersection.
- Maintaining low level vegetation within the line of sight north of the intersection would maximize available sight distance.
- There are two existing speed humps on Rockland Road. Installing warning signs and/or pavement markings on Rockland Road for the speed humps would increase awareness for winery visitors unfamiliar with the access road.



References:

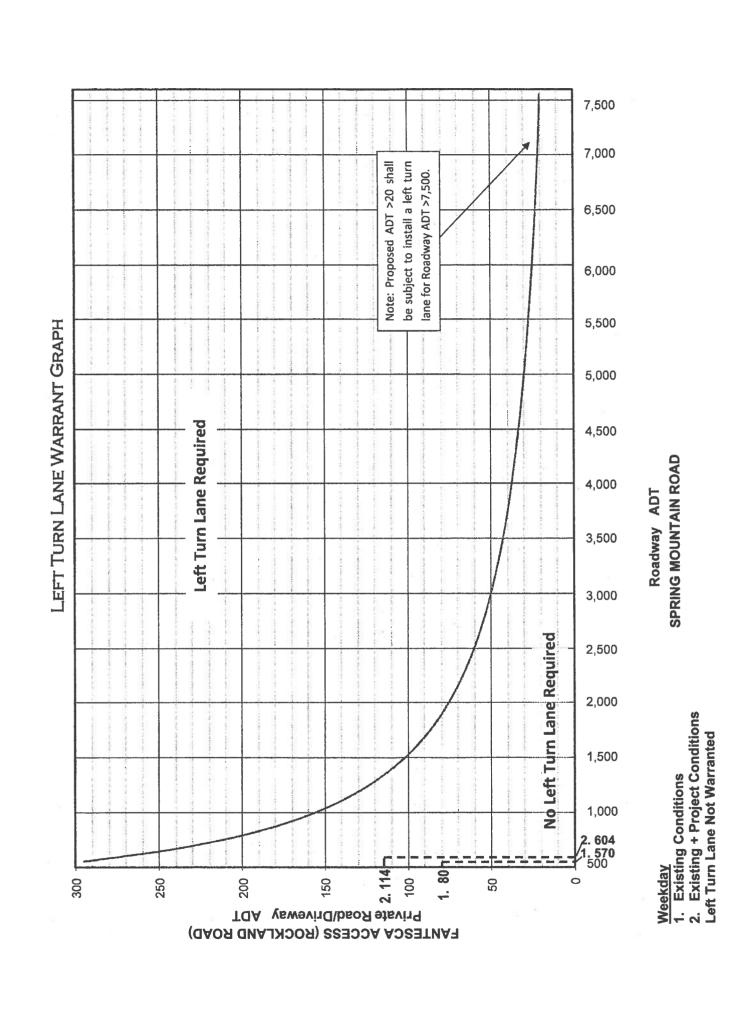
- (1) Omni-Means Engineers & Planners, traffic counts (4:00-6:00 pm), speed surveys, and field measurements on December 13, 2012.
- (2) Production, employee, and visitor data provided by Ms. Donna Oldford, project representative.
- (3) Napa County, Conservation, Planning, & Development Department, "Use Permit Application Package", Napa County Traffic Generation Characteristics, 2012.
- (4) Omni-Means, ibid.
- (5) Caltrans, Highway Design Manual Fifth Edition, July 1, 2004.
- (6) Napa County, Adopted Road and Street Standards, revised November 21, 2006.
- (7) Transportation Research Board, National Cooperative Highway Research Program Report 279, "Intersection Channelization Design Guide", November, 1985.
- (8) Napa County, ibid.

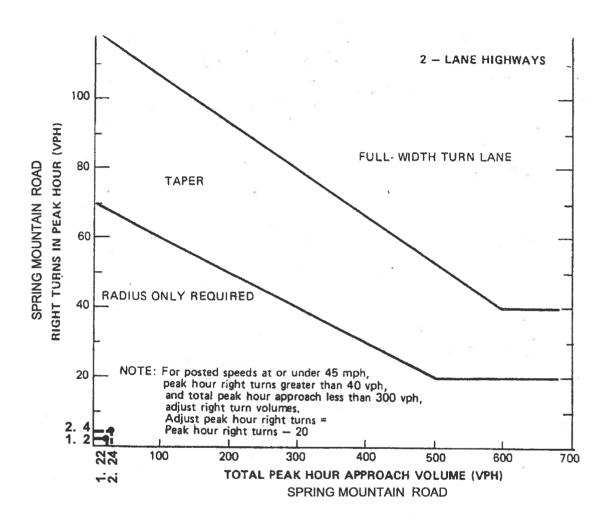


APPENDIX

- Turn Lane Warrant Graphs
- Speed Summary Worksheets
- Existing Count Worksheet
- Level of Service Definitions
- Level of Service Calculations







- 1. Existing Weekday PM Peak Hour Volumes
- 2. Existing + Project Weekday PM Peak Hour Volumes

Right Turn Lane Not Warranted Right Turn Lane Not Warranted

Spring Mountain Road / Fantesca Winery Access (Rockland Road) Intersection

RADAR SPEED SURVEY

Spring Mountain Road approaching Rockland Road

DATE	12/13/12	TIME	START: 3:30 TIME END:	5:45 pm	WEATHER: Dry	ROAD TYPE: 2 lanes; rural
DIREC	CTION: Northbo		LIMIT: 25 aph		OBSERVER: o-m	CALIBRATION TEST: Yes
SPE	D FREQUENCY	ACUM %			AGE BREAKDOWN	
7/				30	40	5060708090100
3(2.0	•			
32	_		#### ####5#### ####			
33		22.0		,		
34		34.0	[####5####[####5####2##		**	
35		48.0	[****5****1***5****2**			T.E.
38	-		:****5****(****5****2**			
37	_	76.0				:**5****5***6***5****7***5*
38	-	86.0				:**5:***5***6**5****7***7****8***5*
39		90.0				:**5****5***6***6****5****5****8*****
40		92.0				:**5****5***6***6****7****5****8***5****9**
41	-	94.0	·			***5****5***6***5****5****
42	-				-	:**5****5****6****5****7****5****8****5****5
43	-					:**5****5****6****5****7****5****8****5****9****5****0
7.						-5060708090100
	50		-020	30	and an approximation of	
50 85 90	ERAGE SPEED = th PERCENTILE th PERCENTILE th PERCENTILE th PERCENTILE	= 35. = 37. = 39	1 7. IA 9 VEH1	E = 30 - 3 I PACE = 9 ICLES IN PA	0	SAMPLE VARIANCE = 8.66486 STANDARD DEVIATION = 2.943613 RANGE 1*S = 72 RANGE 2*S = 94 RANGE 3*S = 100

RADAR SPEED SURVEY OMNI-MEANS LTD.

Spring Mountain Road approaching Rockland Road

DATE: 12/13/12 TIME START: 3:30 pm TIME END: 5:45 pm WEATHER: Dry ROAD TYPE: 2 lanes; rural

DIRECTION: Southbound

		SPEED	LIMIT: 25 mph	OBSERVER: o-	CALIBRATION TEST: Yes	
SPEED		ACUM %		PERCENTAGE BREAKDOWN		
			-020	3040	50607080	90100
27	1	2.0	· 元元			
28	i	4.0	1 * * * *			
29	5	14.0	[***55****]****			
30	6	26.0	{****5****1***5****2**	***5*		
31	å,	34.0	****5**** ****5****2**	***5****3***		
32	5	44.0	****5**** ****5****2*	***5****3***5****4***		
33	8	56.0	{****5****[****5****2**	+**5****3 **** 5****4****5	****5****5*	
34	7	70.0	****5****1****5****2**	***5****3****5***4****5	****5****5****6****5****7	
35	8	86.0	****5****1****5****2**	***5****3****5****4****	****5****5****6****5****7****5****8****5*	
36	3	92.0	****5**** ****5****2**	***5****3****5****4****5	****5****5****6****5****7****5****8****5***	1911
37	2	96.0	****5**** ****5****2**	***5****3****5****4****5	****5****5****6****5****7****5****8****5***	1911151
3B	1	98.0	****5**** ***5****2*	###\$#### \$####\$####	·***5****5****6****5***7** ** 5****8****5***	*9****5***
39	0	98.0	!****5****1****5****2**	***5****3****5****4****5	****5****5****6****5****7****5****8****5***	191115111
40	1	100.0	[****5****[****5****2**			•9****5****(
			-020	40	50607080	30100
	50					
AVER	AGE SPEED =	32.8	PACE	= 28 - 37	SAMPLE VARIANCE = 7.877551	
50th	PERCENTILE	= 32.5	7. 18	PACE = 94	STANDARD DEVIATION = 2.806698	
85th	PERCENTILE	= 34.9	VEHI	ICLES IN PACE = 47	RANGE 1*S = 72	
	PERCENTILE				RANGE 2*S = 96	
	PERCENTILE		1		DANCE THE - 100	

95th PERCENTILE = 36.7

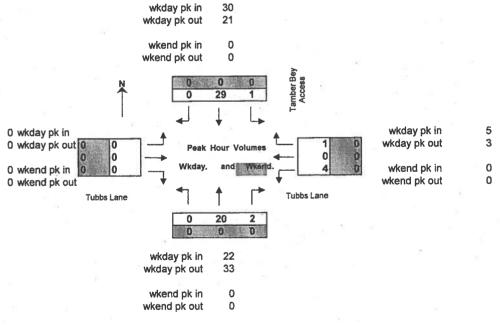
RANGE 3*S = 100

Intersection Volume Worksheet

phf =

0.89

Location: Spring Mountain Road at Rockland Road (Fantesca Winery Access Road and #'s 2900, 3000, 3100) 12/13/12, Thursday Weather: Clear Spring Mtn. Rd. A = Adult Access Road T = Teen **X**87 C = Child B = Bike Peds. & Bikes Weekday PM 5 15 MIN. 60 MIN. a-b/c-d 4:00-4:15 4:15-4:30 4:30-4:45 4:45-5:00 5:00-5:15 5:15-5:30 0 0 5:30-5:45 5:45-6:00 PeakHour: 4:30-5:30



OMNI-MEANS

LEVEL OF	=======================================			CONTRO	CONTROL DELAY (SECONDS/VEHICLE)	(HICLE)
SERVICE	TYPE OF FLOW	DELAY	MANEUVERABILITY	SIGNALIZED	UNSIGNALIZED	ALL-WAY STOP
¥	Stäble Flow	Very slight delay. Progression is very favorable, with most vehicles arriving during the greet phase not stopping at all.	Turning movements are easily made, and nearly all drivers find freedom of operation.	≤ 10.0 secs. ≤ 0.60 v/c.	\$1000 **********************************	10.0
B Statement	Stable Flow	Good progression and/or short cycle lengths. More vehicles stop than for LOS A, causing higher levels of average delay.	Vehicle platoons are formed. Many drivers begin to feel somewhat restricted within groups of vehicles.	>10 and ≤ 20.0 secs. 0.61 - 0.70 v/c	>10 and ≤ 15.0	>10 and ≤ 15.0
Ö	Stable Flow	Higher delays resulting from fair progression and/or longer cycle lengths. Individual cycle failures may begin to appear at this level. The number of velicles stopping is significant, although many still pass through the intersection without stopping.	Back-ups may develop behind turning vehicles. Most drivers feel somewhat restricted.	>20 and < 35.0 secs. 0.71 – 0.80 v/c	>15 and \$25.0.	% 25.05 \$1.5 (1.5 (1.5 (1.5 (1.5 (1.5 (1.5 (1.5 (
Q	Approaching Unstable Flow	The influence of congestion becomes more noticeable. Longer delays may result from some combination of unfavorable progression, long cycle lengths, or high volume-to-capacity ratios. Many vehicles stop, and the proportion of vehicles of stopping declines. Individual cycle failures are noticeable.	Maneuverability is severely limited during short periods due to temporary back-ups.	>35 and < 55.0 secs. 0.81 - 0.90 v/c	>25 and ≤ 35.0	>25 and < 35.0
Щ Н	Unstable Flow	Generally consideredifolds the limit of acceptable delay. Indicative of poor progression, long cycle lengths, and high volume-to-capacity ratios. Individual cycle failures are frequent occurrences	There are typically long quetes of vehicles waiting upstream of the intersection.	>55 and < 80.0 secs. 0.91 - 1.00 v/c	>35and <u>~</u> 50.0	>35 and <50.0
I.	Forced Flow	Generally considered to be unacceptable to most drivers. Often occurs with over saturation. May also occur at high volume-to-capacity ratios. There are many individual cycle failures. Poor progression and long cycle lengths may also be major contributing factors.	Jammed conditions. Back-ups from other locations restrict or prevent movement. Volumes may vary widely, depending principally on the downstream back-up conditions.	> 80.0 secs.	> 50.0	> 50.0

References: 1. Highway Capacity Manual, Fourth Edition, Transportation Research Board, 2000, Contra Costa Transportation Authority (CCTA), Technical Procedures Update, Final, July 9, 2006.

	1	*	1	1	-	↓						
Movement	WBL	WBR	NBT	NBR	SBL	SBT II		į.		na h		
Lane Configurations	¥		7+			4						
Sign Control	Stop		Free			Free	3.1					
Grade	0%	non-ways not use	0%	1000000 A VISL 1000	to make a front a first	0%	ne mar te access	transmission and the	and the second section of the	Dates with a second	a a territoria di sono estrato	C. Halling
Volume (veh/h)	4	1.	20	2		29	ärks					2
Peak Hour Factor	0.89	0.89	0.89	0.89	0.89	0.89	MAN HE TREES		70000 Z0000 Z - 1.00	87894SE778453886		558SE)
Hourly flow rate (vph) Pedestrians	4	10000000000000000000000000000000000000	22	2	2.14	33					\$ 1.50°	
Lane Width (ft)	H121025	EKANCYI		Walt by S		Description of	128		SECTION :	NOTE SENSE	350 (70 820)	
Walking Speed (ft/s)		TENUES:	5.35°N.75°N.0		- 65035720		BEST SERVE	No September	2006-2004-24	5.1 6.510 910 8		Sava.
Percent Blockage		N EXE	Fill C	2 1		348 T		SAME:		Mark Sales	70 FM	姜滋
Right turn flare (veh)	(400 v. 7.4 T. 100 000	***************************************	1995 100 Pu 100 (1016 14 20)	-40096-15000-R-0-G	SSSMANNERS TRACK	1940 NaM1230 X 60	action to the proper	SACHUL DEDA	2 -4 H458006910	Talkina NASA (Kapana) (1)	112605-12468-12286-1	2112363
Median type	None			F L				A TOTAL PARTY			W. ALVE	
Median storage veh)												
Upstream signal (ft)					1							
pX, platoon unblocked		NOTE THE PARTY OF	may Ket Year	mit n/4-0 (45)	**************************************	2.7664*78\30%1/5-7000	THE PROPERTY OF THE PERSON NAMED IN	more mercin	to a second cont.	3 SECTION 64 SEC. 12	22/95/01/5/20/90/09	£10000
vC, conflicting volume	58	24	445		25					110000		2.1
vC1, stage 1 conf vol vC2, stage 2 conf vol	1003115E1000	0.025 1.030 00.003		9.46.3659 EB-52.5	SPLESSA SE	N. CONTRACTOR AND	120 02000	DESCRIPTION OF THE PARTY OF THE	MESSAKORII			NOUSE
vCu, unblocked vol	58	24			25		2 1 0	FINE SERVICE		是是是是		194112
t©, single (s)	6.4	6.2		e de la composição	4.1	500 Mar 8/0		IN SHE			MINISTER IN	
tC, 2 stage (s)	EPRESAL OF	1993 H. L. S. L. C.	\$0\$50000000000000000000000000000000000	1000 A 100 A 1	Series and	CHARLE WE CH	10348/1025 1125	BORDER SEE		TO THE TEMPOR	S E. 1991 CT 00 20	3002
tF (s)	3.5	3.3		W N W	2.2						1004	
p0 queue free %	100	100	sine source 2 days ex	CO - 79 4 57 - 14 4 5	100	contract of North Forest	ZZ LIPOWA NA HA	The Table Consistence	19794499400.85	A 474-TO-48-A (1000-400)	00000 COLUMN 2001 - S.A.	331589
cM capacity (veh/h)	948	1053			1590							
Direction, Lane#	WB 1	NB 1	SB 1									# 87
Volume Total	6	25	34	Yê i di	TOT						302 11	
Volume Left	4	0	1					Large Palacia				
Volume Right	18 18	2	Ō		ot, d			A CONTRACTOR				
cSH	967	1700	1590	con Zestoby etc.			HEROA GINOSI	encettation of	PERSONALISM	WAS ALL TORONS	THE STATE OF THE S	91098
Volume to Capacity Queue Length 95th (ft)	0.01	0.01	0.00				4.2.28					
Control Delay (s)	0 8.7	0.0	0.2	Project Safe	A335667	RANGE CONTROL		G-0-1916		12/3554/11861	07F 143V5 C1	5 265
Lane LOS	Α	0.0	Α	E No.	er mende	TOTAL SAFETS	24日2	# 0.00 PEG	TOTAL SPACE	0.4158861.266		
Approach Delay (ŝ)	8.7	0.0	0.2	\$20 P. LONG	SERECTED STATES		MITTERS :	L MARKET	THE SECTION	KEELSTE	VACUUM.	
Approach LOS	A	222 W. W.		enation's		(Market)	100000000000000000000000000000000000000	S (86/2965)	ENCIA E	27 X X 2 X X X X X X X X X X X X X X X X	KAND SULYUR	19894
Intersection Summary												
Average Delay			0.9									-
Intersection Capacity Ut	ilization		13.3%	IC	CU Leve	el of Servi	ce		Α			
Analysis Period (min)	and the Section of		15	ent de la reine	and the second	NO DESCRIPTION AND ADDRESS OF THE PARTY OF T	error term at the	economic transfer	The section of the	A May now name		erga 7944
	CONTRACTOR	1000 2000	AZZ Z CERCITE	Vinctor Committee	EN SELECTION		POPER DIN	CHARLES AND	142	3072000.89131		1000

		4	†	~	-	↓	
Movement	WBL	WBR	NBT	NBR	SBL	SBT	
Lane Configurations	W		र्द			4	
Sign Control	Stop		Free			Free	
Grade	0%	National Contract	0%	renewater and	ng or wha	0%	essentationen in sind
Volume (veh/h) Peak Hour Factor	11 0.89	0.89	0.89	0.89	0.00	29 0.89	
Hourly flow rate (vph)	12	0.09	22	0.09	0.89	33	
Pedestrians	34-3-3		angl <u>h</u> 65	U - 1 - 7 ™			-31.0 FARES
Lane Width (ft)			200 E07274				
Walking Speed (ft/s)	SECRETARION PROPERTY.	Na Hotel Assertions	Caracanal College	100/00/12/6/44/00/2	A 16420000000 21440	0.5(2.5) + 65 + 65 - 7. (0.5) 2.5 2.5 2.5 2.5 2.5 2.5 2.5 2.5 2.5 2.5	aut. Publishmen
Percent Blockage			PRIM			AND	
Right turn flare (veh)	MOLMONOSONOTIAN	NO SERVE AND BUILDING	e contrate and con-	ni de terri in linne a cintilia	ALLOCATION THORSE NAME	Section 2.5 come annual contract of the desire of Laurence American Section 2.5 come and the contract of the c	We estimate Autorope has
Median type	None				Take 1		
Median storage veh) Upstream signal ² (ft)	Section and		SERVICE OF	TERCHINGS	532011055		NATION SEES PROPERTY.
pX, platoon unblocked	Class avs	Ser Constant	Treatment of the second		Market Will		a to Francisco
vC, conflicting volume	62	25		L	27		- 15 L S I I M
vC1, stage 1 conf vol	EROLLIS ERA	4415D/265-5000	109 Scotte Ball 50425	000044760760	STATE STATE	er of the second of the second se	MANUSCH FORZE
vC2, stage 2 conf vol		\$10 m					
vCu, unblocked vol	62	25	E. C. SWAN DO CO.	10.00 ME 100.00	27	The second secon	AND A SHALL SPECIAL AND A SHALL SHAL
tC, single (s)	6.4	6.2		MARK.	4.1	是多数的基础的 医二甲基苯酚	
tC, 2 stage (s)	3.5	3.3	CHE SEASON	minesta esta esta esta esta esta esta esta	0.0		SECTION DES
tF (s) p0 queue free %	99	100			2.2 100		
cM capacity (veh/h)	943	1052			1587		
A THE OWNER OF PROPERTY OF THE PARTY OF THE	CENTRAL SECTION OF SEC.	M. SECOMEDITIONS		1560333376	ASSESSMENT OF STREET		SECOND CONTRACTOR
Direction, Lane #	WB 1	NB 1	SB 1	可以有自己的			
Volume Total Volume Left	16 12	27 0	35 2				在 特别的
Volume Right	12	4	0	150011123		10.700	STATE OF STREET
cSH	964	1700	1587				i
Volume to Capacity	0.02		0.00		NO RESIDE		
Queue Length 95th (ft)	1	0	0	1817/2/17 (A. 1947) W. S. (B.)	THE STREET	A CALL TO THE STATE OF THE STAT	MARKET CHEMICAL SERVICES
Control Delay (s)	8.8	0.0	0.5				
Lane LOS	Α	Same or the	A	ORDANI MANDO A TARAN	000 c 215,510060		NOTES OF STREET
Approach Delay (s) Approach LOS	8.8 A	0.0	0.5	33.			
Intersection Summary						141至1515 1887 1827 7建设	N 16
Average Delay	et analyses to the		2.0	10 de 100 de			
Intersection Capacity Uti	lization	ar inx	13.3%	IC	U Leve	of Service A	18.35
Analysis Period (min)	SETTLE STORMER	contraction of	15	Secretary and the second	NESS/#06/9600111100	A.M. 186 (1714 A.M.) T. C. T.	200720000000000000000000000000000000000





Spring Mountain Road / Rockland Road (Fantesca Winery Access Road) Intersection



omni-means

figure 2



Fantesca Winery c/o Ms. Donna Oldford Plans 4 Wine 2620 Pinot Way St. Helena, CA 94574 May 2, 2013

Subject:

Traffic Analysis of Cumulative (Year 2030) Conditions for the Fantesca Winery on Spring Mountain Road in Napa County, CA.

Dear Ms. Oldford:

The following letter supplements our previous traffic report for the Fantesca Winery on Spring Mountain Road and Rockland Road in Napa County. At the County of Napa's request, this analysis has evaluated cumulative Year 2030 traffic conditions for the project. The analysis has evaluated conditions based on the forecast County model projections on Spring Mountain Road as well as historical volume trends. Operating conditions would remain acceptable on Spring Mountain Road and Rockland Road under cumulative without the project and cumulative with project conditions.

Transportation Model Forecast Volumes

The cumulative (Year 2030) volume projections on Spring Mountain Road were developed from the Napa County Transportation & Planning Agency's traffic volume forecasts in the Napa County General Plan Update EIR. The forecast increase in volume-to-capacity (v/c) ratio from Year 2003 to Year 2030 on Spring Mountain Road in the project vicinity was applied to the provided Year 2003 peak hour two-way volume of 76 trips on Spring Mountain Road. The resulting increase yields a cumulative Year 2030 volume of 1,155 weekday p.m. peak hour trips and 12,705 daily trips on Spring Mountain Road in Year 2030.

Projected Cumulative Volumes Based On Historical Data

Historical volume data on Spring Mountain Road from several years was obtained in order to identify historical volume growth (or decline) patterns. The County transportation model identified a Year 2003 weekday p.m. peak hour volume of 76 trips. Counts conducted by Napa County in Year 2006 identified a weekday p.m. peak hour volume of 92 trips and a daily volume of 1,015 trips near the St. Helena city limit.²

The counts conducted for the Fantesca Winery study in Year 2012 identified 57 p.m. peak hour trips, which is lower than the Year 2003 and Year 2006 volumes. Comparing only the years when volumes increased (Year 2003 to Year 2006), the volume growth equates to an annual increase of 6.7% per year. Conservatively applying the same annual increase to the current p.m. peak hour volume on Spring Mountain Road of 57 trips, results in approximately 183 p.m. peak hour trips and 2,015 daily trips in Year 2030.

It is noted that the volumes between Year 2003 and Year 2012 have declined. The historical growth calculations evaluated only the years when traffic volumes increased. Therefore, even the historical growth projections are likely conservatively high.

² Napa County, Traffic Volume Counts on Spring Mountain Road conducted March 29-30, 2006.

¹ Napa County, The Napa County General Plan Update EIR, prepared by Dowling Associates, Inc., February 9, 2007.

In order to identify weekend cumulative conditions, the General Plan Update provides a ratio of weekday to weekend peak hour volumes on key streets within the County. Spring Mountain Road in the vicinity of the project was shown to have an average ratio of 0.70, indicating weekend peak hour volumes are expected to be about 70% of weekday volumes. Therefore the future weekend volumes would be expected to be approximately 128 peak hour and 1,410 daily trips.

Cumulative Operating Conditions

The model forecast volumes yield conditions of LOS 'D' or better during the peak hour and LOS 'C' for the total daily volumes on Spring Mountain Road based on the Napa County volume thresholds for a rural two lane arterial road.³ Cumulative conditions based on the evaluated historical volume trends yields operating conditions of LOS 'A' on Spring Mountain Road. Therefore conditions would remain acceptable under cumulative without project conditions.

The proposed project would add 8-12 peak hour trips and 31-34 daily trips (weekends and weekdays) to Spring Mountain Road. Conditions with the project would remain unchanged from cumulative conditions without the project, continuing to operate at acceptable conditions (LOS 'D' or better). The Spring Mountain Road/Rockland Road intersection would also continue to operate at acceptable conditions (LOS 'C' or better) under cumulative and cumulative plus project conditions.

Although volumes may not increase to the model forecast levels, the County has adopted several measures to address potential volume increases identified in the General Plan to improve the street network and also reduce vehicle trips: "The project should support programs to reduce single occupant vehicle use and encourage alternative travel modes." In keeping with the policy, the winery should promote the use of carpooling by employees to the extent possible (by adjusting work schedules, etc.) in order to reduce single vehicle occupancy.

The County has identified other mitigation policies, including development of a traffic impact fee (TIF) to be developed in cooperation with the Napa County Transportation Planning Agency (Mitigation Measure 4.4.1C). This would require new projects to pay their "fair share" of countywide traffic improvements they contribute the need for. The concept is under development, but presumably the fee would be applied on a "per trip" basis if implemented.

Summary

Cumulative Year 2030 volumes were calculated from the County transportation model forecasts and from historical count data on Spring Mountain Road. Operating conditions would remain acceptable on Spring Mountain Road and Rockland Road under cumulative without the project conditions (operating at LOS 'D' or better) and would remain unchanged under cumulative with project conditions.

Sincerely,

George W. Nickelson, P.E. OMNI-MEANS, Ltd.

Engineers & Planners

rt/R1585TIA004.doc/35-5137-02

eorge Nichelson

Napa County, LOS Volume Thresholds, Baseline Data Report, Chapter 11 Transportation and Circulation, November 2005.



In order to identify weekend cumulative conditions, the General Plan Update provides a ratio of weekday to weekend peak hour volumes on key streets within the County. Spring Mountain Road in the vicinity of the project was shown to have an average ratio of 0.70, indicating weekend peak hour volumes are expected to be about 70% of weekday volumes. Therefore the future weekend volumes would be expected to be approximately 128 peak hour and 1,410 daily trips.

Cumulative Operating Conditions

The model forecast volumes yield conditions of LOS 'D' or better during the peak hour and LOS 'C' for the total daily volumes on Spring Mountain Road based on the Napa County volume thresholds for a rural two lane arterial road.³ Cumulative conditions based on the evaluated historical volume trends yields operating conditions of LOS 'A' on Spring Mountain Road. Therefore conditions would remain acceptable under cumulative without project conditions.

The proposed project would add 8-12 peak hour trips and 31-34 daily trips (weekends and weekdays) to Spring Mountain Road. Conditions with the project would remain unchanged from cumulative conditions without the project, continuing to operate at acceptable conditions (LOS 'D' or better). The Spring Mountain Road/Rockland Road intersection would also continue to operate at acceptable conditions (LOS 'C' or better) under cumulative and cumulative plus project conditions.

Although volumes may not increase to the model forecast levels, the County has adopted several measures to address potential volume increases identified in the General Plan to improve the street network and also reduce vehicle trips: "The project should support programs to reduce single occupant vehicle use and encourage alternative travel modes." In keeping with the policy, the winery should promote the use of carpooling by employees to the extent possible (by adjusting work schedules, etc.) in order to reduce single vehicle occupancy.

The County has identified other mitigation policies, including development of a traffic impact fee (TIF) to be developed in cooperation with the Napa County Transportation Planning Agency (Mitigation Measure 4.4.1C). This would require new projects to pay their "fair share" of countywide traffic improvements they contribute the need for. The concept is under development, but presumably the fee would be applied on a "per trip" basis if implemented.

Summary

Cumulative Year 2030 volumes were calculated from the County transportation model forecasts and from historical count data on Spring Mountain Road. Operating conditions would remain acceptable on Spring Mountain Road and Rockland Road under cumulative without the project conditions (operating at LOS 'D' or better) and would remain unchanged under cumulative with project conditions.

Sincerely,

George W. Nickelson, P.E. OMNI-MEANS, Ltd.

Engineers & Planners

rt / R1585TIA004.doc / 35-5137-02

eorge Dichelson

³ Napa County, LOS Volume Thresholds, Baseline Data Report, Chapter 11 Transportation and Circulation, November 2005.

